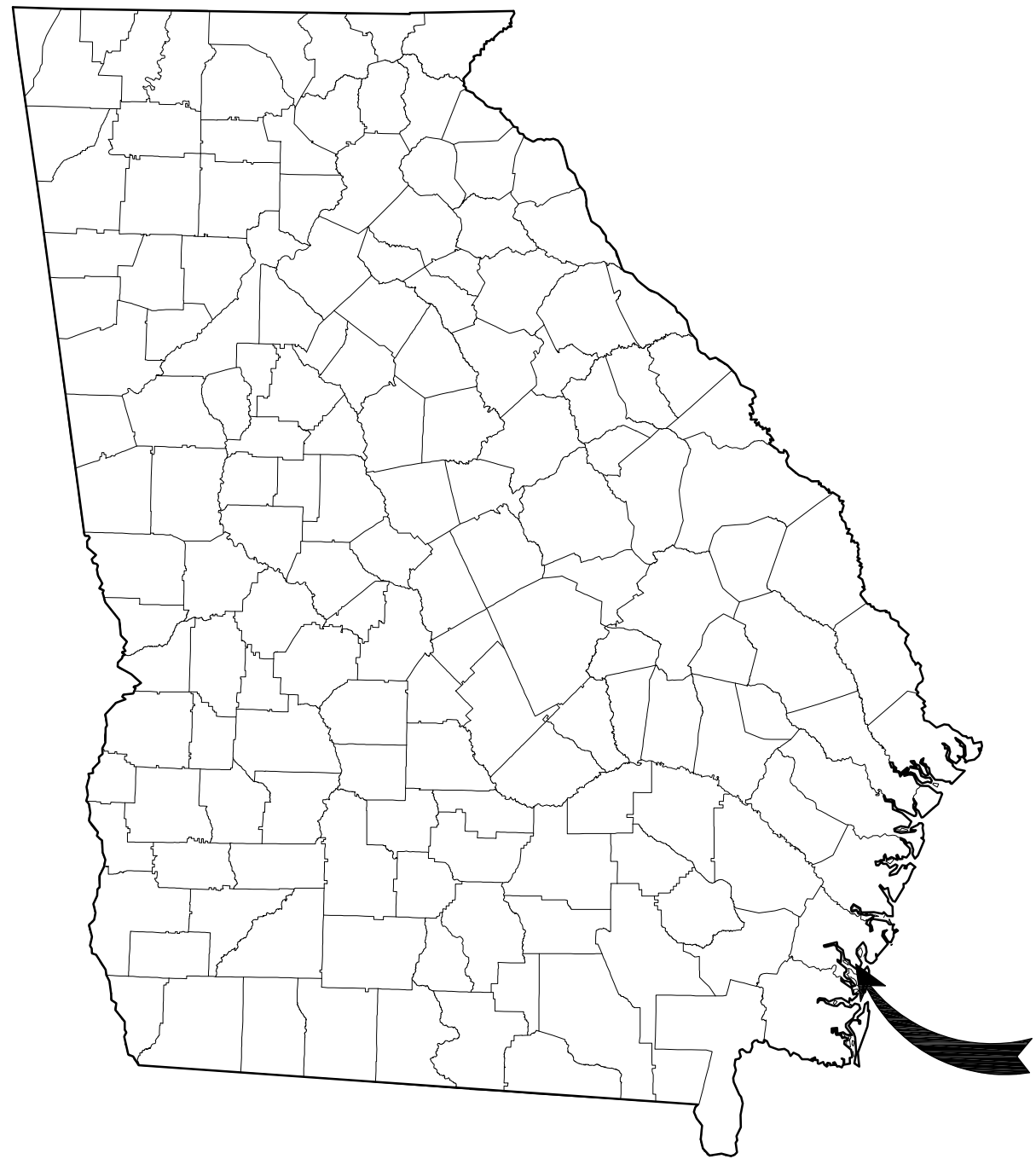


NOTES:
CONTRACTOR SHALL NOTIFY BGJWSC PLANNING
AND CONSTRUCTION DIVISION 48 HOURS BEFORE
STARTING WORK ON THIS PROJECT.



PROJECT
LOCATION

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MR. BOB DUNCAN, VICE CHAIRMAN
MR. CHARLES COOK
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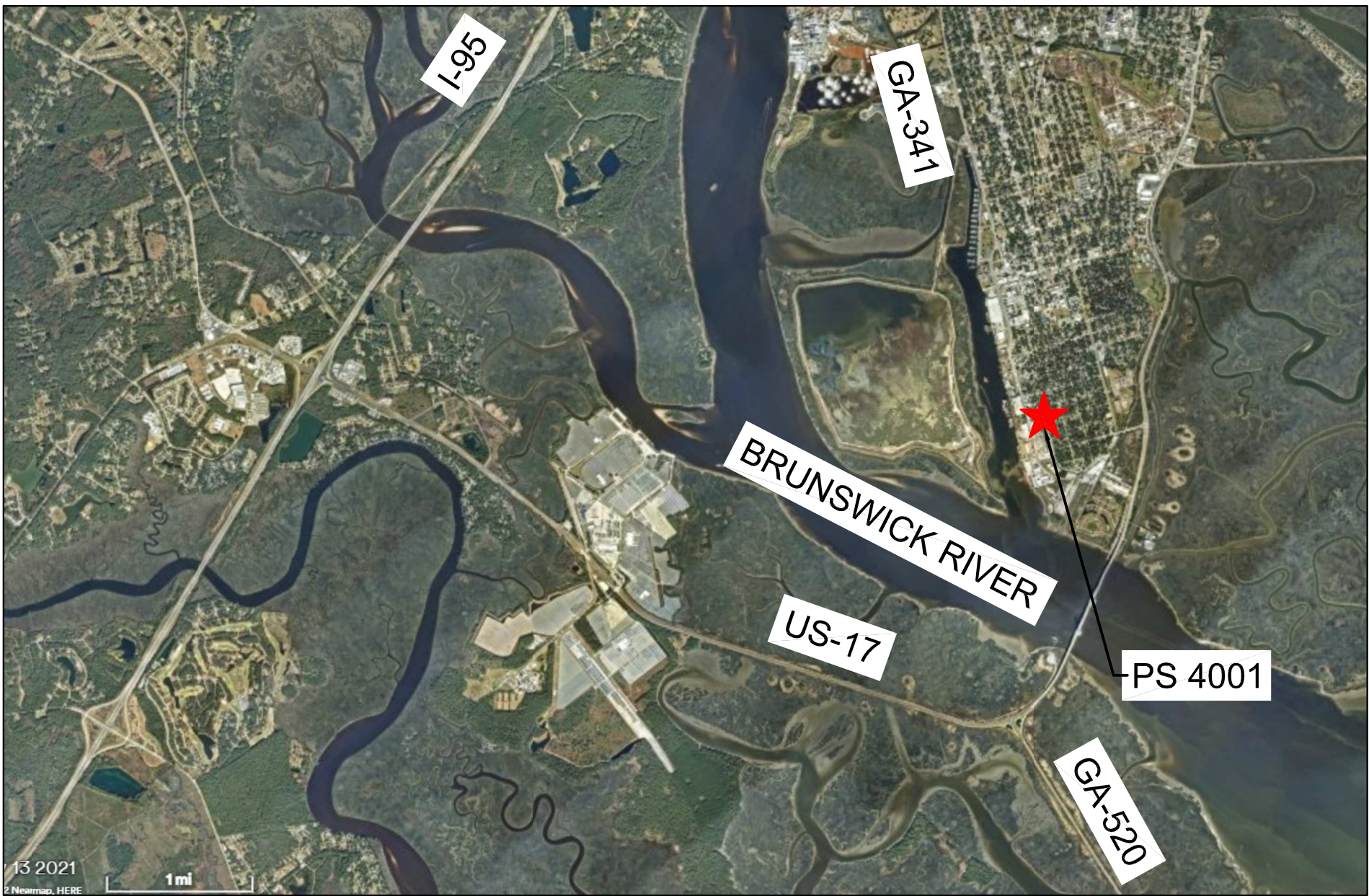
PUMP STATION UPGRADES
PS4001

BRUNSWICK-GLYNN JOINT
WATER & SEWER COMMISSION

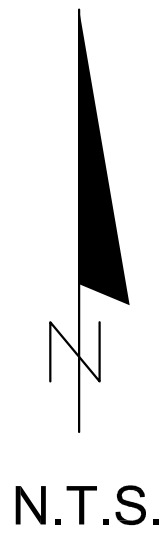
BGJWSC PROJECT #22-014
RE-BID



MARCH 2023



LOCATION MAP



DRAWING INDEX

SHEET NO.	SHEET TITLE
101	COVER SHEET
102	GENERAL NOTES
103	STRUCTURAL GENERAL NOTES
104	STRUCTURAL SPECIAL INSTRUCTIONS
105	ELECTRICAL SYMBOLS LEGEND, ABBREVIATIONS & SCHEDULES
301	PS4001 EXISTING SITE PLAN
302	PS4001 EROSION CONTROL & SEDIMENT PLAN
306	PS4001 ELECTRICAL DEMOLITION PLAN
307	PS4001 ELECTRICAL SITE PLAN
308	PS4001 ELECTRICAL DIAGRAMS AND DETAILS
501	EROSION CONTROL & SEDIMENT PLAN DETAILS
502	EROSION CONTROL & SEDIMENT PLAN DETAILS
507	MISC. TYP. STRUCTURAL DETAILS AND SECTIONS



Kimley»Horn

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LICENSED PROFESSIONAL

KHA PROJECT
045709000

DATE
MARCH 2023

SCALE
NTS

DESIGNED BY
DRM

DRAWN BY
DRM

CHECKED BY
MB/BB

JENNIFER THORINGTON-HINES

GEORGIA LICENSE NUMBER
RA075882

DATE:

COVER SHEET

ENGINEERING SERVICES FOR
PUMP STATION UPGRADES FOR
PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER
101



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GENERAL

1. THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE 2018 EDITION

2. ALL REFERENCES AND ASTM SPECIFICATIONS NOTED ON THESE DRAWINGS PERTAIN TO THE LATEST EDITIONS.

3. THE STRUCTURAL DRAWINGS ARE NOT STAND ALONE DOCUMENTS. THEY SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS.

4. THE CONTRACTOR SHALL COORDINATE THE SIZES AND LOCATIONS OF ALL PENETRATIONS WITH THE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS.

5. THE CONTRACT STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE A METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEANS NECESSARY TO PROTECT THE STRUCTURE AND PERSONNEL DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT LIMITED TO TEMPORARY BRACING, SHORING, FORMING, SCAFFOLDING, PLANKING, AND SAFETY NETS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES.

6. DRAWINGS SHALL NOT BE SCALED.

7. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO PROCEEDING WITH WORK.CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER AND ARCHITECT BY THE RFI SYSTEM OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION, ALONG WITH A RECOMMENDED SOLUTION.

8. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION.

9. WHERE A SECTION OR DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY TO ALL LIKE AND SIMILAR CONDITIONS.

10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND DETAIL OF SIMPLE (SHEAR ONLY CONNECTIONS AND MOMENT CONNECTIONS NOT SHOWN ON THE DRAWINGS.

11. CONTRACTOR SHALL ENSURE THAT STRUCTURAL MEMBERS ARE NOT LOADED IN EXCESS OF DESIGN LIVE LOADS DURING CONSTRUCTION. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT WHEN PLACED ON FLOOR AND ROOF FRAMING.

