

# Brunswick-Glynn County North Mainland Water Loops Brunswick, Georgia

January 11, 2022 Terracon Project No. ES215271

### **Prepared for:**

Four Waters Engineering, Inc. Greenville, South Carolina

## Prepared by:

Terracon Consultants, Inc. Savannah, Georgia

Environmental Facilities Geotechnical Materials

January 11, 2022

Four Waters Engineering, Inc. 150 Milestone Way Greenville, South Carolina 29615



Attn: Mr. Dwaine R. Falls

P: (864) 569 6145 E: dfalls@4weng.com

Re: Geotechnical Engineering Report

**Brunswick-Glynn County North Mainland Water Loops** 

Brunswick, Georgia

Terracon Project No. ES215271

Dear Mr. Falls:

We have completed the Geotechnical Engineering services for the above referenced project. This study was performed in general accordance with Terracon Proposal No. PES215271, dated September 28, 2021. This report presents the findings of the subsurface exploration and provides geotechnical recommendations concerning the earthwork for the proposed project.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely,

**Terracon Consultants, Inc.** 

Matthew Benis

Matthew L. Bemis, E.I.T.

Staff Geotechnical Engineer

CEORGIAN THOMES AND THOMES SHOWN AND THE SHOWN

Guoming Lin, Ph.D., P.E., D.GE Senior Consultant



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## **REPORT SUMMARY**

Topic	Overview Statement		
Proposed Construction	The project consists of installing three water main loops at three different locations around Brunswick, Georgia.		
	The hand auger logs show approximately 7 inches of topsoil. The topsoil thickness may be deeper than 7 inches below the ground surface (BGS) in some areas.		
	<ul> <li>In general, the subsurface conditions are consistent throughout the project site.</li> </ul>		
Geotechnical Characterization	The site at Old Jesup Road consists of silty sand to depths of approximately 2 to 6 feet BGS, followed by clayey sand to depths of approximately 6 to 10 feet BGS. Below the clayey sand layer is silty sand to approximately 25 feet BGS. Hautala Drive and Perry Lane Road largely contains silty sands to depths of approximately 25 feet BGS. Please refer to the Geotechnical Characterization section for details.		
	<ul> <li>Depth to groundwater was measured at 3 to 7 feet BGS in the SPT and hand auger borings.</li> </ul>		
	Install a site drainage system.		
Earthwork	<ul> <li>Strip/grub topsoil when encountered Level and densify subgrade during subgrade preparation. If any soft/weak areas are detected repair subgrade by undercut and backfill.</li> </ul>		
Seismic Considerations	For seismic design purposes, the subject site is classified as <b>Site Class D</b> in accordance with the International Building Code (IBC) 2018 and ASCE 7-16 Section 11.4.2.		
General	This section contains important information about the limitations of this		
Comments	geotechnical engineering report.		



## **Brunswick-Glynn County North Mainland Water Loops**

Brunswick, Georgia Terracon Project No. ES215271 January 11, 2022

#### INTRODUCTION

This report presents the results of our subsurface exploration and geotechnical engineering services performed for the proposed utility installation to be located off Old Jesup Road, Hautala Drive, and Perry Lane Road in Brunswick, Georgia. The purpose of these services is to provide information and geotechnical engineering recommendations relative to:

- Subsurface soil conditions
- Groundwater conditions

- Site preparation and the earthwork
- Seismic site classification per IBC

The geotechnical engineering Scope of Services for this project included the advancement of 9 Standard Penetration Test (SPT) borings to a maximum depth of 25 feet below ground surface (BGS) and 18 Hand Auger (HA) borings to depths of approximately 5 feet BGS. Maps showing the site and boring locations are shown in **Exhibit A**.

#### SITE CONDITIONS

The following description of site conditions is derived from our site visit in association with the field exploration and our review of publicly available geologic and topographic maps.

Item	Description		
Parcel Information	The project consists of three (3) different site locations located in Brunswick, Georgia:  3,000 feet section of Old Jesup Rd and 1,100 feet section of Bailey Rd Latitude: 31.2366°, Longitude: -81.5291°  1,100 feet section of Hautala Dr Latitude: 31.2299°, Longitude: -81.5165°  500 feet section of Perry Lane Road Latitude: 31.2461°, Longitude: -81.5048°  See Site Location Plan in Exhibit A-1		
<b>Existing Improvements</b>	The site locations include asphalt paved roads.		
<b>Current Ground Cover</b>	Asphalt pavement with grassy shoulders.		
<b>Existing Topography</b>	Relatively variable with a maximum elevation change of 2 to 3 feet.		

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#### PROJECT DESCRIPTION

Our initial understanding of the project was provided in our proposal and was discussed in the project planning stage. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

ltem	Description	
Information Provided	A request for proposal and map exhibits created by Brunswick-Glynn County Joint Water and Sewer Commission was provided by Dwaine Falls of Four Waters Engineering via email on September 21, 2021.	
Proposed Structure	The project involves the design and installation of three water main loops to provide more reliable and dependable water access.	

#### **GEOTECHNICAL CHARACTERIZATION**

#### **Subsurface Profile**

We have developed a general characterization of the subsurface soil and groundwater conditions based upon our review of the data and our understanding of the geologic setting and planned construction. The following table provides our geotechnical characterization.

The geotechnical characterization forms the basis of our geotechnical calculations and evaluation of site preparation and earthwork recommendations. As noted in **General Comments**, the characterization is based upon widely spaced exploration points across the site, and variations are likely.

#### **Old Jesup Road**

Stratum	Approximate Depth to Bottom of Stratum (feet) Below Ground Surface	Material Description Based on CPT Soundings and Hand Auger Borings Relative Densit	
Surface	0.6 <sup>1</sup>	Topsoil -	
1	2 to 6	Sand/Silty sands	Very loose to loose
2	6 to 10	Clayey sands	Loose
3	25	Silty sand	Loose to medium dense

#### Note:

1. The thickness of the topsoil at the project site may reach deeper than 7 inches. The depth/thickness of topsoil will vary, depending upon the near-surface soil disturbance during the site preparation.

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#### **Hautala Drive**

Stratum	Approximate Depth to Bottom of Stratum (feet) Below Ground Surface	Material Description Based on CPT Soundings and Hand Auger Borings	Consistency/ Relative Density
Surface	0.6 <sup>1</sup>	Topsoil	-
1	25	Silty sands	Loose to very dense

#### Note:

2. The thickness of the topsoil at the project site may reach deeper than 7 inches. The depth/thickness of topsoil will vary, depending upon the near-surface soil disturbance during the site preparation.

#### **Perry Lane Road**

Stratum	Approximate Depth to Bottom of Stratum (feet) Below Ground Surface	Material Description Based on CPT Soundings and Hand Auger Borings	Consistency/ Relative Density
Surface	0.6 <sup>1</sup>	Topsoil	-
1	25	Silty sands	Loose to very dense

#### Note:

The thickness of the topsoil at the project site may reach deeper than 7 inches. The depth/thickness of topsoil will vary, depending upon the near-surface soil disturbance during the site preparation.

Conditions encountered at each exploration location are indicated on the individual exploration logs shown in **Exhibit B-2** attached to this report. Stratification boundaries on the logs represent the approximate location of changes in native soil types; in situ, **the transition between materials may be gradual**.

#### **Groundwater Conditions**

The SPT and hand auger borings were observed while drilling and after completion for the presence and level of groundwater. Groundwater was measured in eight SPT borings and eleven hand auger borings at depths of approximately 3 to 7 feet BGS. The water levels observed in the borings can be found on the SPT and hand auger logs in **Exhibit B-2** and **Exhibit B-3** respectively. The measured and observed water levels are summarized in the following table:

Boring Number	Measured Groundwater Depth (ft)
B01	6.00
B02	7.00
B03	4.50
B04	4.00

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Boring Number	Measured Groundwater Depth (ft)
B05	3.25
B06	3.00
B07	4.00
B08	4.00
HA01	4.00
HA04	3.00
HA05	5.00
HA07	4.00
HA08	5.00
HA09	5.00
HA13	4.00
HA14	5.00
HA16	5.00
HA17	4.00
HA18	5.00

Groundwater level fluctuations occur due to seasonal variations in the amount of rainfall, runoff, and other factors not evident at the time the borings were performed. Therefore, groundwater levels during construction or at other times in the life of the structure may be higher or lower than the levels indicated on the boring logs.

Mottling, as a strong indicator of water seepage during seasonal high groundwater levels, was noted on hand auger boring locations ranging from 0.8 to 2 feet BGS.

#### **Laboratory Testing**

Laboratory testing procedures were performed on soil samples collected from throughout the site. Bag samples were obtained at multiple depths ranging from 2 to 20 feet below the existing grade surface and shipped to Terracon's laboratory for the following testing procedures:

Moisture Content: ASTM D2216 - Standard Test Methods for Laboratory Determination

of Water Content of Soil and Rock by Mass

Grain Size Analysis: ASTM D422 - Standard Test Method for Particle Size Distribution of

Soils

Atterberg Limits: ASTM D4318 - Standard Test Method for Liquid Limit, Plastic Limit,

and Plasticity Index of Soils

Our laboratory testing results are represented in individual graphs and tables in more detail in **Exhibit B-4** through **Exhibit B-6**.

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**Natural Moisture Content:** The natural moisture contents of the samples ranged from 6.7 percent to 33.0 percent. Based on our experience, we believe the subgrade soils with moisture content above 20 percent may not be stable judged by proofrolling. Therefore, the soils may need to be dried by scarifying or discing. Apparently, the effectiveness of the soil drying will be largely affected by the weather. The months from May to October typically have more favorable weather due to the drying effect from the sun.

**Fine Content:** The soil samples from the shallow depth of upper 10 feet have a percentage of fine passing the No.200 sieve between 7.2 percent and 45.9 percent. The specifications stipulate fines percentage less than 25 percent as structural fills. The majority of the tested samples had less than 25% fines but some are classified as clayey sand. Clayey soils tend to hold to moisture and become difficult to dry after they become wet. These materials are marginally suitable for fill but will require more effort to dry out prior to placement.

**Soil Plasticity:** Soils with liquid limits greater than 50 are classified as high plasticity clays or silts. The soils with the above plasticity may be difficult to work and have the potential to shrink and swell with the change of moisture. As such, these clayey soils are generally not suitable for structural fill.

#### RECOMMENDATIONS FOR DESIGN AND CONSTRUCTION

The following evaluation and recommendations are based upon our understanding of the proposed construction and the results from our field exploration. If the above-described project conditions are incorrect or changed after this report, or subsurface conditions encountered during construction are significantly different from those reported, Terracon should be notified so we can re-evaluate our recommendations and make appropriate revisions.

#### **Geotechnical Considerations**

The subsurface conditions at this site are adaptable for the proposed construction. The generalized soil profile is presented in the **Geotechnical Characterization** section.

Based on the information available, we understand the proposed water main will be constructed using either the open trench or tunneling method. The depths of the trench or tunneling have not been determined at this time. The soils at the site are mostly very loose sands. The open-cut excavations for the water main and the entry and exit pits for directional drilling may require protective measures. The trench and drill pits can be made using open slopes or vertical cuts supported with sheet piles or other temporary retaining structures. Soldier piles with timber lagging shoring may be used in the construction of the trench supporting system. Trench boxes are commonly used for trench safety in open-cut excavations with vertical walls. The site has relatively shallow groundwater table. Dewatering or other forms of groundwater management may be required depending on the depth of excavation.

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It is anticipated that excavations for the proposed construction can be accomplished with a conventional large excavator. The individual contractor(s) is responsible for designing and constructing stable, temporary excavations as required to maintain the stability of both the excavation sides and bottom. Excavations should be sloped or shored following local, and federal regulations, including current OSHA excavation and trench safety standards.

The selection of the appropriate method for directional drilling is the contractor's responsibility. The contractor should select the appropriate boring machine and excavation method for directional drilling based on the subsurface soils and groundwater conditions indicated in the soil boring logs.

Stockpiling of excavated material in proximity to the excavation is not recommended. In general, a distance of half the excavation depth on both sides of the trench should be kept clear of any excavated materials. If this is not possible due to the space limitations, the retaining wall design should take into consideration the surcharge loads from the excavated materials. This is an important consideration. A major slope failure occurred about ten years ago during a Pipemakers Canal improvement project as the excavated material was stockpiled along the canal bank.

Care should be taken during excavations as there is the possibility that sloughing or caving of the excavation trench or excavation slope may cause movement of the surrounding soils leading to a possible settlement of the neighboring structures or features.

<u>Monitoring</u> Despite our best efforts for the thorough geotechnical exploration, the actual subsurface conditions may vary from the anticipated conditions because the subsurface exploration records provided in **Appendix A** represent an interpretation of subsurface conditions at the boring locations and the subsurface conditions between the test locations may vary.

During excavation and pipeline installation, ground movements like settlement and lateral movement may occur and should be monitored and controlled. The monitoring program should include measurements of the groundwater table, ground vibration, lateral ground movements outside excavation, and monitoring of existing cracks at selected locations on the neighboring structures. Terracon can develop a more detailed plan for condition survey and monitoring as construction plans are developed.

We recommend Terracon should be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during subgrade preparation; placement and compaction of controlled compacted fill; backfilling of excavations to the completed subgrade.

#### **EARTHWORK**

Site preparation should include the installation of a site drainage system, topsoil stripping and grubbing, subgrade preparation, and densification. **Please bear in mind,** due to the uneven

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ground surface of the site, the volume of topsoil and organics may be significantly greater than the area times the topsoil/organics thickness indicated in the boring logs. Rutting of the subgrade can also cause the mixing of topsoil/organics with underlying soils, which will result in additional required topsoil/organics stripping. Deeper undercuts may be needed in some localized areas to remove unsuitable materials.

#### **Site Drainage**

An effective drainage system should be installed prior to the initiation of site preparation and grading activities to intercept surface water and to improve overall shallow drainage. The drainage system may consist of perimeter ditches supplemented with parallel ditches and swales. Pumping equipment should be used if the above ditch system cannot effectively drain water away from the site, especially during the rainy season. The site should be graded to shed water and avoid ponding over the subgrade.

