PUMP STATION 2032 REGIONAL FORCEMAIN IMPROVEMENTS ST. SIMONS ISLAND, GEORGIA

ATTACHMENT

Report of Geotechnical Exploration Brunswick-Glynn County JWSC Forcemain



Geotechnical = Construction Materials = Environmental = Facilities

REPORT OF GEOTECHNICAL EXPLORATION BRUNSWICK-GLYNN COUNTY JWSC FORCEMAIN ST. SIMONS ISLAND, GEORGIA E&A PROJECT NO. 35-24198 CLIENT ID: 5117

Prepared for:

Four Waters Engineering, Inc. 324 6th Avenue North Jacksonville Beach, Florida 32250

Prepared by:

Ellis & Associates, Inc. 7064 Davis Creek Road Jacksonville, Florida 32256

September 12, 2016



Geotechnical
Construction Materials
Environmental
Facilities

September 12, 2016

Ms. Angela Bryan, P.E. Four Waters Engineering, Inc. 324 6th Avenue North Jacksonville Beach, Florida 32250

Reference: Report of Geotechnical Exploration Brunswick-Glynn County JWSC Forcemain St. Johns County, Florida E&A Project No. 35-24198 Client ID: 5117

Dear Ms. Bryan:

Ellis & Associates, Inc. has completed a geotechnical exploration for the subject project in accordance with our proposal last revised August 11, 2016. The exploration was conducted to evaluate the general subsurface conditions within the proposed pipeline areas and to provide recommendations for site preparation, pipeline support, and Horizontal Direction Drill recommendations.

We appreciate this opportunity to be of service as your geotechnical consultant on this phase of the project and look forward to providing the materials testing and observation that will be required during the construction phase. If you have any questions, or if we may be of any further service, please contact us.

Very truly yours, **ELLIS & ASSOCIATES, INC.**

Colin A. Shaw, E.I. Staff Engineer Robert W. Clark, P.E. Senior Project Engineer Registered, Georgia No. 19985

Distribution: Ms. Angela Bryan, P.E. – Four Waters Engineering, Inc. 1 pdf



TABLE OF CONTENTS

Subject

Page No.

1.0	PROJECT INFORMATION1
1.1	Site Location and Description1
1.2	Project Description1
2.0	FIELD EXPLORATION1
2.1	SPT Borings1
3.0	VISUAL CLASSIFICATION
4.0	GENERAL SUBSURFACE CONDITIONS1
4.1	General Soil Profile1
4.2	Groundwater Level
5.0	DESIGN RECOMMENDATIONS
5.1	General
5.2	Horizontal Directional Drilling Recommendations
6.0	EARTHWORK RECOMMENDATIONS
6.1	Clearing
6.2	Temporary Groundwater Control
6.3	Preparation of Foundation Soils
6.4	Excavation Protection
6.5	Compaction of Bottom of Excavation
6.6	Structural Backfill and Compaction of Structural Backfill4
7.0	QUALITY CONTROL
8.0	REPORT LIMITATIONS

FIGURES

Figure 1	Site Location Plan
Figure 2	Field Exploration Plan
Figure 3	Generalized Subsurface Profiles

APPENDICES

Appendix A	Soil Boring Logs
	Field Exploration Procedures
	Key to Soil Classification



1.0 PROJECT INFORMATION

1.1 Site Location and Description

The project site is generally located along Frederica Road and Palmetto Street in St. Simons Island, Georgia. The general site location is shown as Figure 1. The surrounding areas generally consist of residential and commercial properties as well as golf courses.

1.2 Project Description

Project information has been provided to us in discussions with you. We have been provided with a copy of a site plan for the subject site indicating the route of the proposed forcemain. This plan showed the boundary limits for the project, the existing roadways adjacent to the site, and the requested boring locations.

We understand that the Brunswick-Glynn Joint Water & Sewer Commission is proposing to install a 16-inch wastewater forcemain on St. Simons Island, Georgia. The line will be approximately 10,800 feet long in total, and it will be constructed along Frederica Road from North Harrington Road to Palmetto Avenue. The forcemain will then extend along Palmetto Avenue to the Dunbar Creek Wastewater Treatment Plant. We further understand that the installation of the pipeline will be by Horizontal Direction Drill (HDD) methods with the possibility of installation by open-cut methods in select areas.

If the project information above is incorrect, then the recommendations in this report may need to be re-evaluated. Any changes in these conditions should be provided so the need for re-evaluation of our recommendations can be assessed.