12. WHEN DIGGING ADJACENT TO EXISTING BUILDING CONTRACTOR SHALL MAKE SURE TO NOT LOAD EXISTING BUILDING BELOW GROUND WALLS WITH HEAVY EQUIPMENT. PLEASE PLACE THE EQUIPMENT AT 45 DEGREES AWAY FROM THE BOTTOM OF THE FOOTING TO MAKE SURE THE LOAD IS NOT DIRECTLY TRANSFERRED. SMALLER OR MANUAL EQUIPMENT SHALL BE USED TO BACKFILL AND COMPACT THE SOIL AFTER THE WORK IS PERFORMED.

DESIGN LOADS

DEAD LOAD

THE WEIGHT OF THE STRUCTURAL MEMBERS THEMSELVES AND ALL PERMANENT CONSTRUCTION INCLUDING WALLS, FLOORS, CEILINGS, ROOF CLADDING AND FIXED EQUIPMENT.

LIVE LOAD ROOF LIVE LOAD REDUCTION WAS NOT USED

ROOF.....20 PSF

SNOW LOAD

GROUND SNOW LOAD (Pg).....5 PSF

WIND LOAD

ULTIMATE DESIGN WIND SPEED (V ULT)..... 120 MPH
NOMINAL DESIGN WIND SPEED (V ASD)..... 92 MPH
RISK CATEGORY.....IV
WIND EXPOSURE.....C
INTERNAL PRESSURE COEFFICIENT.....+/-0.18
COMPONENTS AND CLADDING
 ZONE - EFFECTIVE WIND AREA
 ZONE 4 - 10SF.....-33 PSF
 ZONE 4 - 20SF.....-28 PSF
 ZONE 4 - 50SF.....-25 PSF
 ZONE 5 - 10SF.....-40 PSF
 ZONE 5 - 20SF.....-31 PSF
 ZONE 5 - 50SF.....-25 PSF
BUILDING CLASSIFICATION.....ENCLOSED

SEISMIC LOAD

RISK CATEGORY.....II
SEISMIC IMPORTANCE FACTOR.....1.5
Ss.....0.183
S1.....0.089
SITE CLASS.....C
SDS0.196
SD10.143
SEISMIC DESIGN CATEGORY.....D
BASIC SEISMIC FORCE-RESISTING SYSTEM....ORDINARY REINFORCED MASONRY SHEAR WALL
DESIGN BASE SHEAR.....30KIPS
SEISMIC RESPONSE COEFFICIENT,Cs.....0.1095
RESPONSE MODIFICATION COEFFICIENT(S), R.....2
ANALYSIS PROCEDURE USED.....EQUIVALENT STATIC ANALYSIS

FOUNDATIONS

1. THE FOUNDATION AND SLAB ON GRADE DESIGNS SHALL BE ASSUMPTIONS.

2. THE SHALLOW FOUNDATIONS ARE DESIGNED WITH A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF. THE MINIMUM BEARING CAPACITY SHALL BE VERIFIED ON SITE BY A GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION CONSTRUCTION.

3. FOOTINGS SHALL BEAR ON SUITABLE RESIDUAL SOIL A MINIMUM OF 16" BELOW ADJACENT FINISHED EXTERIOR GRADES

4. FOUNDATION SIDES MAY BE EARTH FORMED IF APPROVED BY A GEOTECHNICAL ENGINEER.

5. THE SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH THOSE MEASURES OUTLINED IN THE GEOTECHNICAL REPORT AS WELL AS THE CIVIL DRAWINGS AND SPECIFICATIONS.

6. CONTROL (SAW CUT) JOINTS SHALL BE PLACED IN THE SLAB ON GRADE AT EACH COLUMN LINE AND AT INTERMEDIATE LOCATIONS. SPACING OF TRANSVERSE CONTROL JOINTS SHALL NOT EXCEED 12'X12'.

CONCRETE (CAST-IN-PLACE)

1. DESIGN OF CONCRETE IS BASED ON ACI 318. CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 301.

2. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL DEVELOP A MINIMUM 28 DAY COMPRESSIVE STRENGTH AS FOLLOWS:
 SLAB ON GRADE.....3500 PSI
 SPREAD FOOTINGS.....3500 PSI
 GROUT IN MASONRY WALL4000 PSI
 NON-SHRINK GROUT AT BASE PLATES.....4000 PSI

3. ALL TENSION SPLICES INCLUDING THOSE BARS NOTED AS CONTINUOUS SHALL BE CLASS B IN ACCORDANCE WITH ACI 318. SPLICES SHALL BE STAGGERED WHERE POSSIBLE.

4. CONCRETE REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315 AND ACI 318. REINFORCING SHALL CONFORM AS FOLLOWS:
 REINFORCING STEEL.....ASTM A615 GRADE 60
 WELDED WIRE REINFORCING (WWR).....ASTM A185
 DOWELS.....ASTM A615 GRADE 60
 DOWELS.....ASTM A663 GRADE 60

5. WELDED WIRE REINFORCING SHALL BE PROVIDED IN FLAT SHEETS AND LAPPED A MINIMUM OF TWO FULL PANELS AND TIED ON EACH SIDE.

6. UNLESS INDICATED OTHERWISE, CONCRETE COVER OVER REINFORCEMENT SHALL BE AS FOLLOWS:
 - CONCRETE NOT EXPOSED TO EARTH OR WEATHER
 FOR #11 BARS OR SMALLER (SLABS, WALLS, JOISTS).....3/4"
 BEAMS AND COLUMNS.....1 1/2"
 -CONCRETE EXPOSED TO EARTH OR WEATHER
 FOR #5 BARS OR SMALLER (INCLUDING WWR).....1 1/2"
 FOR #6 BARS OR LARGER.....2"
 CONCRETE CAST DIRECTLY AGAINST EARTH.....3"

7. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER INSERTS AND EMBEDS SHALL BE SECURED IN POSITION, INSPECTED AND APPROVED PRIOR TO PLACING CONCRETE. REINFORCEMENT IN SLABS AND SLAB ON GRADE SHALL BE PLACED ON CHAIRS AT 36" MAX IN EACH DIRECTION. DOWELS SHALL NOT BE INSERTED INTO FRESHLY PLACED CONCRETE.

8. REINFORCING BARS AND ACCESSORIES SHALL NOT BE IN CONTACT WITH ANY PIPE, PIPE FLANGE OR METAL PARTS EMBEDDED IN CONCRETE, A MINIMUM OF 2 INCHES CLEARANCE SHALL BE PROVIDED

9. ADDITIONAL REINFORCING (SEE TYPICAL DETAILS) SHALL BE PROVIDED AT THE FOLLOWING:
 GRADE BEAM CORNERS AND INTERSECTIONS
 REENTRANT CORNERS
 PENETRATIONS

10. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4"

11. CONCRETE REINFORCEMENT SHALL NOT BE WELDED UNLESS APPROVED BY THE ENGINEER OF RECORD.

GROUT :

1. GROUT UNDER BEARING PLATES SHALL BE NON-SHRINK GROUT- 60,000 PSI MINIMUM COMPRESSIVE STRENGTH. MIXING AND PLACEMENT SHALL BE IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

2. ANCHORING CEMENT FOR RAILINGS: MIX PREPARED ANCHORING CEMENT PRODUCT WITH WATER AS DIRECTED BY MANUFACTURER'S PRODUCT DATE FOR IMMEDIATE USE.

METAL ROOF DECK

1. CONFORM TO STEEL DECK INSTITUTE DESIGN MANUAL, LATEST EDITION.

2. UNLESS NOTED OTHERWISE, ROOF DECK SHALL BE WIDE-RIB (GALVANIZED - ASTM-A653-94, G60). MINIMUM YIELD STRENGTH SHALL BE 33000 PSI.

3. ROOF DECK SHALL BE ATTACHED AS FOLLOWS:
 1. AT SUPPORTS: #12 TEK SCREWS (AT STEEL)

 2. AT SIDELAPS: #10 TEK SCREWS

4. DECK WILL BE PLACED AT THE PERIMETER WITH A COMPLETE RIB BEARING ON THE STEEL SUPPORT. DECK SHALL BE SUPPORTED BY A MINIMUM OF FOUR SUPPORT LOCATIONS (THREE SPAN CONDITION).

STRUCTURAL STEEL

1. DESIGN OF STRUCTURAL STEEL IS BASED ON THE AISC STEEL CONSTRUCTION MANUAL INCLUDING AISC-360.
 MATERIALS:
 WIDE FLANGE SECTIONS..... ASTM A992 FY50KSI
 RECT. STRUCTURAL TUBING..... ASTM A500 GR B FY46KSI
 CHANNELS AND ANGLES..... ASTM A36 FY36KSI
 PLATES..... ASTM A36 FY36KSI
 HIGH STRENGTH BOLTS.....ASTMA325
 ANCHOR BOLTS..... ASTM F1554GR36

2. STRUCTURAL STEEL SHALL BE NEW DOMESTIC STEEL WITH ALL DETAILING, FABRICATION AND ERECTION CONFORMING TO ALL APPLICABLE PROVISIONS SPECIFIED BY AISC.

3. THE DESIGN OF CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT INDICATED ON THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR AS FOLLOWS:
 3/4" ASTM A325 (GROUP A) HIGH STRENGTH BOLTS SHALL BE USED IN SIMPLE SHEAR BEARING TYPE CONNECTIONS WHERE POSSIBLE. SLIP CRITICAL CONNECTIONS SHALL BE USED FOR MOMENT CONNECTIONS.

 SHOP CONNECTIONS SHALL BE WELDED OR BOLTED. FIELD CONNECTIONS SHALL BE BOLTED WHERE POSSIBLE.

 FOR BEAMS: PROVIDE THE MINIMUM NUMBER OF BOLTS REQUIRED TO DEVELOP THE BEAM SHEAR "V" AND MOMENT "M" (IF APPLICABLE) NOTED ON THE CONTRACT DRAWINGS. IF THE BEAM SHEAR IS NOT NOTED, THE CONNECTION SHALL DEVELOP THE BEAM V = W/2 WHERE W IS THE MAXIMUM TOTAL UNIFORM LOAD BASED ON LATERALLY SUPPORTED SIMPLE SPAN AS SHOWN IN TABLES 3-6 THROUGH 3-9 IN THE AISC STEEL CONSTRUCTION MANUAL.

4. ALL FIELD WELDING SHALL BE DONE WITH E70XX ELECTRODES.

5. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND CONFORM TO THE AMERICAN WELDING SOCIETY ANSI/AWS D1.1.

6. THE MINIMUM WELD SIZE SHALL BE 3/16" U.N.O.

7. EXPOSED WELDED CONNECTIONS SHALL BE GROUND SMOOTH.

8. THE FABRICATOR SHALL PREPARE AND SUBMIT FOR REVIEW DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DIAGRAM. FABRICATION SHALL NOT BEGIN UNTIL STEEL SHOP DRAWINGS HAVE BEEN COMPLETED AND REVIEWED BY THE ENGINEER OF RECORD.

9. STRUCTURAL STEEL SHALL BE SHOP PAINTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. DO NOT PAINT MEMBERS THAT ARE TO RECEIVE SPRAY ON FIRE PROOFING, SLIP CRITICAL CONNECTIONS, SURFACES TO BE FIELD WELDED, OR THE TOP FLANGE OF COMPOSITE BEAMS TO RECEIVE HEADED STUDS.

10. INSTALL STRUCTURAL STEEL BEAMS WITH NATURAL CAMBER UP.

11. STRUCTURAL STEEL SHALL NOT BE CUT IN THE FIELD FOR WORK OF OTHER TRADES WITHOUT PRIOR APPROVAL OF THE ENGINEER OF RECORD.