#### **Bore Pit Excavation**

At this time, the extent and depth of the excavation for the entrance and exit pits has not been made available. Based on the soil borings performed to approximately 30 feet BGS, the soils encountered during the excavation will most likely be loose to dense silty/clayey sands. These soils are sensitive to moisture and erosion during construction. The contractor should provide methods to control site drainage and provide erosion control of the excavated slope face.

Depending upon the depth of excavation, dewatering should be planned for deep excavation. Groundwater depths vary significantly along the project length from 3 to 7 feet BGS due to the variation in site topography.

To support the excavation and dewatering activities, a temporary sheet pile wall or similar earth retaining structure should be constructed unless there is a space for a sloped excavation. Shoring may be required to support the temporary retaining structure in order to prevent slope sliding or collapse. If open-pit excavation methods are used for the construction of the entrance and exit pits, a slope inclination of 2 horizontal to 1 vertical or flatter is recommended for slope height less than seven feet due to the nature of the surface soils. A more detailed slope stability analysis should be performed for a slope higher than 7 feet based on the soil conditions and slope configurations. In all cases, excavations should conform to OSHA guidelines.

If the temporary retaining structures are required instead of the sloped open excavation, the temporary retaining walls should be designed for earth pressures equal to those provided in **Section 4.3**.

**Please note**: as the proposed water main is located underneath roadways, the contractor should take necessary precautions to avoid damages to the existing railways and roadways and other structures in the vicinity of the project area.

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#### **Pipe Bedding and Excavations**

Care should be taken so that the soils at the base of excavations are not disturbed during construction. Disturbed or unstable materials should be removed before placing any granular bedding material. Where groundwater, lower strength soils, and unstable conditions are encountered, a greater thickness of bedding material should be provided. The minimum thickness of the bedding material should be 12 inches.

Groundwater varies along the project length. Please refer to the individual boring logs for the groundwater depth in each boring location. Depending upon the location, dewatering of the pipeline trench should be expected. We recommend the pipe excavation to be shored with trench boxes or other means to control erosion of the saturated sands into the trench during construction. Sloped excavation could be used for the pipeline trenching however the groundwater should be lowered to a minimum of 2 feet below the bottom of the excavation and the excavation side slopes should be 2 horizontal to 1 vertical or flatter. The excavations should conform to OSHA guidelines.

#### **Fill Material Consideration**

Structural fill should be placed over a stable or stabilized subgrade. The soils to be used as structural fill should be free of organics, roots, or other deleterious materials. Earthen materials used for structural fill should meet the following material property requirements:

Soil Type <sup>1</sup>	USCS Classification	Acceptable Parameters (for Structural Fill)
Granular	GW, GP, GM, GC, SW, SP, SM, SC	Less than 25% Passing No. 200 sieve

<sup>1.</sup> Structural should consist of approved materials free of organic matter and debris. A sample of each material type should be submitted to the Geotechnical Engineer for evaluation prior to use on this site.

Based on the findings from our hand auger borings, the subject site consists of soils that are mainly silty sands (SM), sand with silt (SP-SM), sand (SP), clayey sand (SC), and sandy clay (CL) in the upper 5 feet BGS. The silty sands (SM), sand with silt (SP-SM), and sand (SP) are generally considered suitable for structural fill, provided that the soils are free of roots, organics or other foreign materials. Clayey sands (SC) may be considered marginally suitable; sandy clays (CL) are deemed unsuitable for structural fill. See Exhibit B-3 for the hand auger boring logs.

We define marginally suitable as the soils that may require extra effort to adjust the moisture before they can be compacted. The amount of effort required will be highly dependent on the season and the weather conditions during construction. We recommend Terracon be retained during construction to determine the suitability of the onsite soil as fill material.

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#### **Fill Compaction Requirements**

Structural fill should meet the following compaction requirements.

Item	Structural Fill
Maximum Lift Thickness	8 to 10 inches or less in loose thickness when heavy, self-propelled compaction equipment is used 4 to 6 inches in loose thickness when hand-guided equipment (i.e. jumping jack or plate compactor) is used
Minimum Compaction Requirements <sup>1</sup>	95% of max. below foundations and below finished pavement subgrade
Water Content Range <sup>1</sup>	Granular: -3% to +3% of optimum

Maximum density and optimum water content as determined by the modified Proctor test (ASTM D 1557).

Some manipulation of the moisture content (such as wetting, drying) will be required during the filling operations to obtain the required degree of compaction. The manipulation of the moisture content is highly dependent on weather conditions and site drainage conditions. Therefore, the contractor should prepare both dry and wet fill materials to obtain the specified compaction during grading. A sufficient number of density tests should be performed to confirm the required compaction of the fill material.

#### **Earthwork Construction Considerations**

Shallow excavations for the water main installation are anticipated to be accomplished with conventional construction equipment. Upon completion of filling and grading, care should be taken to maintain the subgrade water content prior to the utility installation. Construction traffic over the completed subgrades should be avoided. The site should also be graded to prevent ponding of surface water on the prepared subgrades or in excavations. Water collecting over, or adjacent to, construction areas should be removed.

If the subgrade becomes saturated or is disturbed, the affected material should be removed or be scarified, moisture conditioned, and recompacted prior to futility installation. The groundwater table could affect some excavation efforts, particularly over-excavation and replacement of lower strength soils. A temporary dewatering system consisting of sumps with pumps could be necessary to achieve the recommended depth of over-excavation.

As a minimum, excavations should be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P, "Excavations" and its appendices, and in accordance with any applicable local, and/or state regulations.

Construction site safety is the sole responsibility of the contractor who controls the means, methods, and sequencing of construction operations. Under no circumstances shall the

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information provided herein be interpreted to mean Terracon is assuming responsibility for construction site safety, or the contractor's activities; such responsibility shall neither be implied nor inferred.

#### **Construction Observation and Testing**

The earthwork efforts should be monitored under the direction of the Geotechnical Engineer. Monitoring should include documentation of adequate removal of vegetation and topsoil and mitigation of areas of weak subgrade soil.

Each lift of compacted fill should be tested, evaluated, and reworked, as necessary until approved by the Geotechnical Engineer prior to placement of additional lifts. Each lift of fill should be tested for density and water content at a frequency provided by the project plan and specifications.

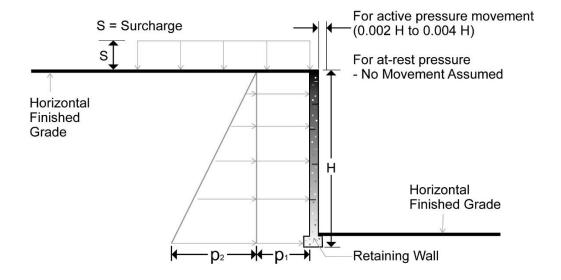
In addition to the documentation of the essential parameters necessary for construction, the continuation of the Geotechnical Engineer into the construction phase of the project provides the continuity to maintain the Geotechnical Engineer's evaluation of subsurface conditions, including assessing variations and associated design changes.

#### LATERAL EARTH PRESSURE CONSIDERATIONS

This project does not include independent permanent retaining walls; however, the temporary entrance and exit pits for the bore process may require temporary shoring retaining walls. The temporary retaining walls with unbalanced backfill levels on opposite sides should be designed for earth pressures at least equal to those indicated in the following table. The earth pressure parameters are recommended based on the soil material obtained in the borings. Earth pressures will be influenced by structural design of the walls, conditions of wall restraint, methods of construction and/or compaction and the strength of the materials being restrained. Two wall restraint conditions are shown. Active earth pressure is commonly used for design of free-standing cantilever retaining walls and assumes wall movement. The "at-rest" condition assumes no wall movement. The recommended design lateral earth pressures do not include a factor of safety or possible hydrostatic pressure on the walls.

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#### **Earth Pressure Coefficients**

Earth Pressure Conditions	Coefficient for Backfill Type	Equivalent Fluid Density (pcf)	Surcharge Pressure, p₁ (psf)	Earth Pressure, p <sub>2</sub> (psf)
Active (Ka)	Granular - 0.36	42	(0.36)S	(42)H
At-Rest (K₀)	Granular - 0.53	62	(0.53)S	(62)H
Passive (K <sub>p</sub> )	Granular - 2.77	319		

#### Applicable conditions to the above include:

- For active earth pressure, wall must rotate about base, with top lateral movements of about 0.002 **H** to 0.004 **H**, where **H** is wall height.
- For passive earth pressure to develop, wall must move horizontally against the fill to mobilize resistance.
- Uniform surcharge, where S is surcharge pressure.
- In situ soil backfill weight a maximum of 115 pcf.
- Horizontal backfill, compacted between 95 percent of modified Proctor maximum dry density. The excavation pit may require a design with sloped backfill depending on its proximity to the existing canal and roadway.
- Loading from heavy compaction equipment or dynamic loading not included.
- No hydrostatic pressures acting on wall.
- No safety factor included in soil parameters.

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The above earth pressure parameters are based on the typical backfill materials available in this area. Backfill placed against structures should consist of granular soils. The granular backfill must extend out from the base of the wall at an angle of at least 45 and 60 degrees from vertical for the active and passive cases, respectively. To calculate the resistance to sliding, a value of 0.35 should be used as the ultimate coefficient of friction between the footing and the underlying soil.

Depending on the depth of excavation and long-term groundwater conditions, the unbalanced hydrostatic pressure may be considered in the design of the retaining wall. Hydrostatic pressure should be added to the lateral earth pressures recommended above. These pressures do not include the influence of surcharge, equipment or floor loading, which should be added. Heavy equipment should not operate within a distance closer than the exposed height of retaining walls to prevent lateral pressures more than those provided.

Due to the presence of the railways and roadways adjacent to the likely excavation areas, the effect of train and vehicular traffic may be considered while designing the lateral support system.

#### **SEISMIC CONSIDERATIONS**

According to the International Building Code (IBC) 2018 and ASCE 7-16, structures should be designed and constructed to withstand the effects of earthquakes and avoid failure during a maximum considered earthquake. The maximum considered earthquake (MCE) is a seismic event that has a 50-year exposure period with a 2% probability of exceedance. The 2,500-year earthquake has a Moment Magnitude (Mw) of 7.3 and a Site Class Adjusted Peak Ground Acceleration (PGA<sub>M</sub>) of **0.132g**, as determined by data provided by the IBC-2018 and ASCE 7-16 Standards.

Based on our findings from the field exploration and our knowledge of the local geological formation in the project area, the site can be classified as **Site Class D** in accordance with International Building Code (IBC) 2018 and ASCE 7-16. The seismic design parameters obtained based on IBC-2018 and ASCE 7-16 are summarized in the table below.

The design response spectrum curve, as presented in **Exhibit C-1**, was developed based on the S<sub>DS</sub> and S<sub>D1</sub> values according to IBC-2018 and ASCE 7-16.

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#### **Summary of Seismic Design Parameters**

Site Location (Latitude, Longitude)	Site Classification	Ss	S <sub>1</sub>	Fa	Fv	S <sub>DS</sub>	S <sub>D1</sub>
31.2366°, -81.5291°	D	0.163g	0.074g	1.600	2.400	0.174g	0.119g

- In accordance with the 2018 International Building Code and ASCE 7-16.
- The 2018 IBC and ASCE 7-16 require a site soil profile determination extending a depth of 100 feet for seismic site classification. The current scope does not include 100-foot soil profile determination. Explorations for this project extended to a maximum depth of 32 feet BGS and this seismic site class definition was provided in consideration of the overall soil conditions as well as the general geology of the area.

#### **GENERAL COMMENTS**

As the project progresses, we address assumptions by incorporating information provided by the design team, if any. Revised project information that reflects actual conditions important to our services is reflected in the final report. The design team should collaborate with Terracon to confirm these assumptions and to prepare the final design plans and specifications. This facilitates the incorporation of our opinions related to implementation of our geotechnical recommendations.

Any information conveyed prior to the final report is for informational purposes only and should not be considered or used for decision-making purposes.

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Natural variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction.

Terracon should be retained as the Geotechnical Engineer, where noted in the final report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our scope of services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence or collaboration through this system are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and

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are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended.

Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client and is not intended for third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly impact excavation cost.

Any parties charged with estimating excavation / earthwork costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety, and cost estimating including, excavation support, and dewatering requirements/design are the responsibility of others.