2.0 FIELD EXPLORATION

A field exploration was performed during the period of September 6 to 9, 2016. The approximate boring locations are shown on the Field Exploration Plan, Figure 2. The approximate boring locations were determined in the field by our personnel using paced measurements from existing site features. This method should be considered accurate only to the degree implied by the methods used.

2.1 SPT Borings

To explore the subsurface conditions within the areas of the proposed pipeline, we located and performed 6 Standard Penetration Test (SPT) borings drilled to depths of approximately 25 feet below the existing ground surface. The borings were performed in general accordance with the methodology outlined in ASTM D 1586. Split-spoon soil samples recovered during performance of the borings were visually classified in the field and representative portions of the samples were transported to our laboratory for further evaluation. A summary of the field procedures is included in Appendix A.

3.0 VISUAL CLASSIFICATION

A geotechnical engineer classified representative soil samples obtained during our field exploration using the Unified Soil Classification System (USCS) in general accordance with ASTM D 2488. A Key to the Soil Classification System is included in Appendix A.

4.0 GENERAL SUBSURFACE CONDITIONS

4.1 General Soil Profile

Graphical presentations of the generalized subsurface conditions are presented on Figures 3 through 4. Detailed boring records are included in Appendix A. When reviewing these records it should be understood that the soil conditions will vary between the boring locations. The following paragraph summarizes the soil conditions encountered.

In general, the borings mostly encountered a surficial layer of topsoil underlain by layers of loose to medium dense fine sands and fine sands with silt (SP, SP-SM) to the boring termination depths of approximately 25 feet below the existing ground surface. Some borings encountered deeper layers of loose to medium dense silty fine sand (SM). Boring B1 encountered a layer of very loose silty fine sand with many organic fines (PT) between 2½ and 4½ feet below the existing ground surface. Boring B6 encountered a layer of very loose very clayey fine sand (SC) between approximately 12 and 17 feet below the existing ground surface.

4.2 Groundwater Level

The groundwater level was encountered at each boring location and recorded at most locations, at the time of drilling, at depths varying from 3.2 to 5.3 feet below the existing ground surface. The depth to the groundwater level at the boring locations is noted on the Generalized Subsurface Profiles and on the Log of Boring records. However, it should be anticipated the groundwater level will fluctuate due to seasonal climatic variations, surface water runoff patterns, construction operations, and other interrelated factors.

5.0 DESIGN RECOMMENDATIONS

5.1 General

Our geotechnical engineering evaluation of the site and subsurface conditions with respect to the planned construction and our recommendations for site preparation and pipeline support are based on (1) our site observations, (2) the field data obtained, and (3) our understanding of the project information as presented in this report.

Should the location of the pipeline be significantly changed, please contact us so that we can review our recommendations. Also, the discovery of any site or subsurface conditions during construction which deviate from the data obtained during this geotechnical exploration should also be reported to us for our evaluation.

The recommendations presented in the subsequent sections of this report present design and construction techniques which are appropriate for the planned construction. We recommend that we be provided the opportunity to review the final plans and earthwork specifications to verify that our recommendations have been properly interpreted and implemented.

5.2 Horizontal Directional Drilling Recommendations

We understand the proposed pipeline may be installed using HDD techniques. The borings generally encountered sandy soils throughout the subsurface profiles. It is our opinion these soil types will be conducive to the HDD operations.

The HDD operations should be performed in accordance with Section 615 of the Georgia Department of Transportation (GDOT) *Standard Specifications Construction of Transportation Systems*, (latest edition). Any soil conditions encountered that are not consistent with those contained within this report should be reported to E&A for our evaluation.

Prior to construction, the location of existing underground utility lines within the construction area should be established. Provisions should then be made to relocate interfering utilities to appropriate locations.

6.0 EARTHWORK RECOMMENDATIONS

If there are sections of the pipeline that are open excavations, earthwork as outlined in this section should be performed to provide more uniform foundation bearing conditions and to reduce the potential for post-construction settlements of the planned structures.



6.1 Clearing

Prior to construction, the location of existing underground utility lines within the construction area should be established. Provisions should then be made to relocate interfering utilities to appropriate locations. It should be noted that if underground pipes are not properly removed or plugged, they may serve as conduits for subsurface erosion which may subsequently lead to excessive settlement of overlying pavement.