12. ALL STEEL EXPOSED TO WEATHER INCLUDING LINTELS SHALL BE HOT DIPPED GALVANIZED.

13. PACK NON SHRINK HIGH STRENGTH (MIN 6000 PSI) GROUT UNDER ALL COLUMN BASE PLATES AFTER SETTING AND LEVELING BEFORE ADDING ANY LOAD. BASE PLATES SHALL BE LEVELED WITH DOUBLE NUTS.

14. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STEEL FRAME IN PROPER ALIGNMENT UNTIL ALL FLOOR AND ROOF DECK, DIAGONAL BRACING, FLOOR SLABS, WELDED CONNECTIONS, ETC. HAVE BEEN INSTALLED AND THE CONCRETE HAS DEVELOPED A STRENGTH OF 3000 PSI.

DESIGN CRITERIA – CODES AND SPECIFICATIONS

1. INTERNATIONAL BUILDING CODE 2018.

2. ACI318-14-BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.

3. ACI301-10–SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.

4. ANSI/AISC 360-10–SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.

5. ASCE/SEI 7-10–MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

6. SDI DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS–NO31

7. SDI DIAPHRAGM DESIGN MANUAL THIRD EDITION.

8. SJI STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOISTS GIRDERS 42ND EDITION 2005.

9. ACI 530-11 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.

CONCRETE MASONRY :

1. ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530, ASCE 5, TMS 402, ASCE 6 AMD TMS 602.

2. CONCRETE MASONRY UNITS SHALL BE LIGHT WEIGHT AND CONFORM TO ASTM C 90. LAY IN RUNNING BOND UNLESS NOTED. F'm SHALL BE 2000 PSI

3. JOINT REINFORCING - TRUSS TYPE, 9 GAUGE OR W1.7 SPACED VERTICALLY AT 16" UNLESS NOTED OTHERWISE AND CONFORM TO ASTM A82.

4. VERTICAL REINFORCING IN CONCRETE MASONRY (AS REQUIRED) SHALL BE DOWELED INTO THE FOUNDATION AND EXTEND INTO THE BOND BEAM AT THE ROOF. PROVIDE MIN. 4" x 4" OPENING AT U BLOCK FOR VERTICAL BAR.

5. PROVIDE REINFORCING IN CONCRETE MASONRY GROUTED CELLS AT EACH SIDE OF OPENING, EQUAL TO THE REINFORCING DISPLACED.

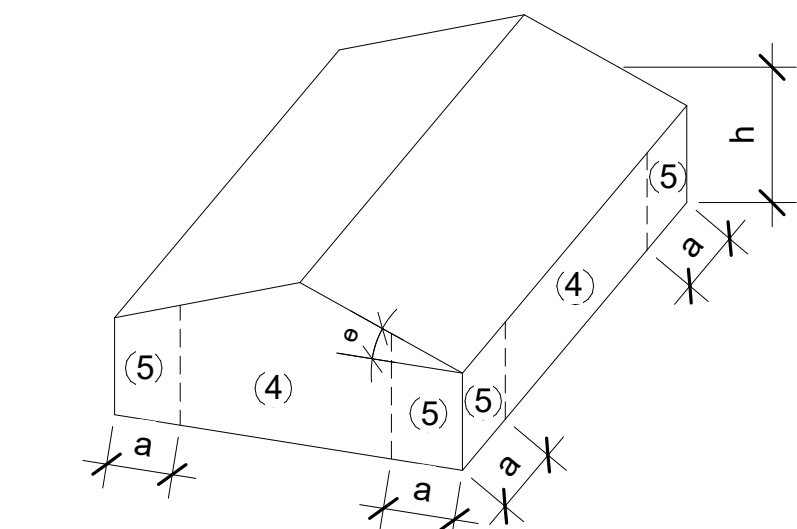
6. PROVIDE JOINT REINFORCING AT 8" AT MASONRY BELOW GRADE. 2 ROWS AT 8" AT TOP AND BOTTOM OF OPENINGS, (EXTEND 24" EACH SIDE) AND 2 ROWS AT 8" AT BOND BEAMS.

7. CONCRETE MASONRY UNITS SHALL BE CUT BELOW BOND BEAMS AS REQUIRED IN ORDER TO GET BOND BEAMS AT THE PROPER ELEVATION. ALL CELLS BELOW GRADE AND SLAB ON GRADE SHALL BE GROUTED SOLID.

9. HORIZONTAL BEAMS, BOND BEAMS AND REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS AT CONTRACTOR OPTION.

10. 16" DEEP BOND BEAMS MAY BE CONSTRUCTED OF 8" U BLOCK BELOW AND 8" STANDARD BLOCK ABOVE WITH BREAK AWAY TOP PART OF WEB.

11. SEE ARCHITECTURAL DRAWINGS FOR LAYING MASONRY AND LOCATION OF OPENINGS.



WIND ZONES. COMPONENTS AND CLADDING.



03/06/2023

BID DOCUMENTS

ENGINEERING SERVICES FOR
PUMP STATION UPGRADES FOR
PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER

103

LICENSED PROFESSIONAL:

KHA PROJECT
045709000

DATE
MARCH 2023

SCALE: AS SHOWN

DESIGNED BY: BG

DRAWN BY: AT

CHECKED BY: BG

BALA GULLIPALLI, P.E.

GEORGIA LICENSE NUMBER
042400

DATE:

Kimley»Horn

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REGISTRY NO. 686

REVISIONS

No.

DATE

BY

ACI 530 - TABLE 3.1.2				
MASONRY LEVEL B QUALITY ASSURANCE				
MINIMUM TESTS				
VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT				
VERIFICATION OF f'_m AND f'_{AAC} IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE				
MINIMUM SPECIAL INSPECTION				
INSPECTION TASK	FREQUENCY (a)		REFERENCE FOR CRITERIA	
	CONTINUOUS	PERIODIC	TMS 402/ ACI 530/ ASCE 5	TMS 602/ ACI 530.1/ ASCE 6
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS		X		ART. 1.5
2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
a. PROPORTIONS OF SITE-PREPARED MORTAR		X		ART. 2.1 , 2.6 A
b. CONSTRUCTION OF MORTAR JOINTS		X		ART. 3.3 B
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		–		ART. 2.4 B , 2.4 H
d. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES		X		ART. 3.4 , 3.6 A
e. PRESTRESSING TECHNIQUE		–		ART. 3.6 B
f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	(b) –	(c) –		ART. 2.1 C
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
a. GROUT SPACE		X		ART. 3.2 D , 3.2 F
b. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		X	SEC. 6.1	ART. 2.4 , 3.4
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES		X	SEC. 6.1, 6.2 , 6.2.6 , 6.2.7	ART. 3.2 E , 3.4 , 3.6 A
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		X		ART. 2.6 B , 2.4 G.1b
e. CONSTRUCTION OF MORTAR JOINTS		X		ART. 3.3 B
4. VERIFY DURING CONSTRUCTION:				
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X		ART. 3.3 F
b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION		X	SEC. 1.2.1 (e) , 6.1.4.3 , 6.2.1	
c. WELDING OF REINFORCEMENT	–		SEC. 8.1.6.7.2 , 9.3.3.4 (c) , 11.3.3.4 (b)	
d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40F (4.4C)) OR HOT WEATHER (TEMPERATURE ABOVE 90F (32.2C))		X		ART. 1.8 C , 1.8 D
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	–			ART. 3.6 B
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	–			ART. 3.5 , 3.6 C
g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	(b) –	(c) –		ART. 3.3 B.9 , 3.3 F.1.b
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X		ART. 1.4 B.2.a.3 , 1.4 B.2.b.3 , 1.4 B.2.c.3 , 1.4 B.3 , 1.4 B.4

- (a) FREQUENCY REFERS TO THE FREQUENCY OF SPECIAL INSPECTION, WHICH MAY BE CONTINUOUS DURING THE TASK LISTED OR PERIODIC DURING THE LISTED TASK, AS DEFINED IN THE TABLE.
- (b) REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.
- (c) REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY.

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD	IBC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	----	X	ACI 318: 3.5, 7.1-7.7	1910.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1705.2.2, ITEM 2B.	----	----	AWS D1.4 ACI 318: 3.5.2	----
3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	----	----	ACI 318: 8.1.3,21.1.8	1908.5, 1909.1
4. INSPECTION OF ANCHORS POST INSTALLED IN HARDENED CONCRETE MEMBERS.	----	X	ACI 318: 3.8.6,8.1.3,21.1.8	1909.1
5. VERIFYING USE OF REQUIRED DESIGN MIX.	----	X	ACI 318: CH. 4, 5.2-5.4	1904.2,1910.2, 1910.3
6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	----	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	----	ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	----	X	ACI 318: 5.11-5.13	1910.9
9. INSPECTION OF PRESTRESSED CONCRETE: a. APPLICATION OF PRESTRESSING FORCES. b. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM.	---- ----	----	ACI 318: 18.20 ACI 318: 18.18.4	----
10. ERECTION OF PRECAST CONCRETE MEMBERS.	----	----	ACI 318: CH. 16	----
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	----	----	ACI 318: 6.2	----
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	----	----	ACI 318: 6.1.1	----
NOTES: FOR SI: 1 INCH = 25.4 MM. A. SEE SPECIAL INSPECTION NOTES ON GENERAL NOTE SHEET FOR ADDITIONAL INFORMATION. B. WHERE APPLICABLE, SEE ALSO SECTION 1705.11, SPECIAL INSPECTION FOR SEISMIC RESISTANCE.				

IBC - TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL (IBC 2018)			
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD
1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK:			
a. IDENTIFICATION MARKINGS TO CONFIRM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	----	X	APPLICABLE ASTM MATERIAL STANDARDS
b. MANUFACTURER'S CERTIFICATE TEST REPORTS.	----	X	
2. INSPECTION OF WELDING:			
a . COLD-FORMED STEEL DECK:			
1) FLOOR AND ROOF DECK WELDS.	----	----	
b . REINFORCING STEEL:			
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	----	----	AWS D1.4 ACI 318: SECTION 3.5.2
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	----	----	
3) SHEAR REINFORCEMENT.		----	
4) OTHER REINFORCING STEEL.	----	----	
3. COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER.			
a . VERIFY TEMPORARY AND PERMANENT RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE.	----	----	



TYPE		CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1.	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	—	X
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	—	X
3.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	—	X
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	—
5.	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	—	X