If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

## **EXHIBITS**

**EXHIBIT A:** Exploration Plan and Procedures

**EXHIBIT B:** Exploration and Testing Results

**EXHIBIT C:** Supporting Information

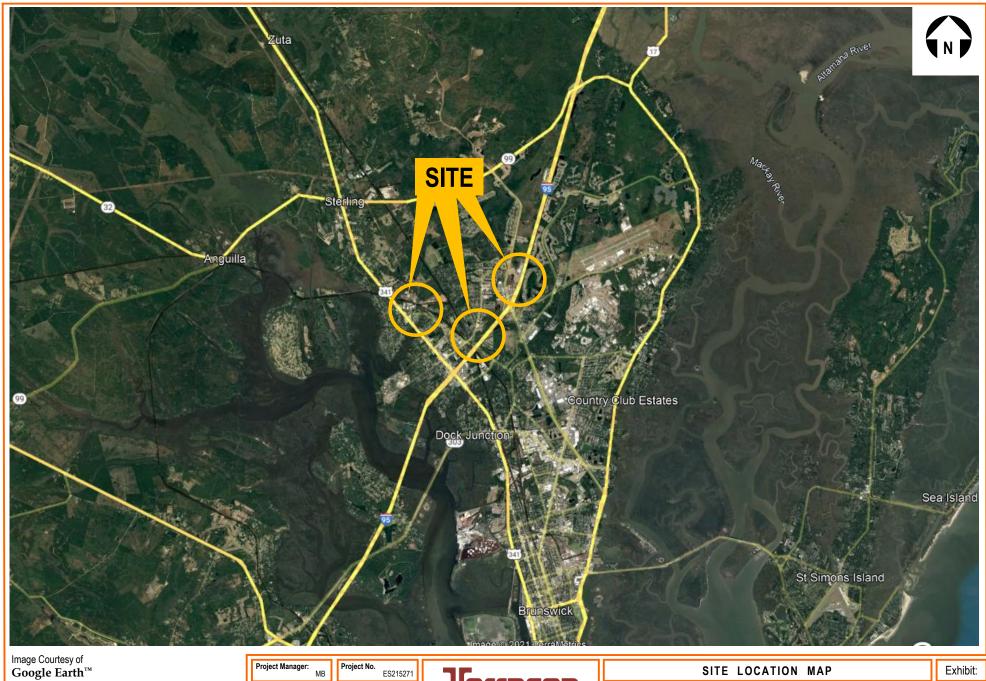
## **EXHIBIT A**

#### **EXPLORATION PLAN AND PROCEDURES**

**Exhibit A-1:** Site Location Plan

**Exhibit A-2:** Exploration Plan

**Exhibit A-3:** Exploration and Testing Procedures

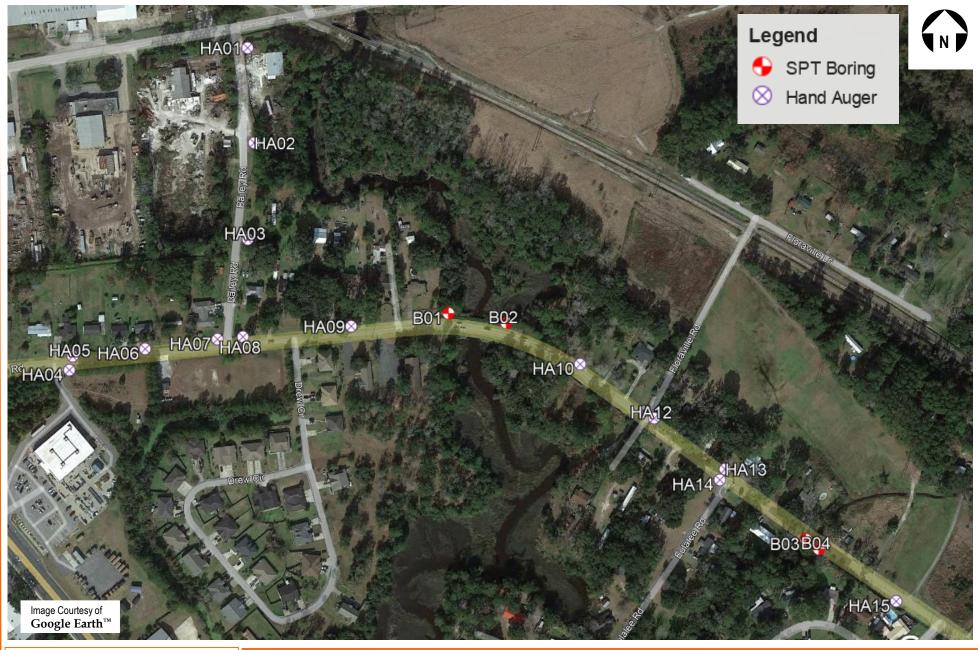


Project Manager:					
r rojest manager.	MB				
Drawn by:	MB				
Checked by:	GL				
Approved by:	GL				

ES215271 Scale: As Shown. File Name: Date: 11-2-2021 2201 Rowland Avenue Savannah, Georgia 31404 Phone (912) 629 4000 Fax (912) 629 4001

**Brunswick-Glynn County North Mainland Water Loops** Brunswick, Georgia

**A-1** 



#### NOTES:

ALL EXPLORATION LOCATIONS WERE LOCATED IN THE FIELD USING A GPS UNIT AND / OR SITE LANDMARKS. EXPLORATION LOCATIONS SHOULD BE CONSIDERED APPROXIMATE. DIAGRAM IS FOR GENERAL LOCATION ONLY; NOT INTENDED FOR CONSTRUCTION PURPOSES.

Project Manager:	MB
Drawn by:	MB
Checked by:	GL
Approved by:	GI

Project No. ES215271

Scale: As Shown.

File Name:

Date: 11-2-2021

Consulting Engineers & Scientists

2201 Rowland Avenue Savannah, Georgia 31404

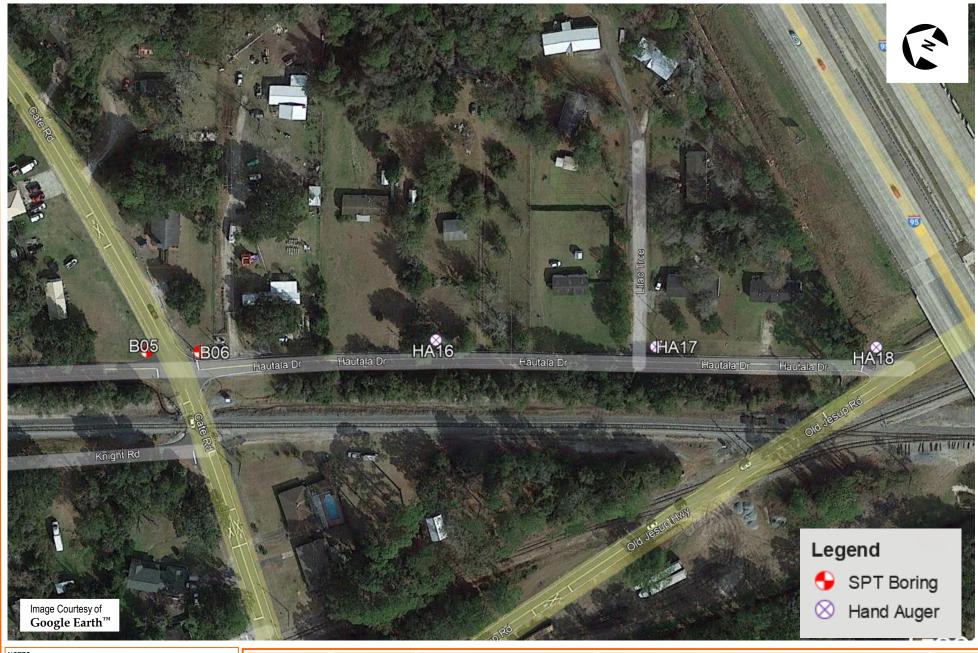
Phone (912) 629 4000 Fax (912) 629 4001

#### **EXPLORATION LOCATION PLAN**

Brunswick-Glynn County North Mainland Water Loops
Old Jessup Road
Brunswick, Georgia

Exhibit:

A-2-1



#### NOTES:

ALL EXPLORATION LOCATIONS WERE LOCATED IN THE FIELD USING A GPS UNIT AND / OR SITE LANDMARKS. EXPLORATION LOCATIONS SHOULD BE CONSIDERED APPROXIMATE. DIAGRAM IS FOR GENERAL LOCATION ONLY; NOT INTENDED FOR CONSTRUCTION PURPOSES.

Project Manager:	MB
Drawn by:	MB
Checked by:	GL
Approved by:	GI

 MB
 Project No.
 ES215271

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 Scale:
 As Shown.

 GL
 File Name:
 Date:

 GL
 11-2-2021



Fax (912) 629 4001

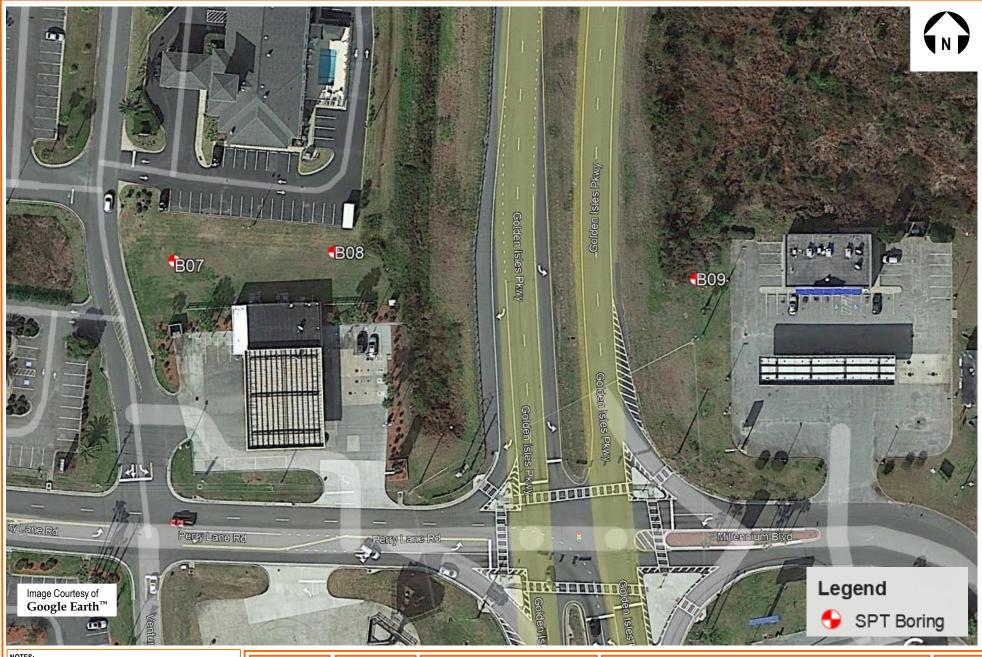
Phone (912) 629 4000

Brunswick-Glynn County North Mainland Water Loops
Hautala Drive
Brunswick, Georgia

**EXPLORATION LOCATION PLAN** 

Exhibit:

A-2-2



ALL EXPLORATION LOCATIONS WERE LOCATED IN THE FIELD USING A GPS UNIT AND / OR SITE LANDMARKS. EXPLORATION LOCATIONS SHOULD BE CONSIDERED APPROXIMATE. DIAGRAM IS FOR GENERAL LOCATION ONLY; NOT INTENDED FOR CONSTRUCTION PURPOSES.

Project Manager:	МВ
Drawn by:	MB
Checked by:	GL
Approved by:	GI

Project No. ES215271 Scale: As Shown. File Name: Date: 11-2-2021

2201 Rowland Avenue Savannah, Georgia 31404

Fax (912) 629 4001

Phone (912) 629 4000

#### **EXPLORATION LOCATION PLAN**

**Brunswick-Glynn County North Mainland Water Loops** Perry Lane Brunswick, Georgia

Exhibit:

A-2-3

#### **EXHIBIT A-3 - EXPLORATION & TESTING PROCEDURES**

Brunswick-Glynn County North Mainland Water Loops ■ Brunswick, Georgia December 9, 2021 ■ Terracon Project No. ES215271



#### **Field Exploration**

No. of Test	Type of Test	Location	Maximum Depth (feet, below ground surface)
9	Standard Penetration Test (SPT) Boring	Roadway shoulder	25
18	Hand Auger Borings	Roadway shoulder	5

**Boring Layout and Elevations:** Unless otherwise noted, Terracon personnel provided the boring layout. Coordinates were obtained with a handheld GPS unit (estimated horizontal accuracy of about  $\pm 10$  feet). The elevations on the borings were interpreted from the topographic survey plan provided the client and should be considered approximate.

#### **Subsurface Exploration Procedures:**

Soil borings were advanced with a truck-mounted drill rig using mud rotary drilling techniques. Five samples were obtained in the upper 10 feet of each boring and at intervals of 5 feet thereafter. Soil sampling was typically performed using thin-wall tube and/or split-barrel sampling procedures. The split-barrel samplers are driven in accordance with the standard penetration test (SPT). The samples were placed in appropriate containers, taken to our soil laboratory for testing, and classified by a Geotechnical Engineer. In addition, we will observe and record groundwater levels during drilling and sampling.

Hand auger borings were conducted in general accordance with ASTM D1452 to determine the subsurface conditions at shallow depths. In this test, the hand auger boring is drilled by rotating and advancing a bucket auger to the desired depths while periodically removing the auger from the hole to clear and examine the auger cuttings. The soils were visually classified by a geotechnical engineer or geologist in accordance with ASTM D2488.

The sampling depths, penetration distances, and other sampling information was recorded on the field boring logs. Our exploration team prepared field boring logs as part of the drilling operations. These field logs included visual classifications of the materials encountered during drilling and our interpretation of the subsurface conditions between samples. Final boring logs were prepared from the field logs. The final boring logs represent the Geotechnical Engineer's interpretation of the field logs and include modifications based on observations and tests of the samples.