6.2 Temporary Groundwater Control

The groundwater level was encountered at the boring locations at depths varying from 3.2 to 5.3 feet below the existing ground surface at the time of our exploration. Because of the need for excavation to the pipeline bearing levels followed by compaction of the soils within the upper one foot below the exposed surface, it may be necessary to install temporary groundwater control measures to dewater the area to facilitate the excavation and compaction processes. The groundwater control measures should be determined by the contractor. The water table should be maintained at least 2 feet below the required depth of excavation. The dewatering system should not be decommissioned until sufficient deadweight exists on the structures to prevent uplift or the uplift protection system as described above, if necessary, is in place.

The fine sands and fine sands with silt in excavations up to a depth of 15 feet can usually be dewatered by conventional methods such as well pointing or rim ditching (for shallow excavations). It should be noted, however, that cuts on that order may expose confined aquifers where relatively permeable sandy soils underlie less permeable zones of clayey fine sand. Therefore, it is recommended that these relatively permeable zones be dewatered to stabilize the excavation and preclude uplift boiling of the overlaying clayey zones.

6.3 Preparation of Foundation Soils

For pipelines which are anticipated to bear in sandy soils (SP and SP-SM), the soils should be excavated to the proposed bearing elevation and the exposed excavation surface should be compacted as outlined in Section 6.5. Once the pipe is installed, the trench should be backfilled with compacted structural backfill to final grade.

We recommend one of the following alternatives be implemented to provide more uniform bearing conditions, and to reduce the potential for post construction settlements of the pipelines which bear in the loose clayey soils as encountered in Boring B6 between approximately 12 and 17 feet below the existing ground surface and the loose organic soils as encountered in Boring B1 between approximately 2½ and 4½ feet below the existing ground surface.

- 1. It is recommended that at least one foot of the clayey soils (SC) below the pipeline inverts, if encountered at the time of excavation, be over-excavated and replaced with compacted structural backfill to final bearing elevations. If encountered at the structure's bearing level, organic soils (A-8) should be completely removed below the structures and replaced with compacted structural fill. Compacted structural fill should then be placed around and above structures and pipelines to final grade.
- 2. Alternatively, to reduce the amount of structural fill and over-excavation, it is recommended that a medium duty woven geotextile such as a Mirafi 600X, or equivalent, be used as barrier between compacted fill and clayey materials. If a woven geotextile is used, the amount of over-excavation can be waived for the pipeline. The geotextile should be placed in the excavation bottom and sides above the clayey soils creating a barrier between the clayey soils and structural backfill to preclude contamination of the backfill. A compacted structural fill

material should then be used to backfill to the final bearing elevation and around and above structures and pipelines to final grade.

3. As an alternative to providing compacted backfill to replace the over-excavated soils below the mid-height of the pipe, flowable fill can be utilized. Flowable fill commonly consists of a mixture of cement, fly ash, fine aggregate, and water. Design, placement, and testing of flowable fill should be in accordance with the latest edition of Georgia Department of Transportation Standard Specifications Construction of Transportation Systems, Section 600. Additionally, if flowable fill is utilized, the geotextile can be eliminated from the design.

6.4 Excavation Protection

Excavation work will be required to meet OSHA Excavation Standard Subpart P regulations, Type C Soils. A trench box or braced sheet pile structure is anticipated for excavation support. The support structure should be designed according to OSHA sheeting and bracing requirements. We recommend a Georgia registered Professional Engineer design the sheeting/bracing system. Use of an open cut is not considered practical because it will require a large excavation to preclude slope sloughing.

6.5 Compaction of Bottom of Excavation

After installing the temporary groundwater control measures, and achieving the required depth of excavation, the exposed surface of sandy soils should be compacted by the use of hand-operated equipment. If clayey soils are at the exposed surface, it is recommended the initial fill layer be placed on top of the exposed surface subsequent to the necessary overexcavation or geotextile placement, then compacted. Typically, the material should exhibit moisture contents within ± 2 percent of the Modified Proctor optimum moisture content (AASHTO T-180) during the compaction operations. Compaction should continue until densities of at least 95 percent of the Modified Proctor maximum dry density (AASHTO T-180) have been achieved within the upper one foot below the exposed surface within the pipeline excavation.

Should the bearing level soils experience pumping and soil strength loss during the compaction operations, compaction work should be immediately terminated and (1) the disturbed soils removed and backfilled with dry structural fill soils which are then compacted, or (2) the excess moisture content within the disturbed soils allowed to dissipate before recompacting.