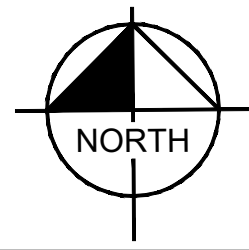
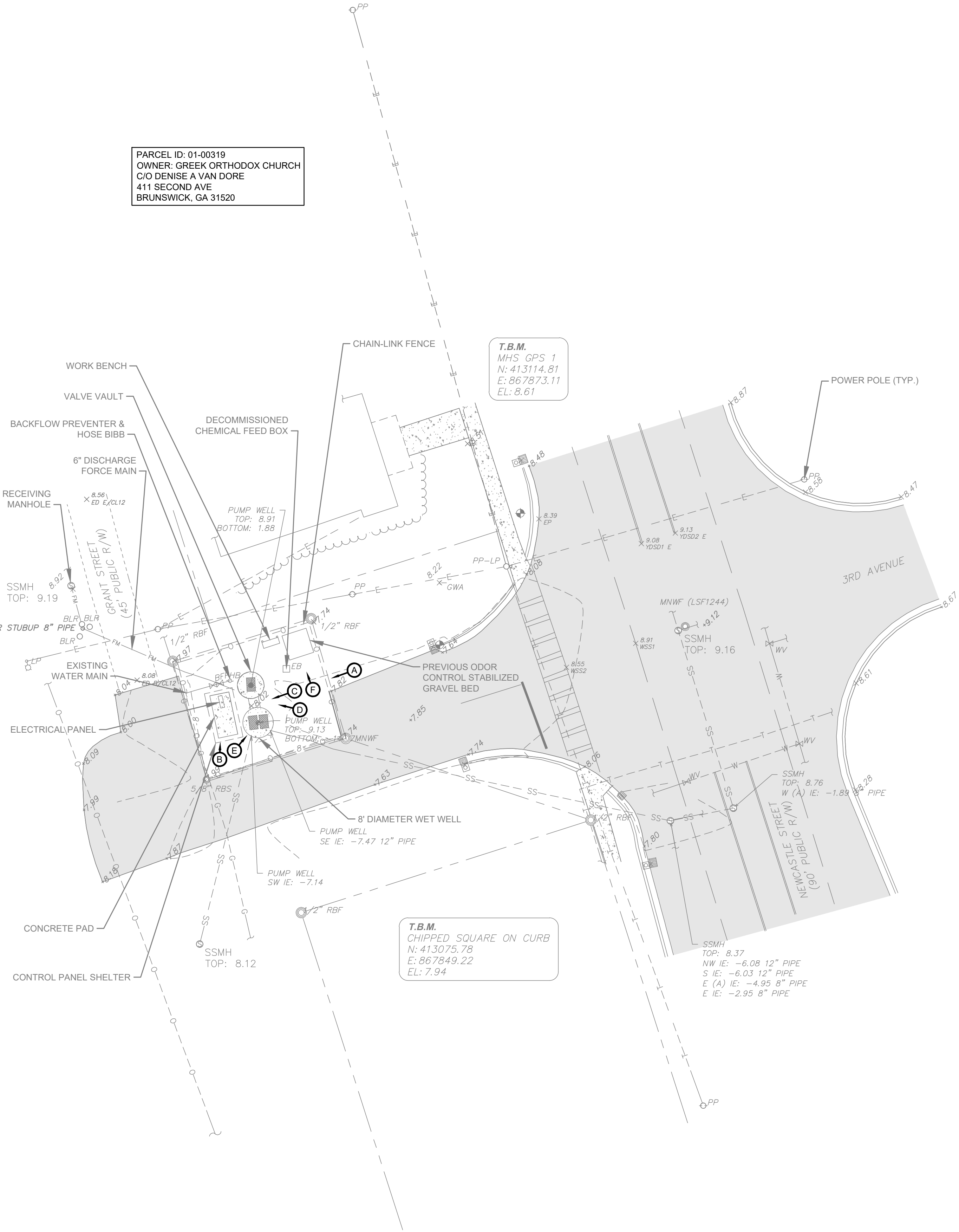


BID DOCUMENTS

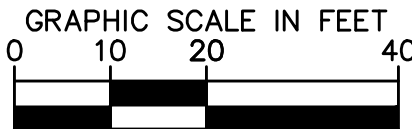
ENGINEERING SERVICES FOR PUMP STATION UPGRADES FOR PS4001 PREPARED FOR BGJWSC	STRUCTURAL SPECIAL INSTRUCTIONS	<div><div><div>KHA PROJECT 0457090000</div><div>DATE MARCH 2023</div><div>SCALE: AS SHOWN</div><div>DESIGNED BY: BG</div><div>DRAWN BY: AT</div><div>CHECKED BY: BG</div></div><div><div>LICENSED PROFESSIONAL: BALA GULLIPALLI, P. E.</div><div>GEORGIA LICENSE NUMBER 042400</div><div>DATE:</div></div></div> <div><div><div><div><div><div>Kimley»»Horn</div><div>© 2021 KIMLEY»»HORN AND ASSOCIATES, INC. 12720 GRAN BAY PARKWAY WEST, SUITE 2350 JACKSONVILLE, FLORIDA 32258 JACKSONVILLE, FLORIDA 32258 PHONE: 904-836-3800 WWW.KIMLEY»»HORN.COM</div><div>REGISTRY NO. 686</div></div></div><div><div>No.</div><div>REVISIONS</div><div>DATE</div><div>BY</div></div></div></div></div>
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Plotted: B:\Eagle, Alex, Sheet, Set: BG, WSC, Layout: 301, PS4001, EXISTING SITE PLAN, May 30, 2023, 11:24:31am. K:\JAX, WaterResources\045709000 - Brunswick Pump Station Upgrades\Re-Big Development\CAD\4001\Plan\Sheet\301_PS4001_EXISTING_SITE_PLAN.dwg

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PS4001 EXISTING SITE PLAN



A



B



C



D



E



F

PS4001 EXISTING CONDITION

NOTES:

- 1) LIFT STATION IS LOCATED AT 301 THIRD AVENUE, BRUNSWICK, GEORGIA 31520.
- 2) SURVEY PROVIDED BY EMC ENGINEERING SERVICES, INC. DATED MAY 18, 2022. HORIZONTAL DATUM IS GEORGIA STATE PLANE COORDINATE SYSTEM OF 1985, EAST ZONE, NORTH AMERICAN DATUM OF 1983 (NAD83). VERTICAL DATUM IS NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
- 3) ALL DIMENSIONS ARE APPROXIMATE AND TO BE FIELD VERIFIED.
- 4) CONTRACTOR TO PROTECT EXISTING GAS MAIN.
- 5) EXISTING WATER SERVICE SIZE, LOCATION, AND MATERIAL TO BE FIELD VERIFIED.
- 6) CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS PRIOR TO BEGINNING CONSTRUCTION AND ORDERING EQUIPMENT. CONTRACTOR SHALL NOTIFY ENGINEER OF DISCREPANCIES.
- 7) CONTRACTOR SHALL SUBMIT A CONSTRUCTION SEQUENCE SCHEDULE AS PART OF HIS SUBMITTALS FOR REVIEW AND APPROVAL FROM OWNER/ENGINEER TO MINIMIZE FLOW INTERRUPTIONS.
- 8) THE EXISTING STORMWATER DRAINAGE SYSTEM SHALL REMAIN FUNCTIONAL AND BE MAINTAINED BY THE CONTRACTOR DURING CONSTRUCTION.
- 9) GEORGIA 811 DESIGN TICKET #220624-003735 RESPONDENT CONTACT INFORMATION IS LISTED BELOW.

UTILITY CONTACT LISTING

ELECTRIC & NATURAL GAS:

ATLANTA GAS LIGHT
10 PEACHTREE PL NE
ATLANTA, GA 30309
PHONE: (800) 427-5463

TELECOMMUNICATIONS:

ALMA TELEPHONE COMPANY
405 W 11TH ST
ALMA, GA 31510
PHONE: (912) 632-8603

TELECOMMUNICATIONS:

AT&T
1100 GLYNN ISLE
BRUNSWICK, GA 31525
(912) 324-2298

MUNICIPAL UTILITIES:

BRUNSWICK GLYNN COUNTY JOINT WATER & SEWER
1703 GLOUCESTER ST
BRUNSWICK, GA 31520
PHONE: (912) 261-7100

WATER:

WATER UTILITY MANAGEMENT
28 ABERCORN ST.
SAVANNAH, GA 31401
PHONE: (912) 352-9339

MUNICIPAL UTILITIES:

GLYNN COUNTY PUBLIC WORKS
4145 NORWICH ST. EXT
BRUNSWICK, GA 31520
(912) 554-7111

MUNICIPAL UTILITIES:

GLYNN COUNTY BOARD OF EDUCATION
1313 EGMONT ST
BRUNSWICK, GA 31520
PHONE: (912) 267-4100

ELECTRIC & NATURAL GAS:

GEORGIA POWER COMPANY
800 GLOUCESTER ST
BRUNSWICK, GA 31520
PHONE: (888) 891-0938

EXISTING CONDITIONS TABLE	
WET WELL DIAMETER	8 FT
PUMPS (QUANTITY)	2
PUMP MANUFACTURER/MODEL	WILO/FA 0.51E; 10 H.P.
BASIS OF DESIGN CAPACITY	504 GPM @ 35.5 TDH
PUMP DISCHARGE ASSEMBLY (QUANTITY)	2



BID DOCUMENTS

PS4001 EXISTING
SITE PLAN

PUMP STATION
UPGRADES PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER
301

Kimley»Horn

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LICENSED PROFESSIONAL

JOSHUA S. PETERSEN, P.E.

GEORGIA LICENSE NUMBER
036753

DATE:

KHA PROJECT
045709000

DATE
MAY 2023

SCALE
AS SHOWN

DESIGNED BY
JSP

DRAWN BY
ACE

CHECKED BY
KBS

No.

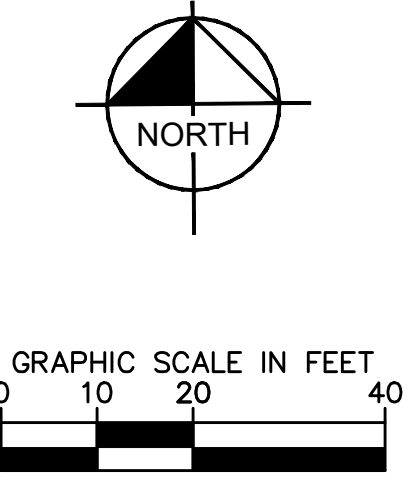
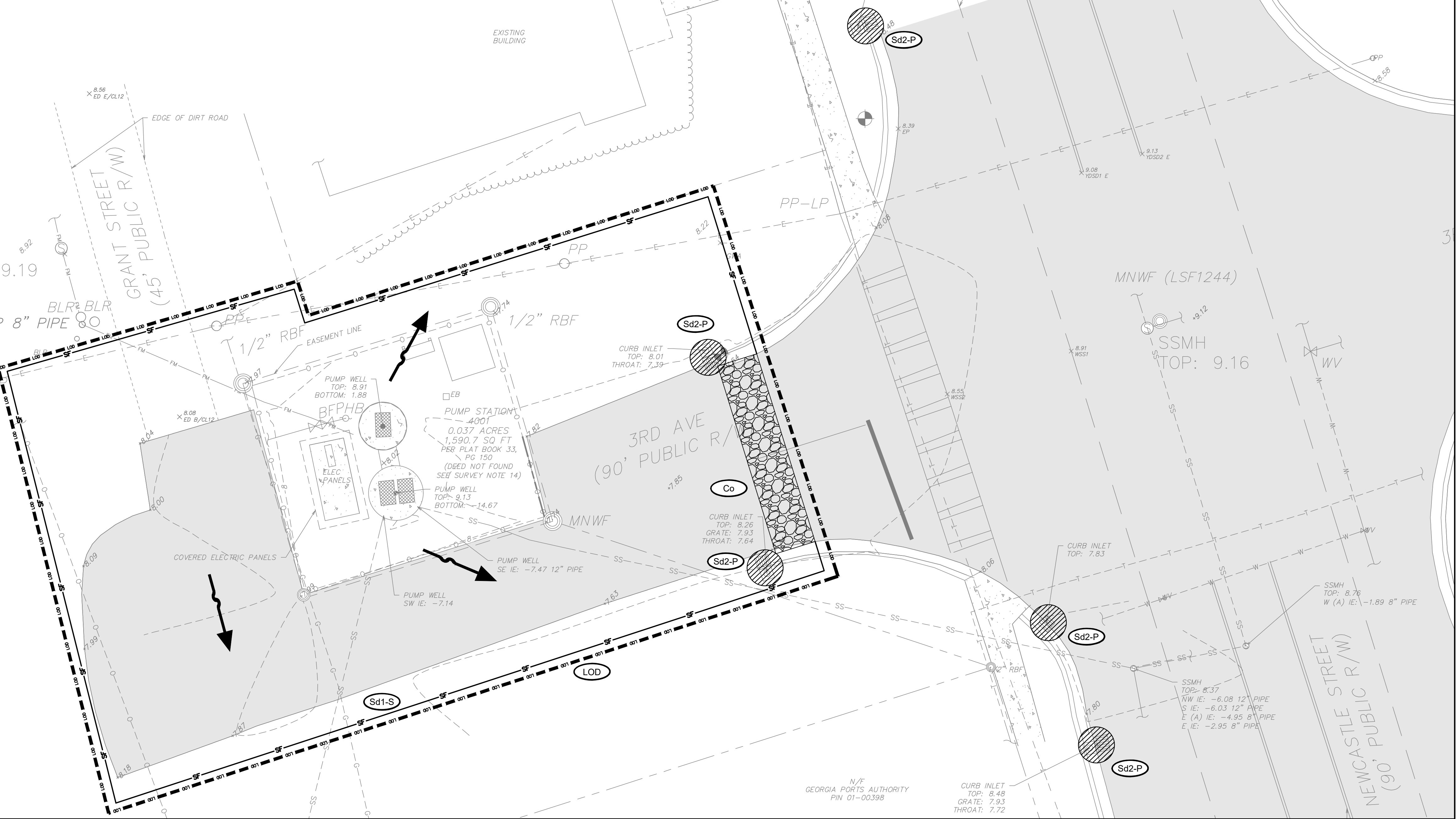
REVISIONS