#### **Laboratory Testing**

• Moisture Content: ASTM D2216 - Standard Test Methods for Laboratory Determination of

Water Content of Soil and Rock by Mass

Grain Size Analysis: ASTM D422 – Standard Test Method for Particle Size Distribution of Soils

• Atterberg Limits: ASTM D4318 – Standard Test Method for Liquid Limit, Plastic Limit, and

Plasticity Index of Soils

Responsive Resourceful Reliable

## **EXHIBIT B**

#### **EXPLORATION AND TESTING RESULTS**

Exhibit B-1: Subsurface Profile

**Exhibit B-2:** SPT Boring Logs

Exhibit B-3: Hand Auger Boring Log

**Exhibit B-4:** Summary of Laboratory Test Results

**Exhibit B-5:** Grain Size Distribution

**Exhibit B-6:** Atterberg Limits

#### **OLD JESSUP ROAD** B01 B02 B03 **B04** 1-2-1-1 3-3-2-3 2-2-3-2 1-1-1-1 N=3 N=5 N=5 $\nabla$ $\overline{\nabla}$ 1-2-1-1 N=3 1-1-1-2 N=2 2-3-3-3 N=6 3-3-4-4 N=7 1-2-2-1 N=4 1-2-2-2 N=4 2-3-4-3 N=7 2-3-4-4 N=7 Depth - Feet 1-1-2 N=3 1-2-2 N=4 1-1-1 N=2 1-1-2 N=3 5-5-5 N=10 3-1-2 5-6-4 2-2-2 N=10 N=3 2-2-2 N=4 2-2-3 N=5 4-4-6 N=10 6-6-4 N=10 BT-25.0 Ft. BT-25.0 Ft. BT-25.0 Ft. BT-25.0 Ft. Poorly-graded Silty Sand Poorly-graded Clayey Sand Sand with Sand **Explanation** — %w LL PL — Liquid and Plastic Limits Lithology SUBSURFACE PROFILE Project No.: ES215271 See Exploration Plan for orientation of soil profile. See General Notes in Supporting Information for symbols and soil llerracon Date: 1/7/2022 BRUNSWICK-GLYNN COUNTY Water Level Reading Soils profile provided for illustration purposes only. Soils between borings may differ AR - Auger Refusal BT - Boring Termination at time of drilling. NORTH MAINLAND WATER LOOPS Water Level Reading 2201 Rowland Ave Savannah, GA Scale: N.T.S. BRUNSWICK, GA

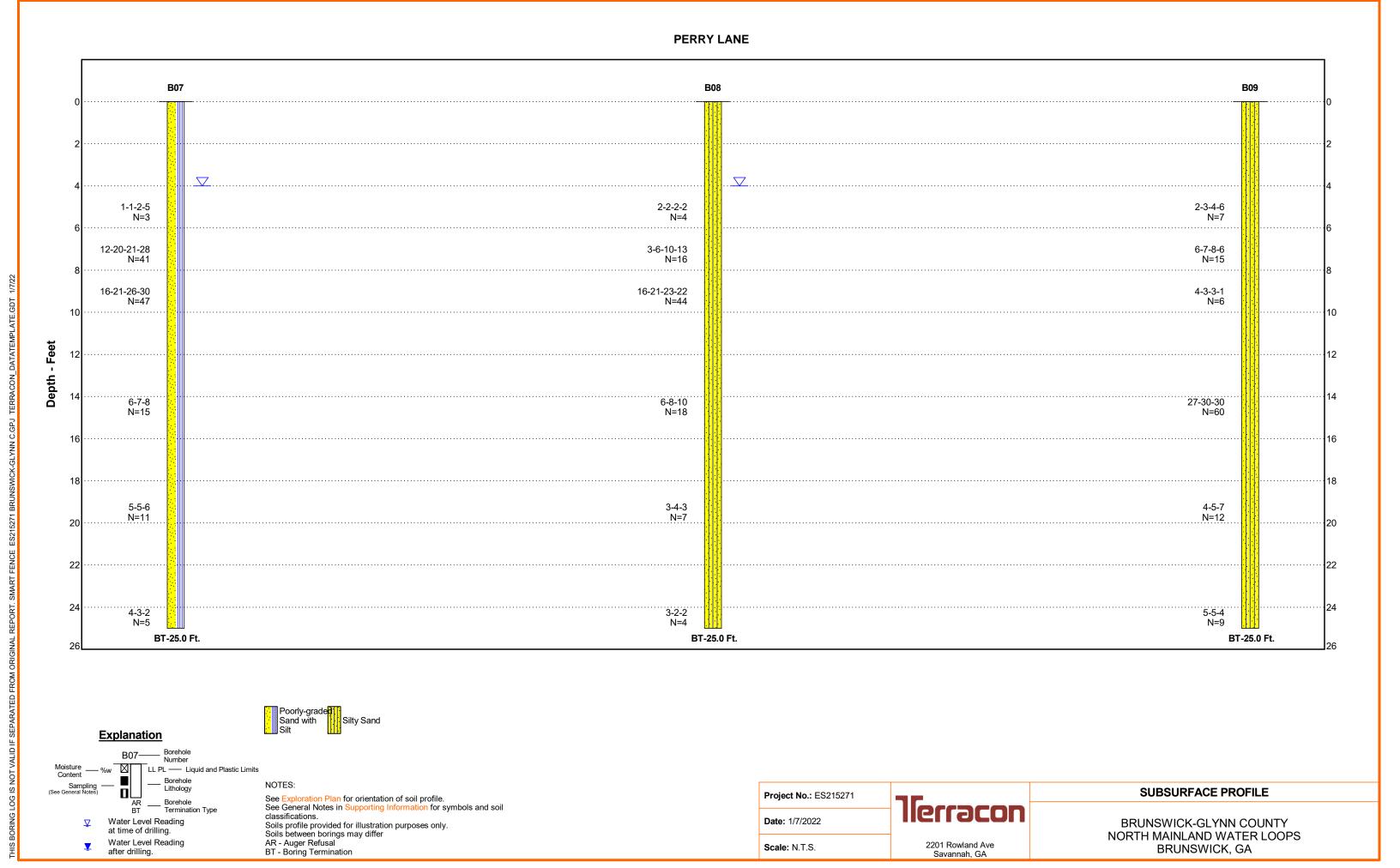
2201 Rowland Ave Savannah, GA

Scale: N.T.S.

Water Level Reading

at time of drilling. Water Level Reading Soils profile provided for illustration purposes only.
Soils between borings may differ
AR - Auger Refusal
BT - Boring Termination

BRUNSWICK, GA



	BORING LOG NO. B01 Page 1 of 1									
	PR	OJECT:	Brunswick-Glynn County Nort Water Loops	h Mainland	CLIENT:	Four Waters Engineerin Greenville, SC	ıg, Ind	C.		
	SIT	ſE:	Old Jesup Road Brunswick, GA		-	, ee				
	GRAPHIC LOG	Latitude: 31	N See Exploration Plan .2393° Longitude: -81.5342°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS
17122		SANI	<b>D (SP)</b> , fine grained, brown, very loose							
LATE.GDT 1		6.0					5 —	abla	$\bigvee$	1-1-1-1 N=2
_DATATEMP		SILT	Y SAND (SM), fine grained, brown, very	loose to medium de	nse		_		X	1-2-1-1 N=3
TERRACON		gray					- 10-		X	1-2-2-1 N=4
ES215271 BRUNSWICK-GLYNN C.GPJ TERRACON_DATATEMPLATE.GDT 1/7/22							- -			
15271 BRUNSW							- 15-		X	1-1-2 N=3
							_			
O SMART LOG							20-		X	5-6-4 N=10
THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL							_ _ _			
M ORIGINAL		25.0 <b>Borir</b>	ng Terminated at 25 Feet				- 25-		X	2-2-2 N=4
ED FRO										
PARAT			on lines are approximate. In-situ, the transition ma material removed via hand auger	ay be gradual.		Hammer Type: Automation	С			
VALID IF SE		cement Meth ow stem	od:	See Exploration and Te description of field and used and additional dat	laboratory proce a (If any).	dures				
OG IS NOT			with auger cuttings upon completion.	See Supporting Informa symbols and abbreviation		IION OT				
INGL	$\overline{\nabla}$		R LEVEL OBSERVATIONS rater encountered @ 6' BGS	75		Boring Started: 12-02-2021	E	Boring	Comp	oleted: 12-02-2021
BOR		Mottling i	<del>-</del>		900	Drill Rig: CME 45		Oriller:	MS	
SHT		Janiy i			wland Ave nah, GA	Project No.: ES215271	E	Exhibit	: B-2-	-1

	BORING LOG NO. B02 Page 1 of 1									
PR	ROJECT:	Brunswick-Glynn County Nor Water Loops	rth Mainland	CLIENT: Four Green	Waters Engineerin	ıg, Ind	c.			
SI	TE:	Old Jesup Road Brunswick, GA								
GRAPHIC LOG	Latitude: 31	N See Exploration Plan 2392° Longitude: -81.5336°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	
1122	DEPTH SAND brown	<b>0 (SP)</b> , fine grained, dark brown, very l	oose							
LAIE.GDI 1/	6.0					5 —		X	1-2-1-1 N=3	
N_DAIAIEM	gray	RLY GRADED SAND WITH SILT (SP-S	<b>im)</b> , fine grained, brov	vn, very loose to med	lium dense	_	$\nabla$	X	1-1-1-2 N=2	
J IEKKACO	9.27					10-		X	1-2-2-2 N=4	
ESZISZTI BKUNSWICK-G-LYNN C. G-J. I ERKRACON DA AI EMPLAIE. GD. 1772						- -				
15271 BRUNSV						- 15 <del></del>		X	1-1-1 N=2	
-NO WELL ESS						_				
HIS BOXING LOG IS NOT VALID IF SEFARATED FROM ORIGINAL REPORT, GEO SWART I LOG-NO WELL.  A part of the property of the propert	trace	rock				20-		X	5-5-5 N=10	
L KEPOKI. GE						_				
OM OKIGINA	25.0 <b>Bori</b> r	g Terminated at 25 Feet				- 25-		X	2-2-3 N=5	
		on lines are approximate. In-situ, the transition r	may be gradual.		Hammer Type: Automation	C				
Advar	ncement Meth Ilow stem		See Exploration and Te description of field and used and additional dat	laboratory procedures	Notes:					
Aband Bor		with auger cuttings upon completion.	See Supporting Informa symbols and abbreviati							
.5 C		R LEVEL OBSERVATIONS	77		Boring Started: 12-02-2021	E	Boring	Comp	oleted: 12-02-2021	
		ater encountered @ 7' BGS	4 lierr	acon	Drill Rig: CME 45		Oriller:	MS		
E N	Mottling not noted  2201 Rowland Ave Savannah, GA Project No.: ES215271						Exhibit: B-2-2			

	BORING LOG NO. B03 Page 1 of 1									
	PR	OJECT: Brunswick-Glynn County Nort Water Loops	th Mainland	CLIENT: Four Green	Waters Engineerir nville, SC	ng, Ind	С.			
	SIT	-			·					
	GRAPHIC LOG	LOCATION See Exploration Plan  Latitude: 31.2374° Longitude: -81.5306°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	
7122		DEPTH  CLAYEY SAND (SC), fine to medium grained	d, light brown, loose							
-ATE.GDT 1/		fine grained, gray 6.0				5 —	$\overline{}$	X	3-3-2-3 N=5	
DATATEMPL		SILTY SAND (SM), fine grained, gray, loose	to medium dense			_		X	2-3-3-3 N=6	
TERRACON		fine to medium grained, dark gray				- 10-		X	2-3-4-3 N=7	
ES215271 BRUNSWICK-GLYNN C.GPJ TERRACON_DATATEMPLATE.GDT 1/7/22		fine grained, gray								
215271 BRUNSM						- 15 <del></del>		X	1-2-2 N=4	
						_				
EO SMART LOC						20-		X	2-2-2 N=4	
THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL						_				
OM ORIGIN		25.0  Boring Terminated at 25 Feet				25 <del>-</del>		X	4-4-6 N=10	
TED FR		Stratification lines are approximate. In-situ, the transition m	av be gradual		Hammer Type: Automati	c				
EPARA	_	First 4' of material removed via hand auger	_, ~o g. adddi.			-				
VALID IF S		cement Method: ow stem	See Exploration and Te description of field and used and additional dat  See Supporting Informa	laboratory procedures a (If any).	Notes:					
JG IS NOT		onment Method: ng backfilled with auger cuttings upon completion.	symbols and abbreviati							
NG LC	$\overline{}$	WATER LEVEL OBSERVATIONS	75		Boring Started: 12-02-2021	E	Boring	Comp	oleted: 12-02-2021	
30RII	<u> </u>	Groundwater encountered @ 4.5' BGS	lierr	acon	Drill Rig: CME 45		Oriller:	MS		
THIS E		Mottling not noted	2201 Ro	wland Ave nah, GA	Project No.: ES215271	E	Exhibit	: B-2-	3	

	BORING LOG NO. B04 Page 1 of 1								
PR	ROJECT:	Brunswick-Glynn County No Water Loops	rth Mainland	CLIENT:	Four Waters Engineerir Greenville, SC	ıg, Ind	 C.		
SIT	TE:	Old Jesup Road Brunswick, GA							
GRAPHIC LOG	Latitude: 31	N See Exploration Plan .2373° Longitude: -81.5305°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS
	2.0	Y SAND (SM), fine grained, dark brown							
AIE.GDI 1//	light t	prown				5 —	<u> </u>	X	2-2-3-2 N=5
DaiaiemPl	light o	gray				_			3-3-4-4 N=7
) IEKKACON	10.0 SILT	<b>Y SAND (SM)</b> , fine grained, gray, very	loose to medium den	se		- 10-		X	2-3-4-4 N=7
ESZ15Z/1 BRUNSWICK-GE-YNN C.GPJ TERRACON DATA IEMPLATE, GDT 17772	<u>5121</u>	<u>. Graso (Gin),</u> ililo grailled, gray, very	loose to mediam don			- -			
15271 BRUNSV						- 15 <del></del>		X	1-1-2 N=3
						_			
SMAKI LOG						20-		X	3-1-2 N=3
HIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SWART LOG-NO WELL.  A draw and the separate of the separate						_			
OM OKIGINAL	25.0 <b>Borir</b>	ng Terminated at 25 Feet				- 25 <del></del>		X	6-6-4 N=10
ED FK									
PAKA		on lines are approximate. In-situ, the transition material removed via hand auger	may be gradual.		Hammer Type: Automati	c _			
Advan	ncement Meth llow stem	od:	See Exploration and Todescription of field and used and additional da	laboratory proced ta (If any).	lures				
Aband Bor	donment Meth ring backfilled	od: with auger cuttings upon completion.	See Supporting Information symbols and abbreviation		on of				
		R LEVEL OBSERVATIONS	75		Boring Started: 12-02-2021	Е	Boring	Comp	oleted: 12-02-2021
	Groundw Mottling r	rater encountered @ 4' BGS	– lieff	900	Drill Rig: CME 45		Oriller:	MS	
三 三 三	iviotuli ig f	ioi notau		wland Ave nah, GA	Project No.: ES215271	E	Exhibit	: B-2-	-4

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL ES215271 BRUNSWICK-GLYNN C. GPJ TERRACON. DATATEMPLATE. GDT 117/22