6.6 Structural Backfill and Compaction of Structural Backfill

Structural backfill within the pipeline excavations, and in areas in which over-excavation of unsuitable soils is required below the pipeline invert elevation, should be placed in loose lifts not exceeding six inches in thickness and compacted by the use of hand-operated compaction equipment. Structural backfill is defined as a non-plastic, inorganic, granular soil having less than 10 percent material passing the No. 200 mesh sieve and containing less than 4.0 percent organic material. The sandy soils (SP and SP-SM) excavated for the structure may be used as backfill. However, it should be anticipated that the clayey soils (SC) may be difficult to dry and compact due to an inherent nature to retain moisture. Typically, the backfill material should exhibit moisture contents within ± 2 percent of the Modified Proctor optimum moisture content (AASHTO T-180) during the compaction operations. Compaction should continue until densities of at least 95 percent of the Modified Proctor maximum dry density (AASHTO T-180) have been achieved within each 6-inch thick lift of the compacted structural backfill.

Because the clayey soils (SC) have an excessive fines content (i.e., greater than 15 percent), and a tendency to retain moisture which makes these soils very difficult to dry and compact, we recommend these soils not be used as structural backfill. These soils are unsuitable for use as backfill materials.



7.0 QUALITY CONTROL

A representative number of field in-place density tests should be performed in each 6-inch thick lift of compacted backfill and in the upper 12 inches below the bearing levels along the pipeline alignment. The density tests are considered necessary to verify that satisfactory compaction operations have been performed. We recommend density testing be performed at one location for every 300 feet of pipeline.