DATE

BY

Plotted By: Eagle, Alex. Sheet: Set: BGJWSC. Layout: 302 PS4001 EROSION CONTROL & SEDIMENT PLAN. May 30, 2023, 11:39:57am. K:\JAX_WaterResources\045709000 - Brunswick Pump Station Upgrades\Re-Bld Development\CAD\4001\Pioneers\302 PS4001 EROSION CONTROL & SEDIMENT PLAN.dwg

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EROSION CONTROL NOTES:

- CONTRACTOR IS TO ADHERE TO THE GLYNN COUNTY STORMWATER MANAGEMENT MANUAL (GSM), COASTAL STORMWATER SUPPLEMENT (CSS), AND THE "MANUAL FOR EROSION AND SEDIMENT CONTROL IN GEORGIA" REQUIREMENTS.
- THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES PRIOR TO LAND DISTURBING ACTIVITIES.
- EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES. IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE FOR EFFECTIVE EROSION CONTROL, ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE.
- ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.
- ALL OPEN DRAINAGE SWALES SHALL BE GRASSED AND RIPRAP SHALL BE PLACED AS REQUIRED TO CONTROL EROSION.
- THE CONTRACTOR SHALL INSTALL AND MAINTAIN SILT BARRIERS AROUND ALL DRAINAGE STRUCTURES UNTIL ALL CONSTRUCTION HAS BEEN COMPLETED.
- SILT FENCES SHALL BE LOCATED ON SITE TO PREVENT SEDIMENT AND EROSION FROM LEAVING THE PROPERTY LIMITS.
- SILT FENCE SHALL MEET THE REQUIREMENTS OF SECTION 171-TEMPORARY SILT FENCE, OF THE GEORGIA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS, CURRENT EDITION AND BE WIRE REINFORCED.
- THERE ARE NO CURRENT APPARENT EROSION CONTROL PROBLEMS ON SITE. CONTRACTOR SHALL TAKE PROCEDURES AS NECESSARY TO PREVENT EROSION AND SEDIMENT TRANSPORT DURING CONSTRUCTION.
- MAXIMUM EMBANKMENT SLOPES ARE TO BE AS FOLLOWS: CUT AREAS - 2:1, FILL AREAS - 2:1.
- ALL SEDIMENT AND EROSION CONTROL MEASURES SHALL BE CLEANED AND MAINTAINED PER THE SPECIFICATIONS.
- ALL CUT AND FILL SLOPES MUST BE SURFACE ROUGHENED AND VEGETATED WITHIN SEVEN (7) DAYS OF THEIR CONSTRUCTION.
- THE CONTRACTOR SHALL MAINTAIN ALL EROSION CONTROL MEASURES UNTIL PERMANENT VEGETATION HAS BEEN ESTABLISHED. THE CONTRACTOR MUST CLEAN ALL SEDIMENT TRAPS AS REQUIRED BY THE OWNER'S REPRESENTATIVE OR LOCAL & STATE REQUIREMENTS.
- FAILURE TO INSTALL, OPERATE AND MAINTAIN ALL EROSION CONTROL MEASURES AS SHOWN ON THE APPROVED PLANS OR AS DIRECTED BY THE OWNER/PROJECT REPRESENTATIVE AND/OR LOCAL MUNICIPALITY AND STATE MAY RESULT IN ALL WORK ON THE CONSTRUCTION SITE BEING STOPPED UNTIL PROPER CORRECTIVE MEASURES HAVE BEEN MET, AS REQUIRED AND/OR DIRECTED.
- THE CONTRACTOR SHALL KEEP AND MAINTAIN ON SITE A LOG NOTING THE DATE OF ALL RAINFALL EVENTS, THE AMOUNT OF RAINFALL RECEIVED, DURATION OF RAINFALL EVENT, INSPECTION NOTES, AND ANY REPAIRS OR CLEANING OF EROSION CONTROL DEVICES.
- ALL SEDIMENT AND EROSION CONTROL DEVICES SHALL BE INSPECTED AT LEAST ONCE EVERY SEVEN (7) DAYS AND AFTER ANY RAINFALL EVENT.

EROSION CONTROL NOTES (CONT.):

- ALL SLOPES 2:1 AND SEDIMENT BASINS SHOULD BE STABILIZED WITH SYNTHETIC OR VEGETATIVE MATS NORTH AMERICAN GREEN SC150 OR EQUIVALENT, IN ADDITION TO GRASSING/HYDROSEEDING.
- SEE DETAILS FOR D62 AND D63 SEEDING AND MULCHING REQUIREMENTS.
- STORM DRAINAGE SYSTEM SHALL BE INSTALLED AS SOON AS POSSIBLE DURING THE CONSTRUCTION PROCESS, AND ALL RUNOFF SHALL BE DIRECTED TO THE DRAINAGE SYSTEM.
- PRIOR TO STORM DRAINAGE SYSTEM INSTALLATION ALL RUNOFF LEAVING THE SITE SHALL BE FILTERED THROUGH SILT FENCES AND FILTERS PRIOR TO DISCHARGE OFFSITE. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THIS REQUIREMENT ONSITE DURING ALL PHASES OF CONSTRUCTION.
- THIS PROJECT LIES WITHIN FLOOD ZONE AE AS DEFINED BY THE F.E.M.A. "FLOOD INSURANCE RATE MAP" NUMBER 13127C0238H, DATED 01/05/2018.
- WETLANDS DO NOT NOT EXIST ON SITE.
- SITE DOES NOT LIE WITHIN 200 FEET OF A JURISDICTIONAL STREAM.
- WASTE MATERIALS SHALL NOT BE DISCHARGED TO WATERS OF THE STATE EXCEPT AS AUTHORIZED BY A SECTION 404 PERMIT.
- AMENDMENTS/REVISIONS TO THE ES&PC PLAN WHICH HAVE A SIGNIFICANT EFFECT ON BMPs WITH A HYDRAULIC COMPONENT MUST BE CERTIFIED BY THE DESIGN PROFESSIONAL.
- ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES WILL BE INSTALLED IN DEEMED NECESSARY BY ON-SITE INSPECTION.
- THE RECEIVING WATER FOR THE PROJECT IS AN UNNAMED TRIBUTARY OF ACADEMY CREEK. THE PROJECT IS NOT WITHIN ONE LINEAR MILE FROM AN IMPAIRED STREAM SEGMENT.
- NON-EXEMPT ACTIVITIES SHALL NOT BE CONDUCTED WITHIN THE 25 OR 50-FOOT UNDISTURBED STREAM BUFFERS AS MEASURED FROM THE POINT OF WRESTED VEGETATION OR WITHIN 25 FEET OF THE COASTAL MARSHLAND BUFFER AS MEASURED FROM THE JURISDICTIONAL DETERMINATION LINE WITHOUT FIRST ACQUIRING THE NECESSARY VARIANCES AND PERMITS.
- EROSION AND SEDIMENT CONTROL MEASURES AND PRACTICES TO BE INSPECTED DAILY.
- WEEKLY EROSION AND SEDIMENT CONTROL REPORTS SHALL BE SUBMITTED TO THE DEVELOPMENT DEPARTMENT STARTING WITH THE ISSUANCE OF THE DEVELOPMENT PERMIT AND ENDING WHEN THE PROJECT IS RELEASED BY THE INSPECTOR.
- INSPECTIONS BY QUALIFIED PERSONNEL PROVIDED BY THE PRIMARY PERMITTEE AND THE ASSOCIATED RECORDS SHALL BE KEPT ON SITE IN COMPLIANCE WITH GAR 100001.
- NO ALTERNATE BMPs WERE USED IN THIS PROJECT.
- SEE SHEETS 501 AND 502 FOR EROSION CONTROL DETAILS.

EROSION CONTROL LEGEND:

- | | | |
|-------|--|-----|
| Sd1-S | SILT FENCE - TYPE S | SF |
| Co | CONSTRUCTION EXIT | |
| Sd2-P | INLET SEDIMENT TRAP: CURB INLET PROTECTION | |
| Du | DUST CONTROL | |
| Ds1 | DISTURBED AREA STABILIZATION | |
| Ds2 | DISTURBED AREA STABILIZATION | |
| LOD | LIMITS OF DISTURBANCE | --- |
| Cd-Hb | STRAW - BALE CHECK DAM | |
| Fr | FILTER RING | |

SITE AREA SUMMARY:
TOTAL SITE AREA = 0.037 ACRES
TOTAL DISTURBED AREA = 0.2 ACRES

24-HOUR CONTACT:
NAME: TODD KLINE
COMPANY: BGJWSC
NUMBER: 912-261-7122

BID DOCUMENTS



GEORGIA811.
Utilities Protection Center, Inc.
Know what's below.
Call before you dig.

PS4001 EROSION
CONTROL &
SEDIMENT PLAN

PUMP STATION
UPGRADES PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER
302

LICENSED PROFESSIONAL
JOSHUA S. PETERSON, P.E.
GEORGIA LICENSE NUMBER
036753

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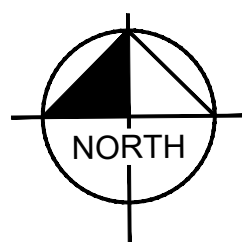
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CHECKED BY: KES
DESIGNED BY: JSP
DRAWN BY: ACE
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DATE: MAY 2023
KHA PROJECT: 045709000

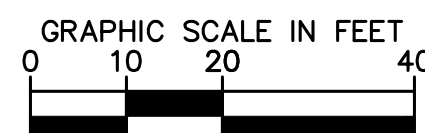
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No.