			BORING L	OG NO. B0	6			F	age 1 of 1
PF	ROJECT:	Brunswick-Glynn County Nor Water Loops	th Mainland	CLIENT: Four Green	Waters Engineerir	ng, Inc			
SI	TE:	Old Jesup Road Brunswick, GA							
90-	LOCATIO	N See Exploration Plan				j ::	/EL	/PE	T. S
GRAPHIC LOG	Latitude: 31	.2311° Longitude: -81.5169°				DEPTH (Ft.)	ER LE	SAMPLE TYPE	FIELD TEST RESULTS
GRA							WATER LEVEL OBSERVATIONS	SAMF	핊묎
	DEPTH SILT	Y SAND (SM), fine grained, dark brown,	, very loose to very o	lense					
	· .					-			
	light	gray					abla		
77/1/57	: :								
	dark	brown				5		M	2-1-1-1
TAIE						٦			N=2
A I EMI	<u>:</u>							M	1-6-12-15
DAU						_			N=18
ESZISZT BKUNSWICK-GLYNN C.GFJ IERKRACON_DATALEWITALE.GJJ 17772						_		X	16-28-28-32 N=56
Ž.	brow	n and orange				10-			
20.02		v				_			
S LAN	· ·					_			
SWICK	• • • • • • • • • • • • • • • • • • •								
RYON RYON						4-		X	8-10-11 N=21
15271	light l	prown				15-			
L ESZ	<u>.</u>								
O WEL	<mark>:</mark> :								
N-50	i. :								2-3-4
MAK	gray					20-		$\triangle$	N=7
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<del>Š</del>	<u>:</u> :					_			
A XE						_			
N C C C C C C C C C C C C C C C C C C C						-		X	3-4-3 N=7
	25.0 Borii	ng Terminated at 25 Feet				25			
	Stratificati	on lines are approximate. In-situ, the transition m	nay be gradual.		Hammer Type: Automati	c			
A A A	First 4' of I	material removed via hand auger	_						
	ncement Meth blow stem	OO:	See Exploration and Todescription of field and used and additional da	laboratory procedures	Notes:				
Δhan	donment Meth	nod:		ation for explanation of					
Boi		with auger cuttings upon completion.	Symbolo and abbreviat						
		R LEVEL OBSERVATIONS	75		Boring Started: 12-02-2021	Во	oring	Comp	oleted: 12-02-2021
NOW WE WANTED	Groundw Mottling	rater encountered @ 3' BGS not noted		acon	Drill Rig: CME 45	Di	riller:	MS	
<u></u>	J			owland Ave nnah, GA	Project No.: ES215271	E	xhibit	: B-2-	6

			BORING L	OG NO. B0	7			F	Page 1 of 1
PR	OJECT:	Brunswick-Glynn County No	rth Mainland	CLIENT: Four	Waters Engineerin	ng, In	C.		
SI	ГЕ:	Water Loops Old Jesup Road Brunswick, GA		Greei	nville, SC				
90	LOCATION	See Exploration Plan				~	EL SNS	TYPE	F
GRAPHIC LOG	Latitude: 31	.2467° Longitude: -81.5058°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	E TY	FIELD TEST RESULTS
GRAP						DEP.	NATE BSER	SAMPLE	FIELI
	DEPTH POOF	RLY GRADED SAND WITH SILT (SP-S	<b>SM)</b> , fine grained, dark	brown, very loose to	o dense		-0	0)	
						-			
						_			
						-			
						_	$\nabla$		1-1-2-5
						5 –	-		N=3
						_			12-20-21-28
						_	-		N=41
						_			16-21-26-30
						10-			N=47
	browr	1				10-			
						_			
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						15-			N=15
						_	-		
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						_			5-5-6
	aray					20-			N=11
	gray					_	-		
						_			
						-			
						_			4-3-2 N=5
_	25.0 <b>Borin</b>	ng Terminated at 25 Feet				25-			С-И
		•							
		on lines are approximate. In-situ, the transition in material removed via hand auger	may be gradual.		Hammer Type: Automati	С			
	ncement Meth	od:	See Exploration and Te description of field and	esting Procedures for a	Notes:				
			used and additional dat	a (If any).					
	donment Meth	od: with auger cuttings upon completion.	See Supporting Informa symbols and abbreviation						
501									
$\overline{\nabla}$		R LEVEL OBSERVATIONS ater encountered @ 4' BGS		acon	Boring Started: 12-02-2021				oleted: 12-02-2021
	Mottling r	<del>-</del>		Wland Ave	Drill Rig: CME 45		Oriller:		
				nah, GA	Project No.: ES215271	[1	Exhibit	: B-2-	-7

	BORING LOG NO. B08			F	Page 1 of 1				
PF	ROJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: Four Green	Waters Engineerir nville, SC	ng, Ind	C.		
SI	TE:	Old Jesup Road Brunswick, GA							
GRAPHIC LOG		N See Exploration Plan  .2469° Longitude: -81.5054°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS
G R	DEPTH						WA	SAN	Ξ.
		<b>Y SAND (SM)</b> , fine grained, gray, loos	se to dense						
	dark	prown				_			
						_	$\overline{\mathbf{Z}}$		
						5 —			2-2-2-2 N=4
						_			3-6-10-13 N=16
	browi	1				- 10-			16-21-23-22 N=44
						- -			
						_			6-8-10 N=18
						15 <del>-</del>			14-10
						_			
						_			3-4-3 N=7
	gray					20 <del>-</del> -			
						_			
	25.0	ng Terminated at 25 Feet				- 25-			3-2-2 N=4
	Bom	g reminated at 20 reet							
		on lines are approximate. In-situ, the transition naterial removed via hand auger	n may be gradual.		Hammer Type: Automati	С			
	ncement Meth Illow stem	od:	description of field an used and additional d		Notes:				
	donment Meth ring backfilled	od: with auger cuttings upon completion.	See Supporting Inforr symbols and abbrevia	nation for explanation of ations.					
$\nabla$		R LEVEL OBSERVATIONS	75-6		Boring Started: 12-02-2021	E	Boring	Com	oleted: 12-02-2021
	Mottling i	ater encountered @ 4' BGS not noted		acon	Drill Rig: CME 45		Oriller:	MS	
				lowland Ave annah, GA	Project No.: ES215271	E	Exhibit	: B-2	-8

			BORING L	OG NO. HA	01	Pa	ige 1	of 1	
PR	OJECT:	Brunswick-Glynn County N Water Loops	orth Mainland	CLIENT: Four Gree	Waters Engineering, Industrial		<u> </u>		
SIT	Œ:	Old Jesup Road Brunswick, GA							
99.	LOCATIO	N See Exploration Plan					t.	/EL	Ĺ
GRAPHIC LOG	Latitude: 31	.2416° Longitude: -81.5362°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	L C 1 C 1 C 1 C 1 C 1 C 1 C 1 C 1 C 1 C
1 <sup>N</sup> 'I	DEPTH TOPS	<b>SOIL</b> , dark brown fine grained silty sa	and with grass					>0	+
· <u>\1 /,</u>			J						
		Y SAND (SM), fine grained, light brow	vn						
							1 -		
							2 -		
	2.3	(EV CAND (CO) for a major of Body							
	CLAY	<u><b>(EY SAND (SC)</b>,</u> fine grained, light gr	ray						
							3 -		
							3 -		
							4 -		-
<u>///</u>	5.0 <b>Borir</b>	ng Terminated at 5 Feet					5 –		ł
	Stratification	on lines are approximate. In-situ, the transitio	n may be gradual.						
	cement Meth nual hand aug		description of field an	Testing Procedures for a d laboratory procedures	Notes:				
	onment Meth	nod: with auger cuttings upon completion.	used and additional of See Supporting Information symbols and abbrevia	nation for explanation of					
DOF	_		<u> </u>						
Z		R LEVEL OBSERVATIONS vater encountered @ 4' BGS		acon	<u> </u>	Boring Comple	eted: 12	-01-20	)21
	Mottling I	noted @ 2' BGS	2201 F	Rowland Ave		Oriller: CS			
			Sava	annah, GA	Project No.: ES215271	Exhibit: B-3-1			

			BORING L	OG NO. H	IA02	Page	e 1 o	of 1	
PR	OJECT:	Brunswick-Glynn County N Water Loops	orth Mainland	CLIENT: Fo	our Waters Engineering, Inc. reenville, SC				
SIT	E:	Old Jesup Road Brunswick, GA							
C LOG		N See Exploration Plan  .2408° Longitude: -81.5362°				į	(Ft.)	EVEL TIONS	TYPE
GRAPH						F C C	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMDI E TVDE
1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1/ 1	DEPTH TOPS	SOIL, dark brown fine grained silty sa	and with grass						_
<u>\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ </u>		<b>/ SAND (SM)</b> , fine grained, light brov	wn						
							1 -		
						2	2 –		
						;	3 –		
							1		
							4 –		
	5.0 <b>Borin</b>	ng Terminated at 5 Feet				;	5 —		_
	Stratification	on lines are approximate. In-situ, the transitio	n may be gradual.						_
Advano	cement Meth	oq.	Coo Explanation and 3	Sastina Duaga duna fa	ra Notes:				_
Man	ual hand aug		See Exploration and I description of field an used and additional d	ata (If any).	es				
	onment Meth	od: with auger cuttings upon completion.	See Supporting Inform symbols and abbrevia	iauon for explanation tions.	OI				
	WATE	R LEVEL OBSERVATIONS	75		Boring Started: 12-01-2021 Bori	ing Completed	d: 12-	01-20	21
	Groundw	ater not encountered	lieu	acor	Drill Rig: Hand Auger Dril	ler: CS			_
	woluling f	noted @ 2' BGS	2201 R	owland Ave nnah, GA		nibit: B-3-2			_

			BORING LO	OG NO.	HA03	Paç	ge 1	of 1	
PRO	JECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT:	Four Waters Engineering, Inc Greenville, SC	· -			
SITE	<b>:</b>	Old Jesup Road Brunswick, GA							
2		N See Exploration Plan 2400° Longitude: -81.5362°					DEPTH (Ft.)	LEVEL ATIONS	SAMPI E TYPE
	EPTH						DEPT	WATER LEVEL OBSERVATIONS	SAMPI
<u> </u>	TOPS	<b>GOIL</b> , dark brown fine grained silty sa	nd with grass						
0.6		<u>/ SAND (SM)</u> , fine grained, light brow	n						
							1 -	-	
							2 -		
							3 -		
							4 -		
							7		
. 1 . 5.0	Borin	g Terminated at 5 Feet					5 –		T
	Stratification	on lines are approximate. In-situ, the transition	may be gradual.						_
Advancer Manua	ment Metho al hand aug		See Exploration and T description of field and used and additional da	esting Procedures I laboratory proced ata (If any).	s for a dures Notes:				
	ment Meth backfilled	od: with auger cuttings upon completion.	See Supporting Inform symbols and abbrevia	ation for explanati tions.	ion of				
	WATE	R LEVEL OBSERVATIONS	76		Boring Started: 12-01-2021 Bo	oring Complet	ted: 12-	-01-20	)21
(	Groundw Mottling ::	ater not encountered	liem	900	Drill Rig: Hand Auger D	riller: CS			
,	violuii ig r	noted @ 1.5' BGS	2201 R	owland Ave nnah, GA		xhibit: B-3-3			_

			BORING LO	OG NO.	HA04	Page	e 1	of 1	
PRO	JECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: I	Four Waters Engineering, Inc. Greenville, SC				
SITE	<b>:</b>	Old Jesup Road Brunswick, GA							
90 <u>.</u>	OCATION	See Exploration Plan					t.)	/EL ONS	10/
GRAPHIC LOG	atitude: 31.	2388° Longitude: -81.5380°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	DAMPI E TVDE
D	EPTH							>8	٥
7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		<b>GOIL</b> , dark brown fine grained silty sa	nd with grass						
0.0	6 CLAY	<u><b>(EY SAND (SC)</b></u> , fine grained, light gra	ау						
							1 -		
							2 -		
							3 –	$\overline{\nabla}$	
							4 –		
5.0	.0 Borin	g Terminated at 5 Feet					5 —		t
	Stratification	on lines are approximate. In-situ, the transition	may be gradual.					1	_
	ment Methoral hand aug		See Exploration and Te description of field and used and additional da	laboratory proced	for a Notes:				_
	nment Meth g backfilled	od: with auger cuttings upon completion.	See Supporting Informations symbols and abbreviate		on of				
	WATE	R LEVEL OBSERVATIONS	75		Boring Started: 12-01-2021 Boring	Complete	ed: 12-	-01-20	)21
		ater encountered @ 3' BGS noted @ 1.67' BGS	- lierr	900	Drill Rig: Hand Auger Driller	•	12-		
,	wouling f	10.60 W 1.01 DGS	2201 Ro	wland Ave nah, GA		t: B-3-4			_

			BORING LO	OG NO.	HA05	Page 1	of 1	
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: I	Four Waters Engineering, Inc. Greenville, SC			
SIT	E:	Old Jesup Road Brunswick, GA						
GRAPHIC LOG	Latitude: 31	N See Exploration Plan .2389° Longitude: -81.5380°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	חמאד ח ומאאי מ
11. <u>11.</u>		<b>SOIL</b> , dark brown fine grained silty sa	and with grass					
		Y SAND (SM), fine grained, light brow	vn			1 -		
	1.5 <u>CLA</u> )	<b>/EY SAND (SC)</b> , fine grained, light gr	ay			2 -		
						3 -		
						4 -		
	5.0					5-	$\nabla$	
	Borii	ng Terminated at 5 Feet						
		on lines are approximate. In-situ, the transition			T.:.		•	_
Man	cement Meth nual hand aug onment Meth ng backfilled	ger	See Exploration and To description of field and used and additional da  See Supporting Inform symbols and abbreviat	laboratory proced ta (If any).  ation for explanation	lures			
	_	R LEVEL OBSERVATIONS			Design Object 40 04 0004	- Camerilat I ti	2.04.05	
$\nabla$	Groundw	rater encountered @ 5' BGS		<b>aco</b>	Boring Started: 12-01-2021 Borin	ig Completed: 12	2-01-20	121
	Mottling I	noted @ 2' BGS	2201 Rd	wland Ave		er: CS		_
			Savar	nnah, GA	FIUJECTIVO ESZ 1SZ / 1 EXNIK	oit: B-3-5		