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BY



PS4001 ELECTRICAL DEMOLITION SITE PLAN

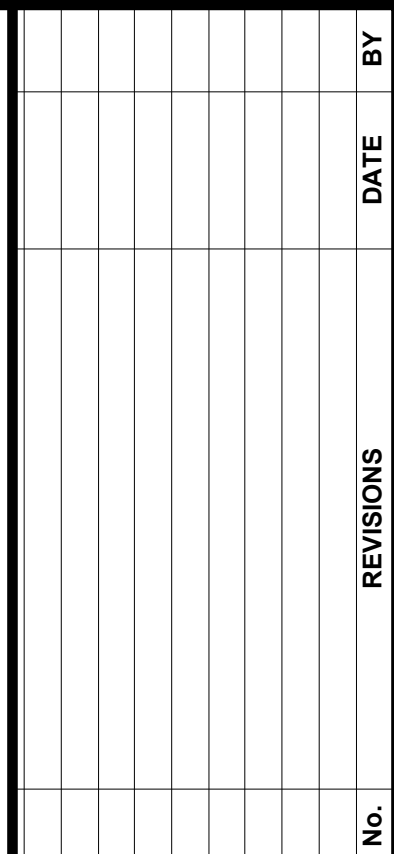
SCALE: 1" = 10'



GEORGIA811
Utilities Protection Center, Inc.

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PHONE: 904-828-3900
WWW.KIMLEY-HORN.COM REGISTRY NO. 696

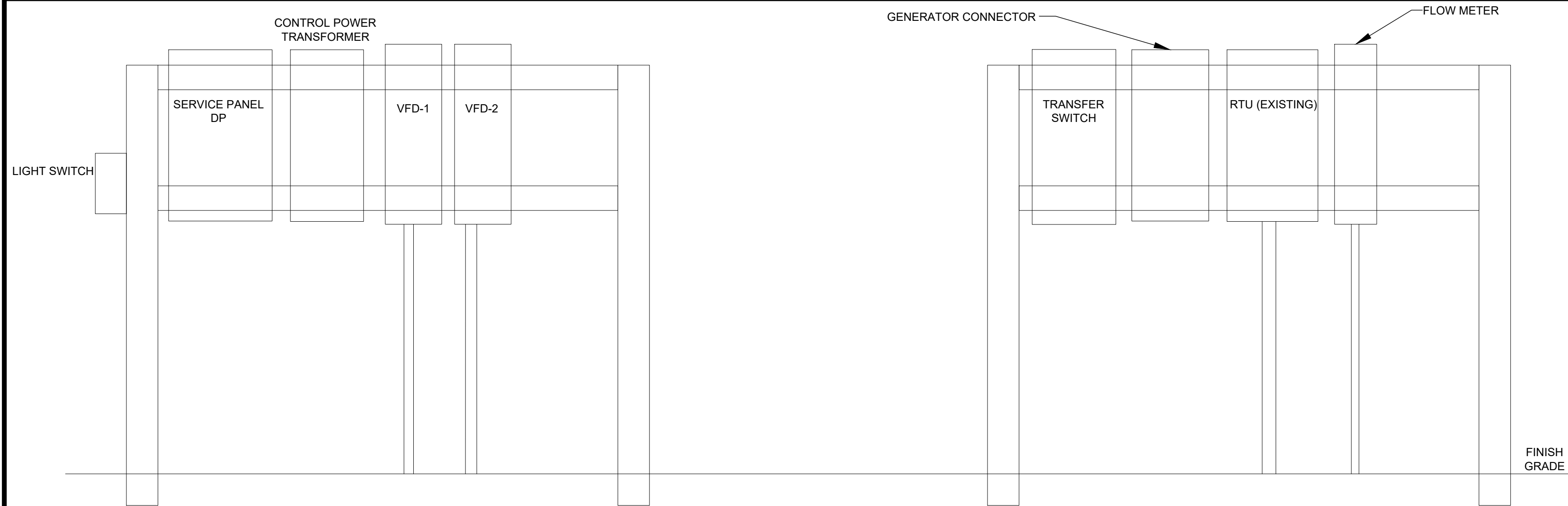
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PS4001
ELECTRICAL
DEMOLITION PLAN

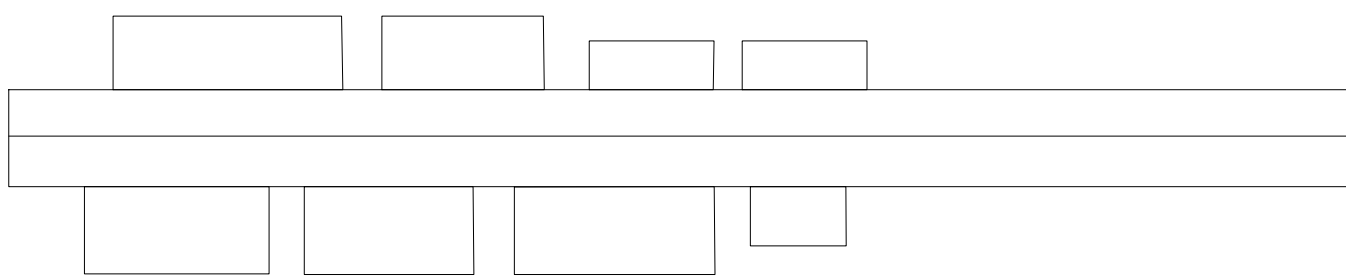
ENGINEERING SERVICES FOR
PUMP STATION UPGRADES FOR
PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER
306



EQUIPMENT RACK - FRONT ELEVATION

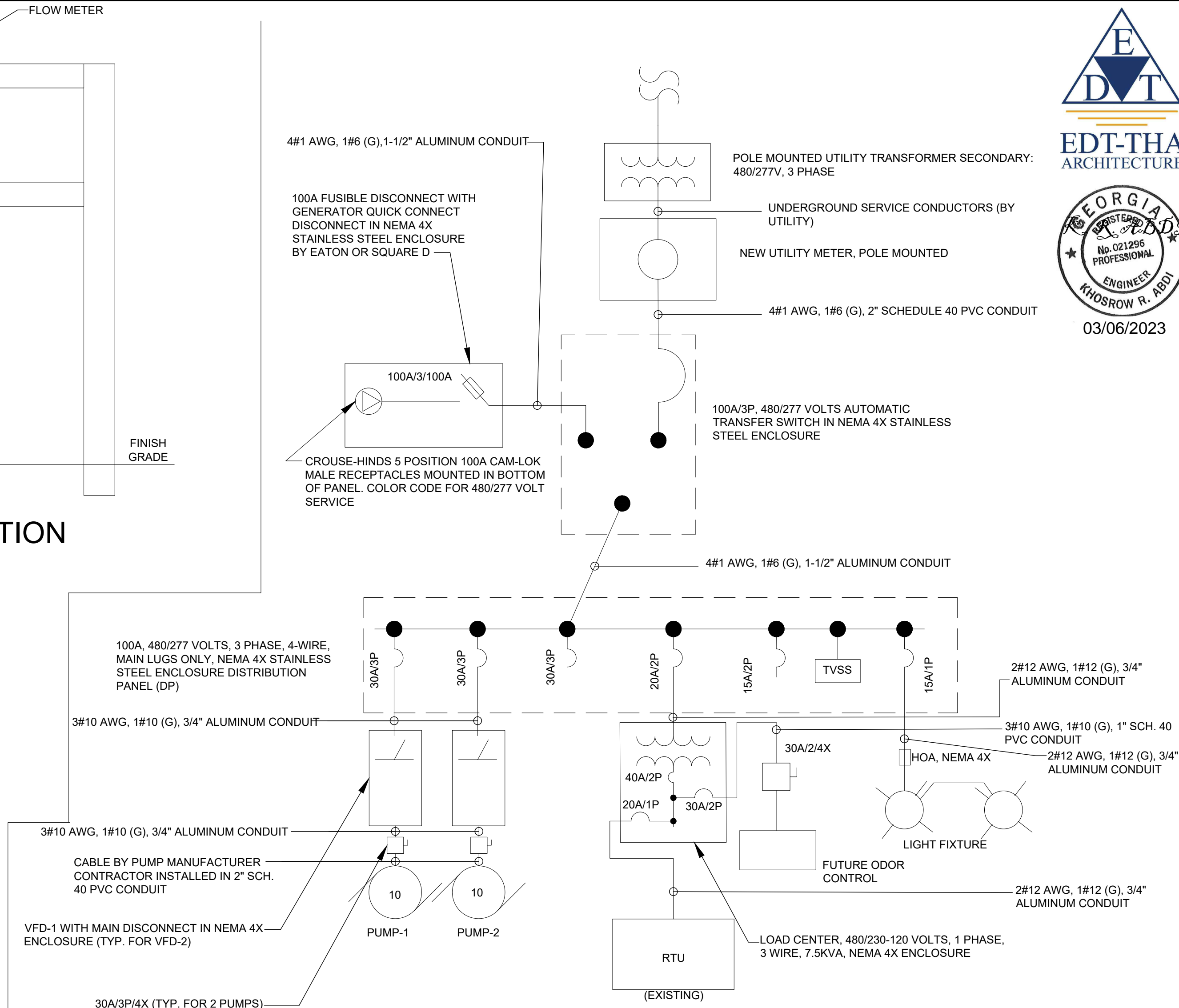
EQUIPMENT RACK - BACK ELEVATION



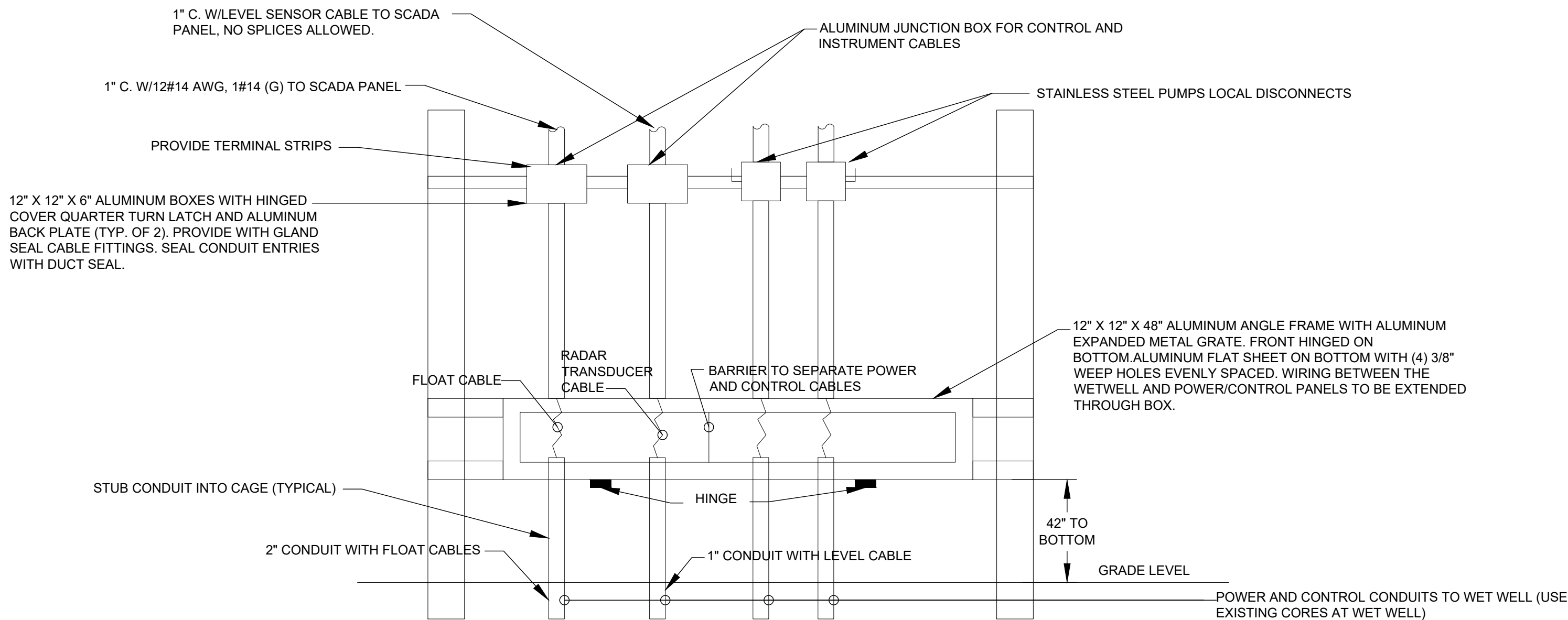
EQUIPMENT RACK - TOP PLAN

1 PUMP STATION PS4001 - ELECTRICAL EQUIPMENT RACK ELEVATION
308 SCALE: N.T.S.

NOTES:
EXISTING EQUIPMENT RACK TO BE REMOVED AND RELOCATED WHERE SHOWN ON CIVIL SITE PLAN.



2 PUMP STATION PS4001 - SINGLE LINE DIAGRAM
308 SCALE: N.T.S.



PUMP STATION PS4001 - RACK MOUNTED AIR BREAK BOX FOR LOCAL PUMPS DISCONNECTS AND JUNCTION BOXES FOR FLOAT SWITCHES AND RADAR LEVEL TRANSDUCER

3
308 SCALE: N.T.S.



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LICENSED PROFESSIONAL:

KHOSROW ABDI, P.E.

GEORGIA LICENSE NUMBER

021296

DATE:

KHA PROJECT

045709000

DATE

MARCH 2023

SCALE: AS SHOWN

DESIGNED BY: KA

DRAWN BY: TF

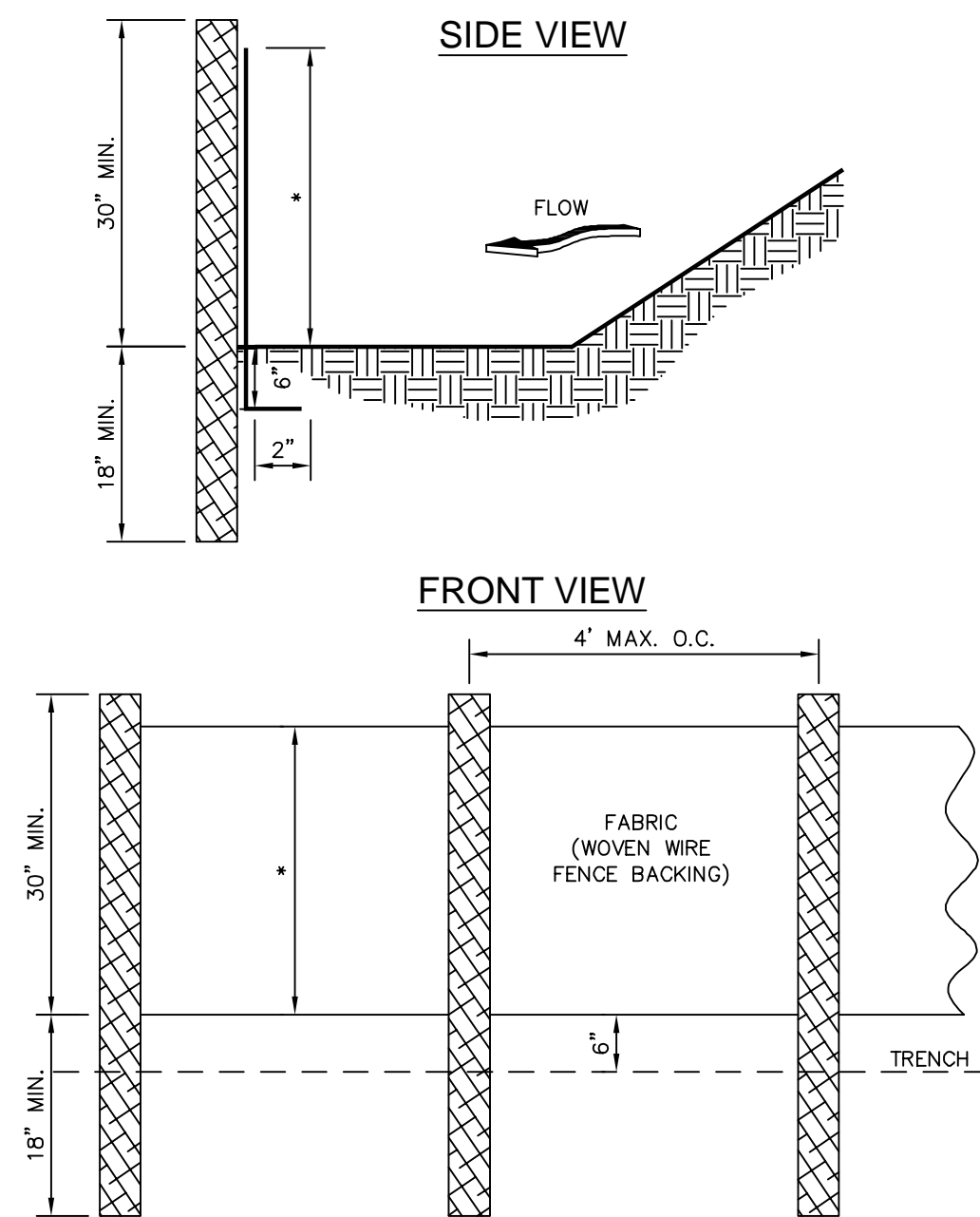
CHECKED BY: KA

PS4001
ELECTRICAL
DIAGRAMS AND
DETAILS

ENGINEERING SERVICES FOR
PUMP STATION UPGRADES FOR
PS4001
PREPARED FOR
BGJWSC

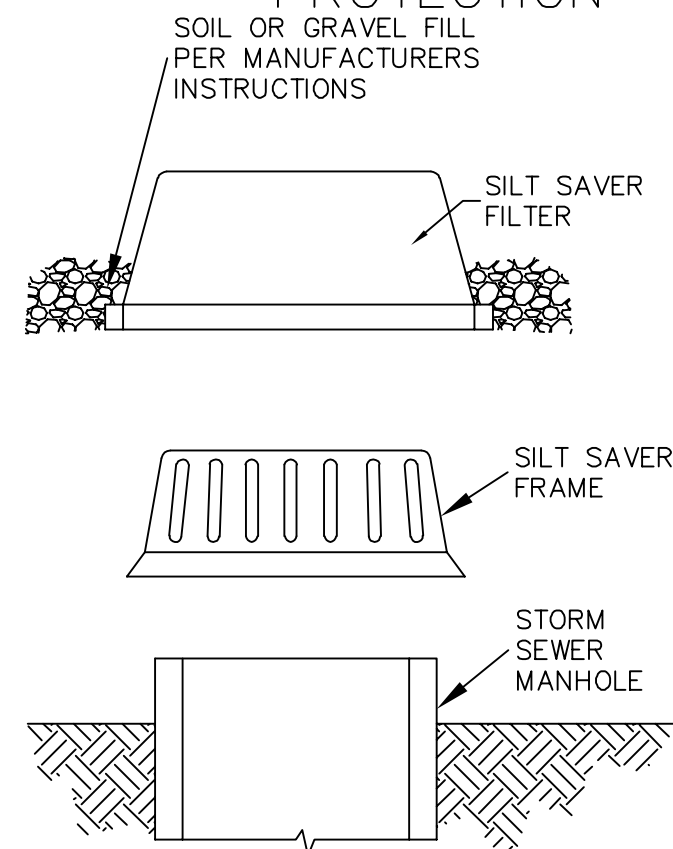
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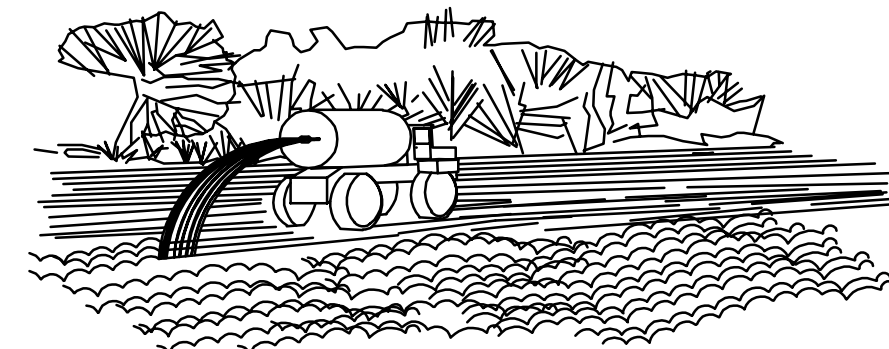
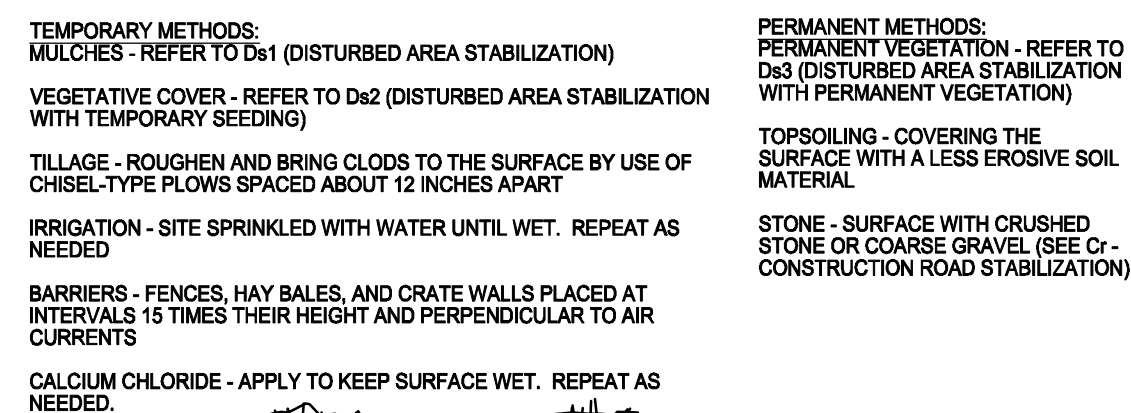
- NOTES:**
1. USE STEEL OR WOOD POSTS OR AS SPECIFIED BY THE EROSION, SEDIMENTATION, AND POLLUTION CONTROL PLAN.
 2. HEIGHT (*) IS TO BE SHOWN ON THE EROSION, SEDIMENTATION, AND POLLUTION CONTROL PLAN.

SILT FENCE - TYPE SENSITIVE (Sd1-S)



1. EXCAVATE APPROXIMATELY 4" TO 6" BELOW THE TOP OF THE INLET STRUCTURE.
2. PLACE THE FRAME ONTO THE INLET STRUCTURE, ENSURING PROPER SEATING OF FRAME TO STRUCTURE.
3. SLIDE THE FILTER OVER THE FRAME.
4. FILL THE FILTER POCKETS WITH SOIL, #57 GRAVEL OR EQUIVALENT. THE FILTERPOCKETS SHOULD BE COMPLETELY FILLED TO ENSURE A GOOD SEAL BETWEEN THE GROUND AND INLET STRUCTURE.
5. BACKFILL AROUND THE FRAME AND FILTER ASSEMBLY IS NOT REQUIRED TO COMPLETE INSTALLATION; HOWEVER, BACKFILLING MAY BE NECESSARY TO COMPLETE EXCAVATION REQUIREMENTS FOR THE SITE.
6. THE TRAP SHALL BE INSPECTED DAILY AND AFTER EACH RAIN AND REPAIRS MADE AS NEEDED. SEDIMENT SHALL BE REMOVED WHEN THE SEDIMENT HAS ACCUMULATED TO ONE-HALF THE HEIGHT OF THE TRAP.