			BORING LO	OG NO. F	IA06	Page	1 0	of 1	
PR	OJECT:	Brunswick-Glynn County N Water Loops	North Mainland	CLIENT: F	our Waters Engineering, Inc. reenville, SC				
SIT	E:	Old Jesup Road Brunswick, GA							
SLOG		N See Exploration Plan				£		EVEL	TYPE
GRAPHIC	Latitude: 31	.2390° Longitude: -81.5373°				DEPTH (Ft.)	i	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
· · · · · · · · · · · · · · · · · · ·	DEPTH TOPS	SOIL, dark brown fine grained silty s	sand with grass					-0	_
<u>, , , , , , , , , , , , , , , , , , , </u>	10.6								
		DY LEAN CLAY (CL), dark gray							
						1	_		
						2	· –		
						3			
							,		
						4	. –		
	- 0								
<u>/////</u>	5.0 <b>Borir</b>	ng Terminated at 5 Feet				5	; <del>-  </del>		-
	Stratification	on lines are approximate. In-situ, the transition	on may be gradual.						
	cement Meth nual hand auç		See Exploration and I description of field and used and additional dates	esting Procedures for I laboratory procedure ata (If any).	or a Notes:				
	onment Mething backfilled	nod: with auger cuttings upon completion.	See Supporting Inform symbols and abbrevia	nation for explanation tions.	of				
		R LEVEL OBSERVATIONS	76		Boring Started: 12-01-2021 Borin	ng Completed:	12-	01-202	<u>-</u>
		rater not encountered noted @ 0.83' BGS		acor	Drill Rig: Hand Auger Drille	er: CS			_
				owland Ave nnah, GA	Project No.: ES215271 Exhil	bit: B-3-6			

		I	BORING LO	OG NO. H	HA07	P	age 1	of 1	
PR	OJECT:	Brunswick-Glynn County No Water Loops	rth Mainland	CLIENT: F	our Waters Engineering, Inc Greenville, SC	· ·			
SIT	ΓE:	Old Jesup Road Brunswick, GA			2.00.11				
90.	LOCATIO	See Exploration Plan		1			· · ·	E SNS	Į Į
GRAPHIC LOG	Latitude: 31	.2391° Longitude: -81.5365°					DEPTH (Ft.)	WATER LEVEL	SAMPI E TYPE
<u>, v. </u>	DEPTH	<b>SOIL</b> , dark brown fine grained silty san	ad with groop					≥ ₽	Ü
<u>, , , , , , , , , , , , , , , , , , , </u>		<u>sole,</u> dark blown line granied slity san	id willi grass						
	0.6 SILT	<b>/ SAND (SM)</b> , fine grained, light brown	າ						
							1 -		
							-		
							2 -		
							3 -		
							4 -	$\nabla$	-
	5.0						_		
	Borir	ng Terminated at 5 Feet					5-		
	Stratification	on lines are approximate. In-situ, the transition	may be gradual						
	Ottatilloati	or inics are approximate. It situ, the transition	may be gradual.						
	cement Meth nual hand auç		See Exploration and Tedescription of field and used and additional date	laboratory procedu	or a Notes:				
	lonment Mething backfilled	od: with auger cuttings upon completion.	See Supporting Informa symbols and abbreviati		n of				
	WATE	R LEVEL OBSERVATIONS	7-		Boring Started: 12-01-2021 B	oring Comp	leted: 10	_01_2	N24
$\nabla$	Groundw	ater encountered @ 4' BGS	llerr	9C0I	Drill Rig: Hand Auger D	oring Compl Priller: CS	ieleu. 12	-01-2	∪∠ I
	Mottling I	noted @ 2' BGS	2201 Ro	wland Ave nah, GA		xhibit: B-3-7			
			Savan	nan, GA	1 10/001 NO LOZ 102/1	VI IIDIL. D-9-1			

			BORING LO	OG NO. H	HA08	Pί	age 1	of 1	
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: F	our Waters Engineering, Inc. Greenville, SC				
SIT	E:	Old Jesup Road Brunswick, GA			neenvine, 30				
90	LOCATION	N See Exploration Plan					_	NS EL	Щ
GRAPHIC LOG	Latitude: 31	.2391° Longitude: -81.5363°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
	DEPTH	2011	1 20					> 8	ù
11/		<u>SOIL</u> , dark brown fine grained silty sa	nd with grass						
16.7	0.6 <b>SILT</b>	Y SAND (SM), fine grained, light brow	'n						
							1 -		
							•		
							2 -		
							3 -		
							4 -		
	5.0								
1.1.1.		ng Terminated at 5 Feet					5 –		T
	Stratification	on lines are approximate. In-situ, the transition	ı may be gradual.						
Advan	cement Meth	oq.	lo = 1		or a Notes:				
	ual hand aug		See Exploration and Te description of field and used and additional da	laboratory procedu	or a				
	onment Meth	od: with auger cuttings upon completion.	See Supporting Informa symbols and abbreviati		n of				
		R LEVEL OBSERVATIONS	<del> </del>						
$\nabla$	Groundw	rater encountered @ 5' BGS		9C0	Boring Started: 12-01-2021 Bo	ring Compl	eted: 12	-01-2	)21
	Mottling r	noted @ 2' BGS	2201 Ro	wland Ave		iller: CS			
			Savan	nah, GA	Project No.: ES215271 Ext	hibit: B-3-8			

			BORING LO	OG NO.	HA09		Pa	age 1	of 1	
PRO	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT:	Four Waters Greenville, S	Engineering, Inc. C		J		
SIT	E:	Old Jesup Road Brunswick, GA								
90	LOCATION	See Exploration Plan						·	'EL ONS	Ĺ
GRAPHIC LOG	Latitude: 31	.2392° Longitude: -81.5352°						DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	1
	DEPTH								>8	3
711/		<u>SOIL</u> , dark brown fine grained silty sar	nd with grass							
<u> </u>	0.6 SILT	Y SAND (SM), fine grained, light brown	n							
								1 -		
	2.3 CLAN	VEV SAND (SC) fine grained light bro	owalarov					2 -		
	CLAY	<u><b>EY SAND (SC)</b></u> , fine grained, light bro	own/gray							
								3 -		
								3		
								4 -		
	5.0 <b>Bori</b> r	ng Terminated at 5 Feet						5 –	$\nabla$	-
		g rommatou at or oot								
	Stratification	on lines are approximate. In-situ, the transition	may be gradual.							
	ement Meth ual hand aug		See Exploration and Te description of field and used and additional dat	laboratory proce	Notes:					
	onment Meth	od: with auger cuttings upon completion.	See Supporting Informa symbols and abbreviati		tion of					
	WATE	R LEVEL OBSERVATIONS			Roring Cto	rted: 12-01-2021 Borii	ng Comple	ated: 10	_01_20	12,
V_	Groundw	ater encountered @ 5' BGS	llerr	900	Drill Rig: H		er: CS	JIGU. IZ	-∪ 1-∠(	/4
	Mottling r	noted @ 2' BGS	2201 Ro	wland Ave nah, GA		-	bit: B-3-9			
			Savan	nan, on	I JOJECT NO	[EXIII	DIC D-0-8			

			BORING LO	OG NO. H	HA10	Pa	age 1	of 1	
PR	OJECT:	Brunswick-Glynn County N Water Loops	orth Mainland	CLIENT: F	our Waters Engineering, Inc. reenville, SC				
SIT	E:	Old Jesup Road Brunswick, GA							
GRAPHIC LOG		N See Exploration Plan  .2389° Longitude: -81.5329°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
	DEPTH						DE	WAT	
<u>, , , , , , , , , , , , , , , , , , , </u>		SOIL, dark brown fine grained silty sa	and with grass						
	0.6 CLAY	<b>'EY SAND (SC)</b> , fine grained, light bi	rown						
							1 -		
							2 -		
							3 -		
							4 -		
	<b>5</b> 0								
·/·	Borin	ng Terminated at 5 Feet					5 –		T
	Stratification	on lines are approximate. In-situ, the transitio	n may be gradual.						
A .l					Tw.				
	cement Meth ual hand aug		See Exploration and Te description of field and used and additional da	laboratory procedul ta (If any).	res				
	onment Meth ng backfilled	od: with auger cuttings upon completion.	See Supporting Information symbols and abbreviate		i Ui				
		R LEVEL OBSERVATIONS	75		Boring Started: 12-01-2021 Boring	ng Comple	eted: 12	-01-2	021
		rater not encountered noted @ 2' BGS	liell	acor	Drill Rig: Hand Auger Drill	er: CS			_
				wland Ave nah, GA	Project No.: ES215271 Exhi	bit: B-3-10	0		

			BORING LO	OG NO. HA	.11	P	age 1	of 1	
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: Four	Waters Engineering, Ir	ıc.	_		
SIT	E:	Old Jesup Road Brunswick, GA							
907c		N See Exploration Plan  .2385° Longitude: -81.5322°					(Ft.)	EVEL	TVPE
GRAPHIC LOG	Lautude. 31	.2363 LOrigitude61.3322					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	DAMPI E TVDE
	DEPTH		1 20					> 0	,
1 · 2 · 1/2		SOIL, dark brown fine grained silty sa	and with grass						
<u>(1 () : \(\frac{1}{2}\)</u>	0.6 <u>CLAY</u>	<b>/EY SAND (SC)</b> , fine grained, dark bi	rown/gray						
							1 -		
							2 -		
							3 -		
							4 -		
	5.0						_		
	Borir	ng Terminated at 5 Feet					5 –		
	Stratification	on lines are approximate. In-situ, the transition	n may be gradual.						
	cement Meth nual hand aug		See Exploration and Te description of field and used and additional da	laboratory procedures	Notes:				
	onment Meth	od: with auger cuttings upon completion.	See Supporting Informations symbols and abbreviate						
		R LEVEL OBSERVATIONS	75.		Boring Started: 12-01-2021	Boring Comp	leted: 12	-01-20	)21
		rater not encountered noted @ 0.83' BGS	Drill Rig: Hand Auger			Driller: CS			
	3.			wland Ave nah, GA	Project No.: ES215271	Exhibit: B-3-1	1		_

BORING LOG NO. HA12 Page 1 of 1								
PROJEC <sup>*</sup>	F: Brunswick-Glynn County Water Loops	North Mainland	CLIENT: Four Gree	Waters Engineering, Innville, SC	c.			
SITE:	Old Jesup Road Brunswick, GA							
9	ON See Exploration Plan 31.2384° Longitude: -81.5321°					DЕРТН (Ft.)	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
DEPTH	<b>PSOIL</b> , dark brown fine grained silty	and with gross				DEF	WATE	OMAG
<u>10</u>	<u>FSOIL</u> , dark brown line grained sitty	sand with grass						
	.TY SAND (SM), fine grained, light br	own				1 -		
						•		
						2 -		
						0		
						3 –		
						4 –		
5.0						_		
Во	ring Terminated at 5 Feet					5 —		
Stratific  Advancement M Manual hand  Abandonment M Boring backfil  WA  Groun Mottlin								
Stratific	ation lines are approximate. In-situ, the transi	tion may be gradual.			<u> </u>			_
Advancement M Manual hand		used and additional of		Notes:				
Abandonment M Boring backfil	ethod: ed with auger cuttings upon completion.	See Supporting Infonsymbols and abbrevi	mation for explanation of ations.					
WA	TER LEVEL OBSERVATIONS	77		Boring Started: 12-01-2021	Boring Complete	d: 12-	01-202	21
Groundwater not encountered Mottling noted @ 2' BGS  Boring Started: 12-01-2021 Boring Con Drill Rig: Hand Auger Driller: CS				Driller: CS	cs			
2201 Rowland Ave Savannah, GA Project No.: ES215271 Exhibit: B-3-12							_	

	BORING LOG NO. HA13							of 1			
F	PRC	DJECT:	Brunswick-Glynn County Not Water Loops	rth Mainland	CLIENT:	Four \	Vaters Engineering, I	nc.			
Ş	SITI	E:	Old Jesup Road Brunswick, GA			0.00	,				
5	تع ا	LOCATIO	N See Exploration Plan		1				·	NS NS	PE
		Latitude: 31	.2380° Longitude: -81.5314°						DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE
9	75   EXA 15								DEP	WATE	SAMP
. <u> 7,</u>	71.17 X7,7	DEPTH TOPS	SOIL, dark brown fine grained silty san	d with grass						- 0	0,
1											
	<u> </u>	).6 <b>SILT</b>	Y SAND (SM), fine grained, light brown	<u> </u>							
12/10/21									1 -		
DT 12									'		
ATE.G											
TEMPL											
DATA	2		DY LEAN CLAY (CL), dark gray						2 -		
ACON											
TERR											
ES215271 BRUNSWICK-GLYNN C.GPJ TERRACON_DATATEMPLATE.GDT									3 -		
LYNN											
ICK-G											
WSNO											
271 BR									4 -		
ES215;											
O WELL											
	5	5.0	T :						5-		
RT LO		Borii	ng Terminated at 5 Feet								
O SMA											
₹T. GE											
ZEPOF											
SINAL											
A ORIG											
FROM											
SEPARATED FROM ORIGINAL REPORT. GEO SWART LOG-N		Stratificati	on lines are approximate. In-situ, the transition r	may be gradual.					1	1	
Ad Ad		ement Meth		See Exploration and Te			Notes:				
ALID I	ivianu	ıal hand au	y <del>c</del> ı	description of field and used and additional da	ta (If any).						
Ab		nment Meth	nod: I with auger cuttings upon completion.	See Supporting Informations symbols and abbreviation		tion of					
THIS BORING LOG IS NOT VALID IF	ווווטם							1			
ZING L	Z		ER LEVEL OBSERVATIONS vater encountered @ 4' BGS		<b>aco</b>		Boring Started: 12-01-2021	Boring Comp	leted: 12	-01-20	)21
IS BOF			noted @ 2' BGS		wland Ave		Drill Rig: Hand Auger	Driller: CS			
Ŧ	Savannah, GA Project No.: ES215271 Exhibit: B-3-13						3				

	BORING LOG NO. HA14 Page 1 of 1								
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: Fou	r Waters Engineering, li enville, SC		_		
SIT	ΓE:	Old Jesup Road Brunswick, GA		_	·				
90	LOCATIO	N See Exploration Plan		•				NS EL	П
GRAPHIC LOG	Latitude: 31	.2379° Longitude: -81.5315°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	AMDI E TVDE
	DEPTH							Š g	ő
· · <u>/</u> · · <u>/</u>	TOPS	<u>6OIL</u> , dark brown fine grained silty sa	nd with grass						
<u> </u>	0.6	(EV OAND (OO) 5							
	CLAY	<b>YEY SAND (SC)</b> , fine grained, dark br	own						
							1 -		
	,								
//							2 -		
							3 -		
	,								
							4 -		
///	5.0 <b>Borir</b>	ng Terminated at 5 Feet					5 –		+
		•							
	Stratification	on lines are approximate. In-situ, the transition	n may be gradual.						
	ncement Meth nual hand aug		See Exploration and Te description of field and used and additional da	laboratory procedures	Notes:				
	lonment Mething backfilled	nod: with auger cuttings upon completion.	See Supporting Informations symbols and abbreviation	ation for explanation of					
	WATE	R LEVEL OBSERVATIONS	+		Daving Chartest 40.04.0004	Davis C	-4-1 *C	04.5	20.
$\nabla$		rater encountered @ 5' BGS		acon	Boring Started: 12-01-2021	Boring Compl	eted: 12	-01-2	J21
	Mottling I	noted @ 2' BGS	2201 Ro	wland Ave	Drill Rig: Hand Auger	Driller: CS			
				nah, GA	Project No.: ES215271	Exhibit: B-3-1	4		

BORING LOG NO. HA15 Page 1 of 1								
PROJE	ECT: Brunswick-Glynn County Water Loops	North Mainland	CLIENT: Four Gree	Waters Engineering, Innville, SC	C.			
SITE:	Old Jesup Road Brunswick, GA							
의	ATION See Exploration Plan ude: 31.2368° Longitude: -81.5297°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMDI E TVDE
DEP	TH <b>TOPSOIL</b> , dark brown fine grained silty	y sand with grass				<u> </u>	WA.	0
<u>/ · ½ · ½</u> <u>½ · ½ · ½</u> 0.6	TOT SOLE, GAIN Brown line grained sity	Sand Will grass						
	SILTY SAND (SM), fine grained, dark b	prown				1 -		
						2 -	_	
						3 -		
						Ü		
						4 -	_	
5.0						5 –		
	Boring Terminated at 5 Feet					5-		
Stra	atification lines are approximate. In-situ, the trans	sition may be gradual.						
Advanceme Manual ha		See Exploration and description of field ar	Testing Procedures for a d laboratory procedures	Notes:				_
Advancement Manual has Boring ba	nt Method: ckfilled with auger cuttings upon completion.	used and additional of See Supporting Information symbols and abbrevia	nation for explanation of					
\	NATER LEVEL OBSERVATIONS	7-		Boring Started: 12-01-2021	Boring Comple	eted: 12	-01-20	121
Gro	oundwater not encountered	llen	acon		Driller: CS	7.GU. 12	-U 1 <b>-</b> ZU	
Мо	ttling not noted	2201 F	Rowland Ave annah, GA		Exhibit: B-3-15	5		

BORING LOG NO. HA16 Page 1 of 1									
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: F	our Waters Engineering, In reenville, SC	С.			
SIT	ΓE:	Old Jesup Road Brunswick, GA							
90	LOCATIO	N See Exploration Plan					$\overline{}$	EL	Щ
GRAPHIC LOG	Latitude: 31	.2303° Longitude: -81.5165°					DEPTH (Ft.)	WATER LEVEL	SAMDI E TVDE
	DEPTH							ĕĕ	ú
<u>, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,</u>		SOIL, dark brown fine grained silty sa	nd with grass						
1 () <u>. \</u>	0.6 CLAY	<b>'EY SAND (SC)</b> , fine grained, light gra	av				-		
	<u> </u>	<u></u>	-,						
							1 -		
							2 -		
							2		
							3 -		
							4 -		
	5.0						_		
<u>// // · / · · · · · · · · · · · · · · ·</u>	Borir	ng Terminated at 5 Feet					5-		
	01 115 11								
	orauncatio	on lines are approximate. In-situ, the transition	rmay be graduar.						
	ncement Meth nual hand auç		See Exploration and Te description of field and used and additional da	laboratory procedui	or a Notes:				
	lonment Mething backfilled	nod: with auger cuttings upon completion.	See Supporting Informations symbols and abbreviate		of				
	WATE	R LEVEL OBSERVATIONS			Daving Charles 40 04 0004	Davin C	late 1 40	04.5	200
$\nabla$	Groundw	rater encountered @ 5' BGS		acor	Boring Started: 12-01-2021  Drill Rig: Hand Auger	Boring Compl  Driller: CS	eted: 12	-01-2	J21
	Mottling i	noted @ 1.67' BGS	2201 Ro	wland Ave					
			Savar	nah, GA	Project No.: ES215271	Exhibit: B-3-1	Ö		

	BORING LOG NO. HA17 Page 1 of 1								
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: F	our Waters Engineering, Increenville, SC		<u> </u>		
SIT	E:	Old Jesup Road Brunswick, GA							
GRAPHIC LOG	Latitude: 31.	N See Exploration Plan .2295° Longitude: -81.5162°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
7 · 7 · 7 · 7		<b>SOIL</b> , dark brown fine grained silty sa	nd with grass						
<u>\                                    </u>	0.6 SILTY	<b>Y SAND (SM)</b> , fine grained, light brow	n				1 -		
	1.7 <u>CLAY</u>	<b>′EY SAND (SC)</b> , fine grained, light gra	ау						
CLAYEY SAND (SC), fine grained, light gray  2 -									
							3 -	-	
							4 -	$\searrow$	-
	5.0						5-		
	Borin	ng Terminated at 5 Feet							
	Stratification	on lines are approximate. In-situ, the transition	ı may be gradual.						
	cement Meth ual hand aug		See Exploration and Te description of field and used and additional da See Supporting Information	laboratory procedur a (If any).	res				
	_	with auger cuttings upon completion.	symbols and abbreviati						
$\overline{\nabla}$		R LEVEL OBSERVATIONS	75		Boring Started: 12-01-2021 B	oring Comp	leted: 12	-01-2	)21
Groundwater encountered @ 4' BGS  Mottling noted @ 2' BGS			– lierr	acor	Drill Rig: Hand Auger D	riller: CS			
	2201 Rowland Ave Savannah, GA Project No.: ES215271 Exhibit: B-3-17								

	BORING LOG NO. HA18 Page 1 of 1								
PR	OJECT:	Brunswick-Glynn County No Water Loops	orth Mainland	CLIENT: F	our Waters Engineering, In Preenville, SC				
SIT	ΓE:	Old Jesup Road Brunswick, GA							
90	LOCATIO	N See Exploration Plan					·	EL	Дd
GRAPHIC LOG	Latitude: 31	.2287° Longitude: -81.5159°					DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPI E TYPE
Ŋ	DEPTH							× S	ď
<u>, 71 /</u>		<b>SOIL</b> , dark brown fine grained silty sa	nd with grass						
<u> </u>	0.6	<b>'EY SAND (SC)</b> , fine grained, light gra	av						
	CLA	ret SAND (SC), line grained, light gra	ay						
							1 -		
							2 -		
							3 -		
							4 -		
<u>//</u>	5.0 <b>Borir</b>	ng Terminated at 5 Feet					5 –	$\bigvee$	╀
	20111	ig reminated at 0 reet							
	Stratification	on lines are approximate. In-situ, the transition	n may be gradual.						
		-1			Lv.				
	cement Meth nual hand aug		See Exploration and Te description of field and used and additional da	laboratory procedu	or a Notes: res				
	lonment Mething backfilled	od: with auger cuttings upon completion.	See Supporting Information symbols and abbreviate		n of				
	WATE	R LEVEL OBSERVATIONS	+		Davis Charles 1 40 C4 CCC	Dania - C	late 1 12	04.5	20.4
$\nabla$		rater encountered @ 5' BGS		acor	Boring Started: 12-01-2021	Boring Compl	eted: 12	-01-2	)21
	Mottling I	noted @ 0.83' BGS	l l	wland Ave		Driller: CS			
				nah, GA	Project No.: ES215271	Exhibit: B-3-1	8		

# **SUMMARY OF LABORATORY TEST RESULTS**

BORING	Depth (ft.)	Matrial Description	USCS	Water	Fine Grained Soil Classification		ssification	Coarse Grained Soil Classification				
No.	Deptii (it.)	Matrial Description	0303	Content (%)		PL	PI	% Gravel	% Sand	% Fine	Сс	Cu
B02	6-8	POORLY GRADED SAND WITH SILT	SP-SM	6.7				0.2	92.9	6.9	1.14	2.34
B03	4-6	CLAYEY SAND	SC	29.8	30	16	14		54.1	45.9		
B04	6-8	CLAYEY SAND	SC	33.0					79.6	20.4		
B05	8-10	POORLY GRADED SAND WITH SILT	SP-SM	23.7					93.3	6.7	0.87	2.18
B06	4-6	POORLY GRADED SAND	SRADED SAND SP 27.4				95.7	4.3	0.95	2.1		
B07	2-4	POORLY GRADED SAND WITH SILT	SP-SM	12.2					92.8	7.2	0.92	2.18
B08	13.5-15	POORLY GRADED SAND	SP	25.3					96.8	3.2	1.25	2.27
B09	18.5-20	18.5-20 POORLY GRADED SAND SP 29.0						98.6	1.4	0.87	2.03	
	<del>                                     </del>											
		I Glynn County North Mainland						PROJECT I	NUMBER: E	ES215271	<u>l</u>	<u>l</u>
	Water Loops				واا			CLIENT: Fo	our Waters I	Engineering.	Inc.	
SITE: Brur	nswick, GA			2201 Rowl	and Ave Savan	Ave Savannah, GA Greenville, SC						

### **GRAIN SIZE DISTRIBUTION**

**ASTM D422 / ASTM C136** 

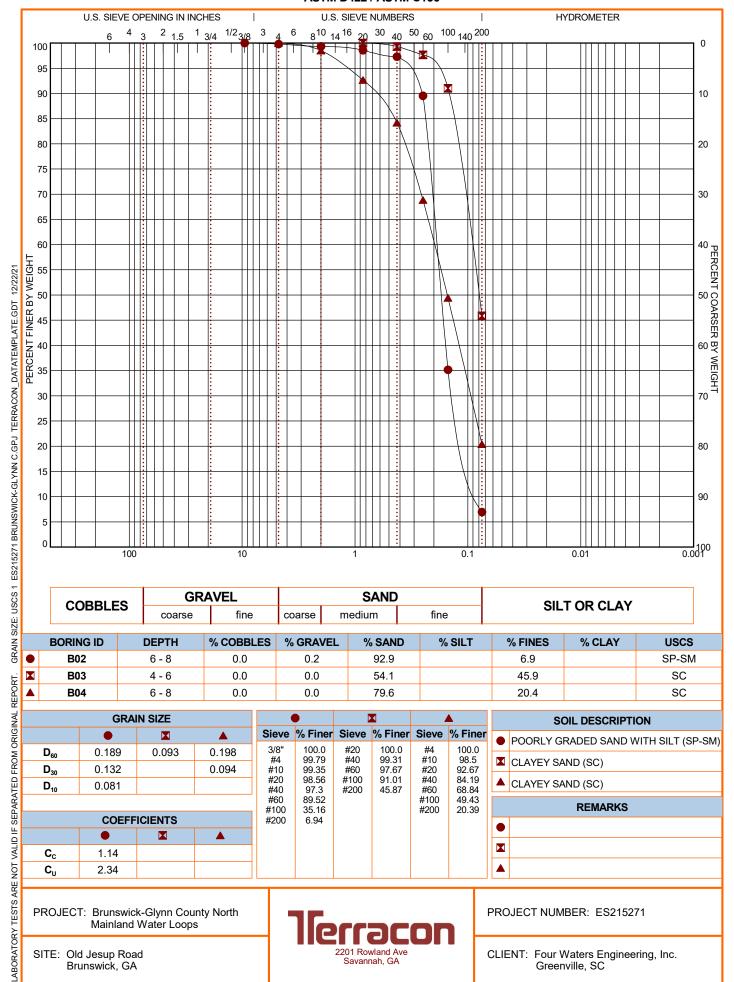
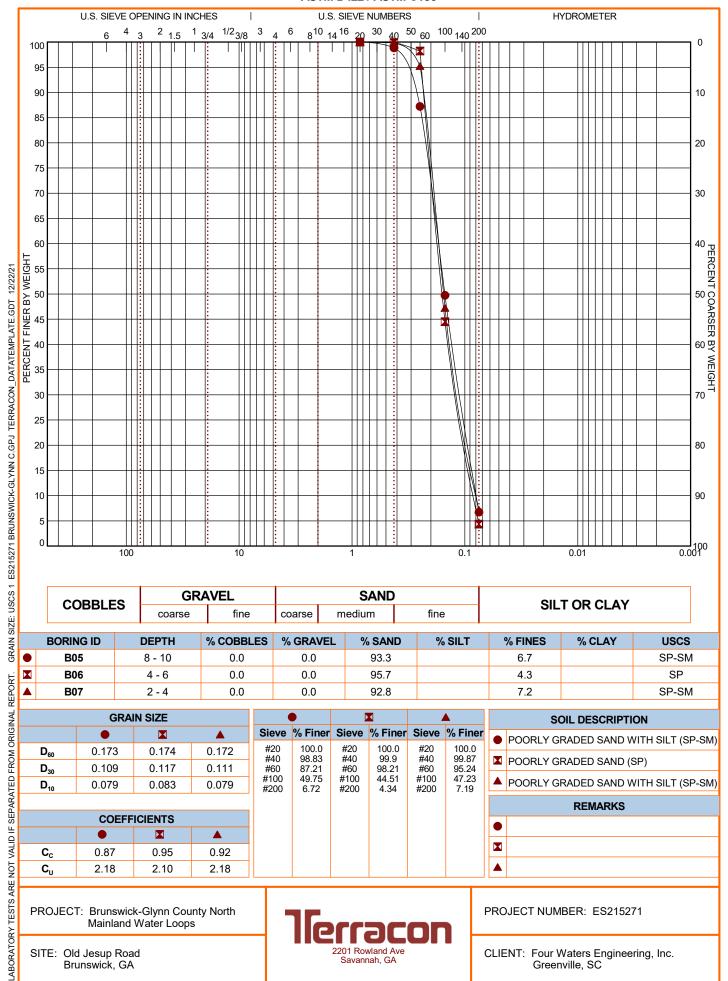