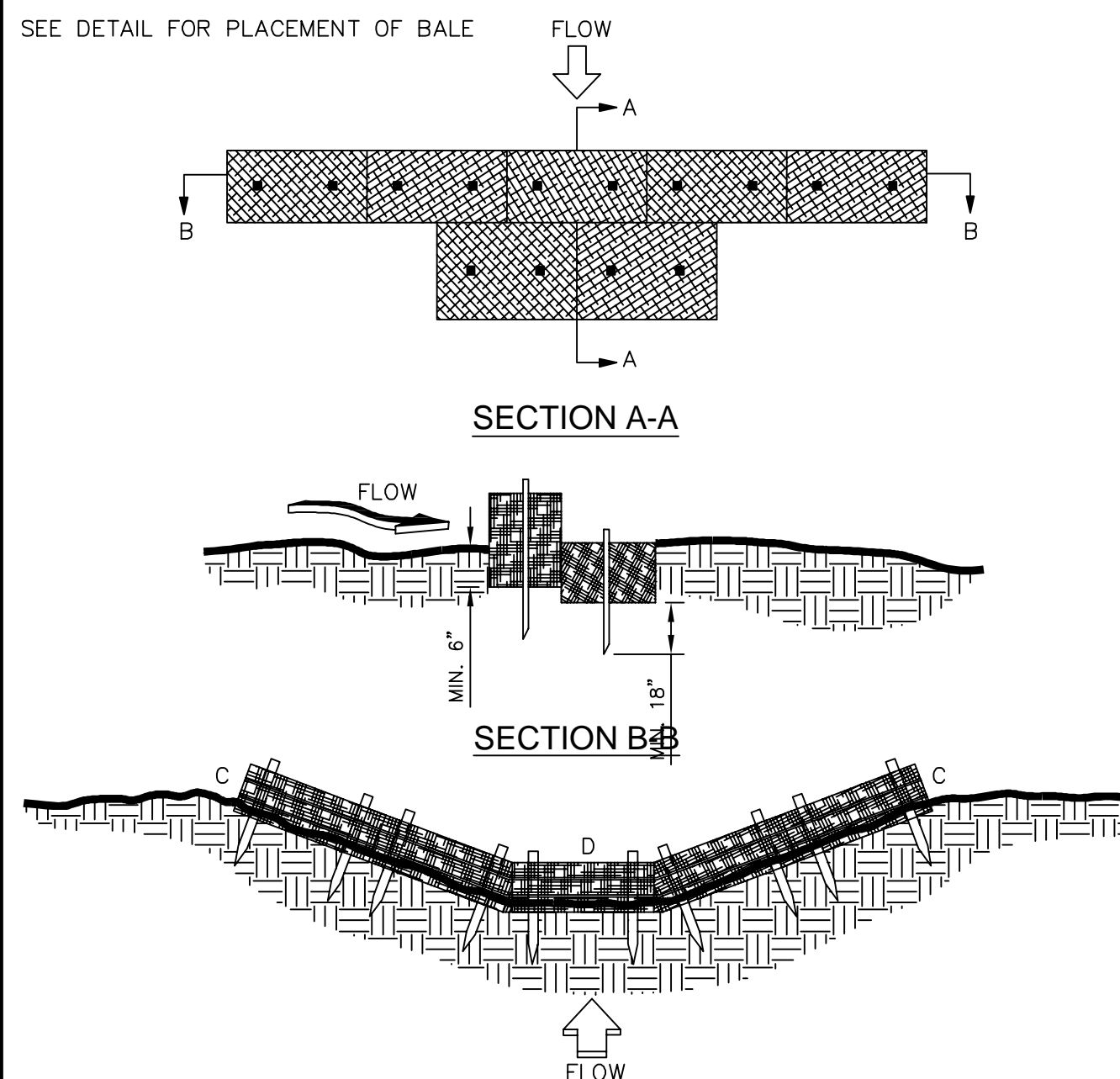
INLET SEDIMENT TRAP - SILT SAVER (Sd2-SS)



DUST CONTROL ON DISTURBED AREAS (Du)

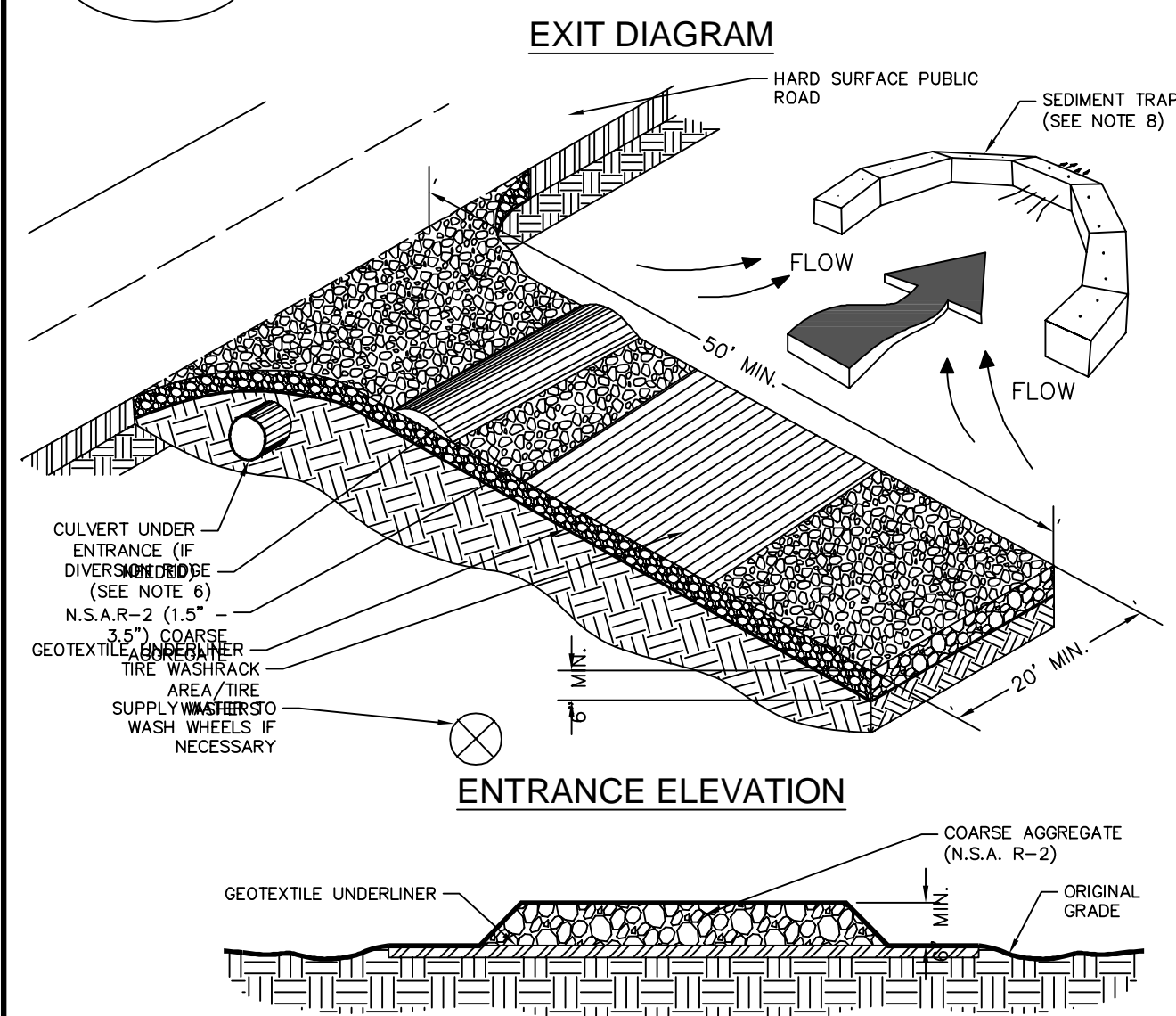


SEE DETAIL FOR PLACEMENT OF BALE



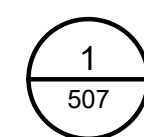
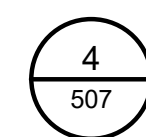
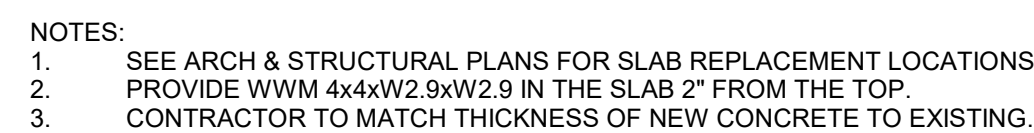
- NOTES:
1. BALES SHOULD BE BOUND TIGHTLY WITH WIRE OR NYLON STRING AND SHOULD BE PLACED IN ROWS WITH BALE ENDS TIGHTLY ABUTTING THE ADJACENT BALES.
 2. REMOVE #4 REBAR AFTER STRAW BALES ARE NO LONGER IN PLACE.
 3. POINT C OF SECTION B-B SHOULD ALWAYS BE HIGHER THAN POINT D.

CHECK DAM: STRAW-BALE (Cd-Hb)



- NOTES:**
1. AVOID LOCATING ON STEEP SLOPES OR AT CURVES ON PUBLIC ROADS.
 2. REMOVE ALL VEGETATION AND OTHER UNSUITABLE MATERIAL FROM THE FOUNDATION AREA, GRADE, AND CROWN FOR POSITIVE DRAINAGE.
 3. AGGREGATE SIZE SHALL BE IN ACCORDANCE WITH NATIONAL STONE ASSOCIATION R-2 (1.5"-3.5" STONE).
 4. GRAVEL PAD SHALL HAVE A MINIMUM THICKNESS OF 6".
 5. PAD WIDTH SHALL BE EQUAL FULL WIDTH AT ALL POINTS OF VEHICULAR EGRESS, BUT NO LESS THAN 20".
 6. A DIVERSION RIDGE SHOULD BE CONSTRUCTED WHEN GRADE TOWARD PAVED AREA IS GREATER THAN 2%.
 7. INSTALL PIPE UNDER THE ENTRANCE IF NEEDED TO MAINTAIN DRAINAGE DITCHES.
 8. WHEN WASHING IS REQUIRED, IT SHOULD BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN (DIVERT ALL SURFACE RUNOFF AND DRAINAGE FROM THE ENTRANCE TO A SEDIMENT CONTROL DEVICE).
 9. WASHRACKS AND/OR TIRE WASHERS MAY BE REQUIRED DEPENDING ON SCALE AND CIRCUMSTANCE. IF NECESSARY, WASHRACK DESIGN MAY CONSIST OF ANY MATERIAL SUITABLE FOR TRUCK TRAFFIC THAT REMOVE MUD AND DIRT.
 10. MAINTAIN AREA IN A WAY THAT PREVENTS TRACKING AND/OR FLOW OF MUD ONTO PUBLIC RIGHTS-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.

CONSTRUCTION EXIT (Co)


$$3/4'' = 1'-0''$$

$$\frac{3}{4}'' = 1'-0''$$


03/06/2023

[illegible]

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KHA PROJECT
04570000MARCH 2023
SALE: AS SHOWN

SIGNED BY: BO

AWN BY: A-

MISC. TYP.
STRUCTURAL
DETAILS AND
SECTIONS

ENGINEERING SERVICES FOR
PUMP STATION UPGRADES FOR
PS4001
PREPARED FOR
BGJWSC

SHEET NUMBER

507