Exhibit B-5-1

### **GRAIN SIZE DISTRIBUTION**

**ASTM D422 / ASTM C136** 



### **GRAIN SIZE DISTRIBUTION**

**ASTM D422 / ASTM C136** 

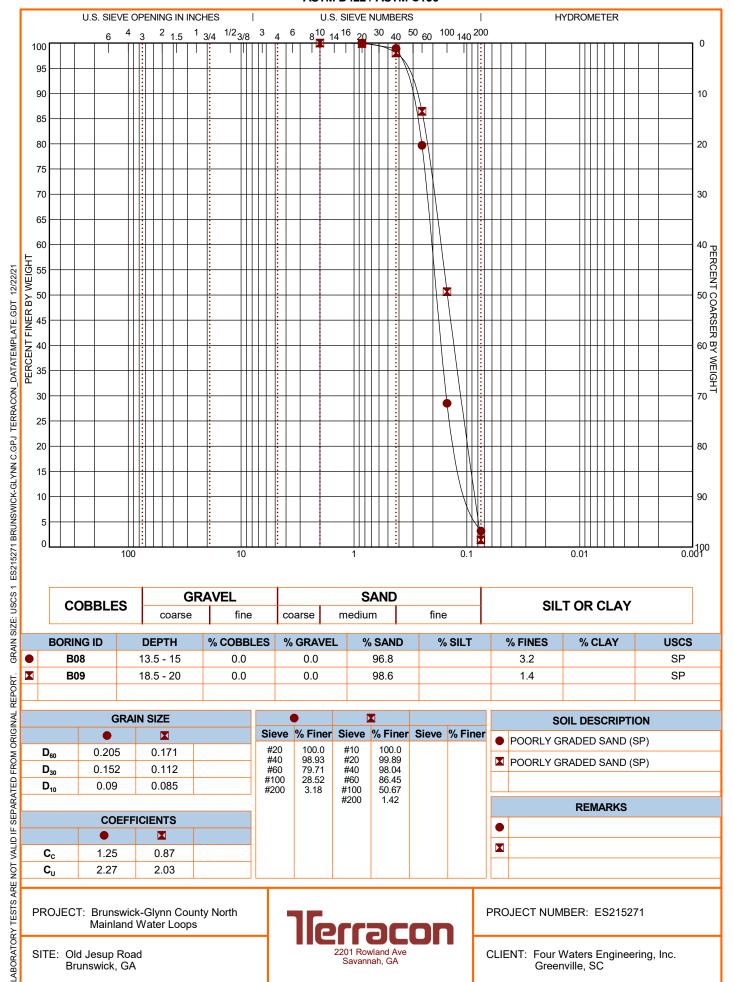
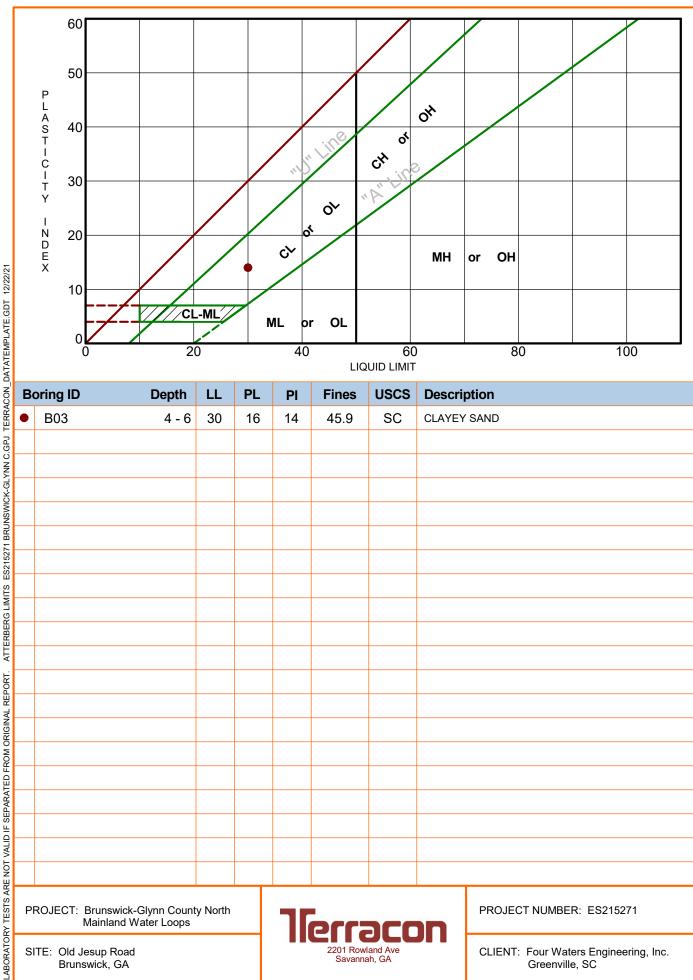


Exhibit B-5-3

## ATTERBERG LIMITS RESULTS

**ASTM D4318** 



2 2	В	oring ID	Depth	LL	PL	PI	Fines	USCS	Description
1 1 1 1	•	B03	4 - 6	30	16	14	45.9	SC	CLAYEY SAND
2									
ا ا									
אַ-פר פר									
ATTERBERG LIMITS ESZISZ/ BRUNSWICK-GLYNN C.GPJ TERRACON									
פאס									
17617									
ν ΓΩ									
ABER P									
- E									
<u>۔</u> بے									
도 전									
SINAL BINAL									
کا ا									
ב ה									
TAA T									
, VEP.									
ACID									
2									
S ARE NOT VALID IF SEPARALED FROM ORIGINAL REPORT.									

PROJECT: Brunswick-Glynn County North Mainland Water Loops

SITE: Old Jesup Road Brunswick, GA



PROJECT NUMBER: ES215271

CLIENT: Four Waters Engineering, Inc. Greenville, SC

## **EXHIBIT C**

## **SUPPORTING INFORMATION**

**Exhibit C-1:** Seismic Design Parameters

**Exhibit C-2:** General Notes

**Exhibit C-3:** Unified Soil Classification System

### Seismic Design Parameters Based on IBC2018 Code and ASCE 7-16 Standard

Terracon Project Name: Brunswick-Glynn County North Mainland Water Loops

Terracon Project No: ES215271

Site Location: Brunswick, GA

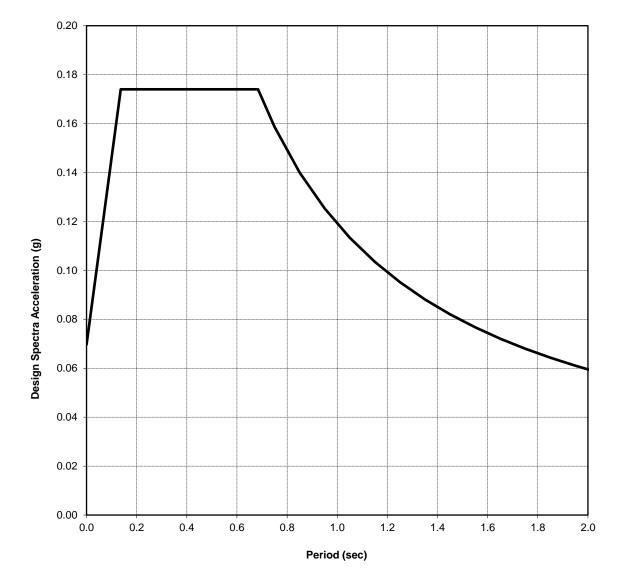
**Latitude :** 31.2366° **Longitude :** -81.5291°

Site Class: D

**Design Response Spectrum for the Site Class** 

$S_s = 0.163$	$S_1 = 0.074$
$F_a = 1.600$	$F_v = 2.400$
$S_{MS} = 0.262$	$S_{M1} = 0.179$
$S_{DS} = 0.174$	$S_{D1} = 0.119$

	Period (sec)	<u>Sa (g)</u>
	0.000	0.070
$T_0 =$	0.137	0.174
	0.200	0.174
$T_S =$	0.684	0.174
T =	0.750	0.159
	0.850	0.140
	0.950	0.125
	1.050	0.113
	1.150	0.103
	1.250	0.095
	1.350	0.088
	1.450	0.082
	1.550	0.077
	1.650	0.072
	1.750	0.068
	1.850	0.064
	1.950	0.061
	2.050	0.058
	2.150	0.055



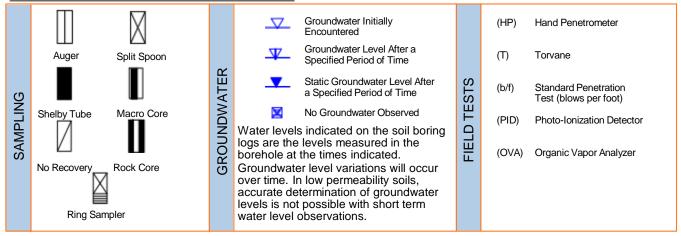
Responsive ■ Resourceful ■ Reliable

**Exhibit C-1** 

-llerracon



### DESCRIPTION OF SYMBOLS AND ABBREVIATIONS



#### DESCRIPTIVE SOIL CLASSIFICATION

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

#### LOCATION AND ELEVATION NOTES

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	(More than 50% re Density determined by S	TY OF COARSE-GRAINED SOILS etained on No. 200 sieve.) tandard Penetration Resistance yels, sands and silts.	CONSISTENCY OF FINE-GRAINED SOILS (50% or more passing the No. 200 sieve.) Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance				
RMS	Descriptive Term Std. Penetration Resistance (blows per foot)		Descriptive Term (Consistency)	Undrained Shear Strength (kips per square foot)	Std. Penetration Resistance (blows per foot)		
뿌	Very Loose	0 - 3	Very Soft	less than 0.25	0 - 1		
NGT	Loose	4 - 9	Soft	0.25 to 0.50	2 - 4		
REN	Medium Dense	10 - 29	Medium-Stiff	0.50 to 1.00	5 - 7		
S	Dense	30 - 50	Stiff	1.00 to 2.00	8 - 14		
	Very Dense	> 50	Very Stiff	2.00 to 4.00	15 - 30		
			Hard	above 4.00	> 30		

#### RELATIVE PROPORTIONS OF SAND AND GRAVEL

Descriptive Term(s) of other constituents	Percent of Dry Weight	<u>Descriptive Term(s)</u> of other constituents	Percent of Dry Weight
Trace With Modifier	< 15 15 - 29 > 30	Boulders Cobbles Gravel Sand Silt or Clay	Over 12 in. (300 mm) 12 in. to 3 in. (300mm to 75mm) 3 in. to #4 sieve (75mm to 4.75 mm) #4 to #200 sieve (4.75mm to 0.075mm Passing #200 sieve (0.075mm)

### RELATIVE PROPORTIONS OF FINES

Percent of
Dry Weight_
< 5
5 - 12
> 12

### PLASTICITY DESCRIPTION

Term	Plasticity Index
Non-plastic	0
Low	1 - 10
Medium	11 - 30
High	> 30

**GRAIN SIZE TERMINOLOGY** 



		Soil Classification				
Criteria for Assigni	riteria for Assigning Group Symbols and Group Names Using Laboratory Tests A					
Coarse-Grained Soils: More than 50% retained on No. 200 sieve	Gravels: More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels:	Cu <sup>3</sup> 4 and 1 £ Cc £ 3 E		GW	Well-graded gravel F
		Less than 5% fines <sup>C</sup>	Cu < 4 and/or [Cc<1 or Cc>3.0] E		GP	Poorly graded gravel <sup>F</sup>
		Gravels with Fines:	Fines classify as ML or N	ИΗ	GM	Silty gravel F, G, H
		More than 12% fines <sup>C</sup>	Fines classify as CL or CH		GC	Clayey gravel <sup>F, G, H</sup>
	Sands: 50% or more of coarse fraction passes No. 4 sieve	Clean Sands:	Cu <sup>3</sup> 6 and 1 £ Cc £ 3 E		SW	Well-graded sand I
		Less than 5% fines D	Cu < 6 and/or [Cc<1 or C	Cc>3.0] E	SP	Poorly graded sand
		Sands with Fines: More than 12% fines D	Fines classify as ML or MH		SM	Silty sand <sup>G, H, I</sup>
			Fines classify as CL or CH		sc	Clayey sand <sup>G, H, I</sup>
Fine-Grained Soils: 50% or more passes the No. 200 sieve	Silts and Clays: Liquid limit less than 50	Inorganic:	PI > 7 and plots on or above "A"		CL	Lean clay <sup>K</sup> , <sup>L, M</sup>
			PI < 4 or plots below "A"	line <sup>J</sup>	ML	Silt K, L, M
		Organic:	Liquid limit - oven dried	< 0.75 OL	Organic clay K, L, M, N	
			Liquid limit - not dried		OL	Organic silt <sup>K</sup> , <sup>L</sup> , <sup>M</sup> , <sup>O</sup>
	Silts and Clays: Liquid limit 50 or more	Inorganic:	PI plots on or above "A" line		СН	Fat clay <sup>K, L, M</sup>
			PI plots below "A" line		MH	Elastic Silt K, L, M
		Organic:	Liquid limit - oven dried	< 0.75 O	ОН	Organic clay <sup>K, L, M, P</sup>
			Liquid limit - not dried		OH	Organic silt K, L, M, Q
Highly organic soils:	Primarily organic matter, dark in color, and organic odor				PT	Peat

- A Based on the material passing the 3-inch (75-mm) sieve.
- If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.
- Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
- D Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.

E 
$$Cu = D_{60}/D_{10}$$
  $Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ 

- F If soil contains <sup>3</sup> 15% sand, add "with sand" to group name.
- <sup>G</sup> If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

- $\ensuremath{^{\text{H}}}\xspace$  If fines are organic, add "with organic fines" to group name.
- If soil contains 3 15% gravel, add "with gravel" to group name.
- J If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.
- K If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.
- └ If soil contains <sup>3</sup> 30% plus No. 200 predominantly sand, add "sandy" to group name.
- MIf soil contains <sup>3</sup> 30% plus No. 200, predominantly gravel, add "gravelly" to group name.
- NPI <sup>3</sup> 4 and plots on or above "A" line.
- OPI < 4 or plots below "A" line.
- P PI plots on or above "A" line.
- <sup>Q</sup>PI plots below "A" line.