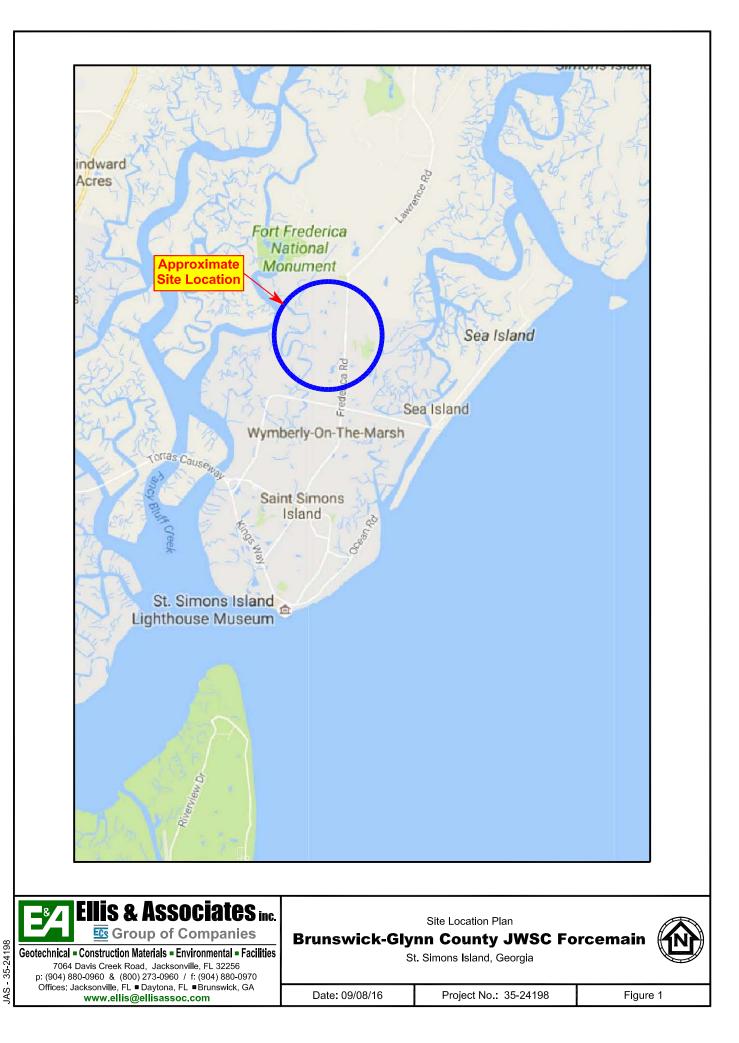
8.0 **REPORT LIMITATIONS**

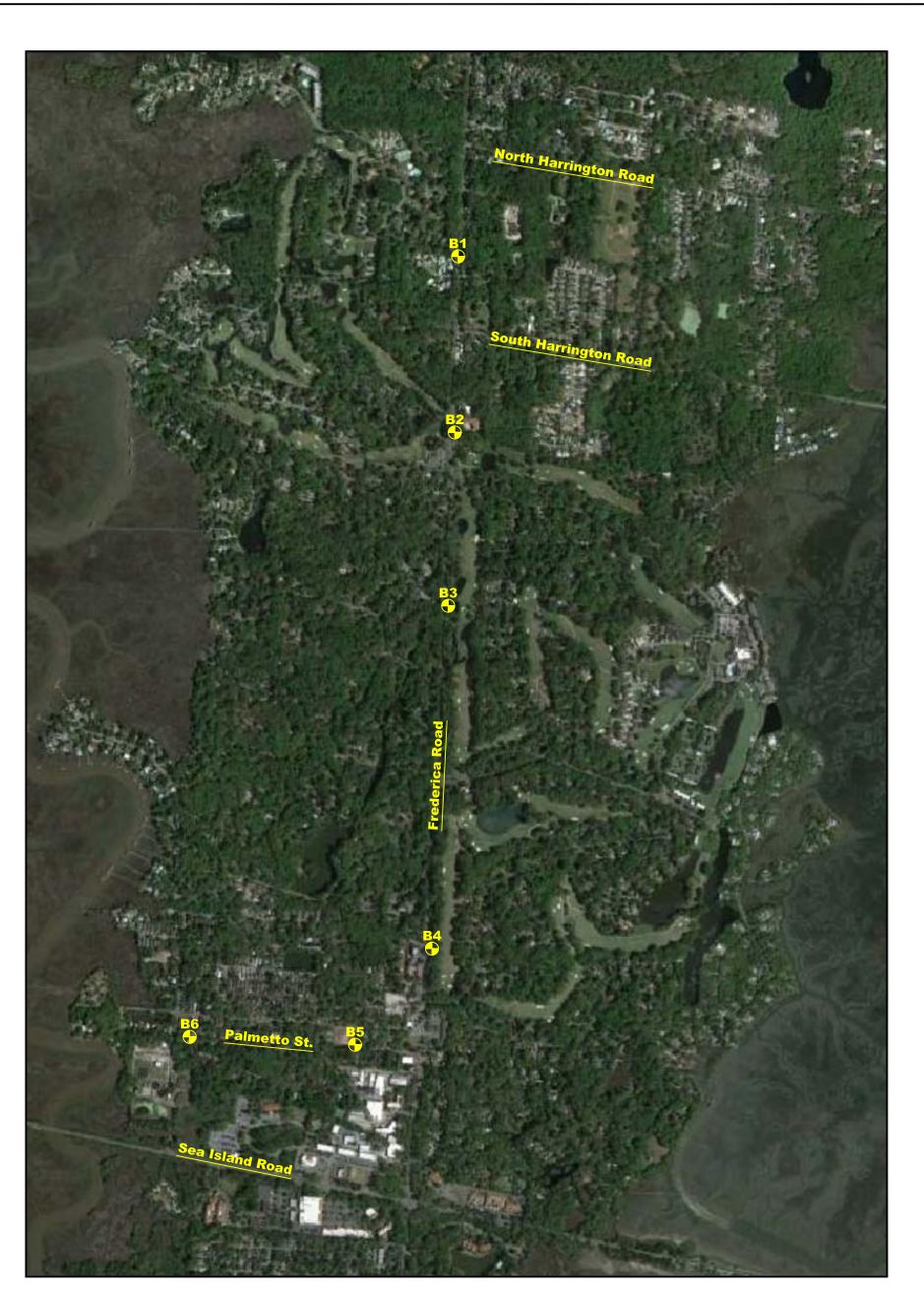
Our geotechnical exploration has been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. Ellis & Associates, Inc. is not responsible for any independent conclusions, interpretation, opinions or recommendations made by others based on the data contained in this report.

Our scope of services was intended to evaluate the soil conditions within the zone of soil influenced by the pipeline bearing conditions. Our scope of services does not address geologic conditions such as sinkholes or soil conditions existing below the depth of the soil borings.

This report does not reflect any variations which may occur adjacent to or between soil borings. The discovery of any site or subsurface condition during construction which deviate from the data obtained during this geotechnical exploration should be reported to us for our evaluation. Also, in the event of any change to the location of the pipeline alignment, please contact us so that we can review our recommendations. We recommend that we be provided the opportunity to review the earthwork specifications to verify that our recommendations have been properly interpreted and implemented.

FIGURES

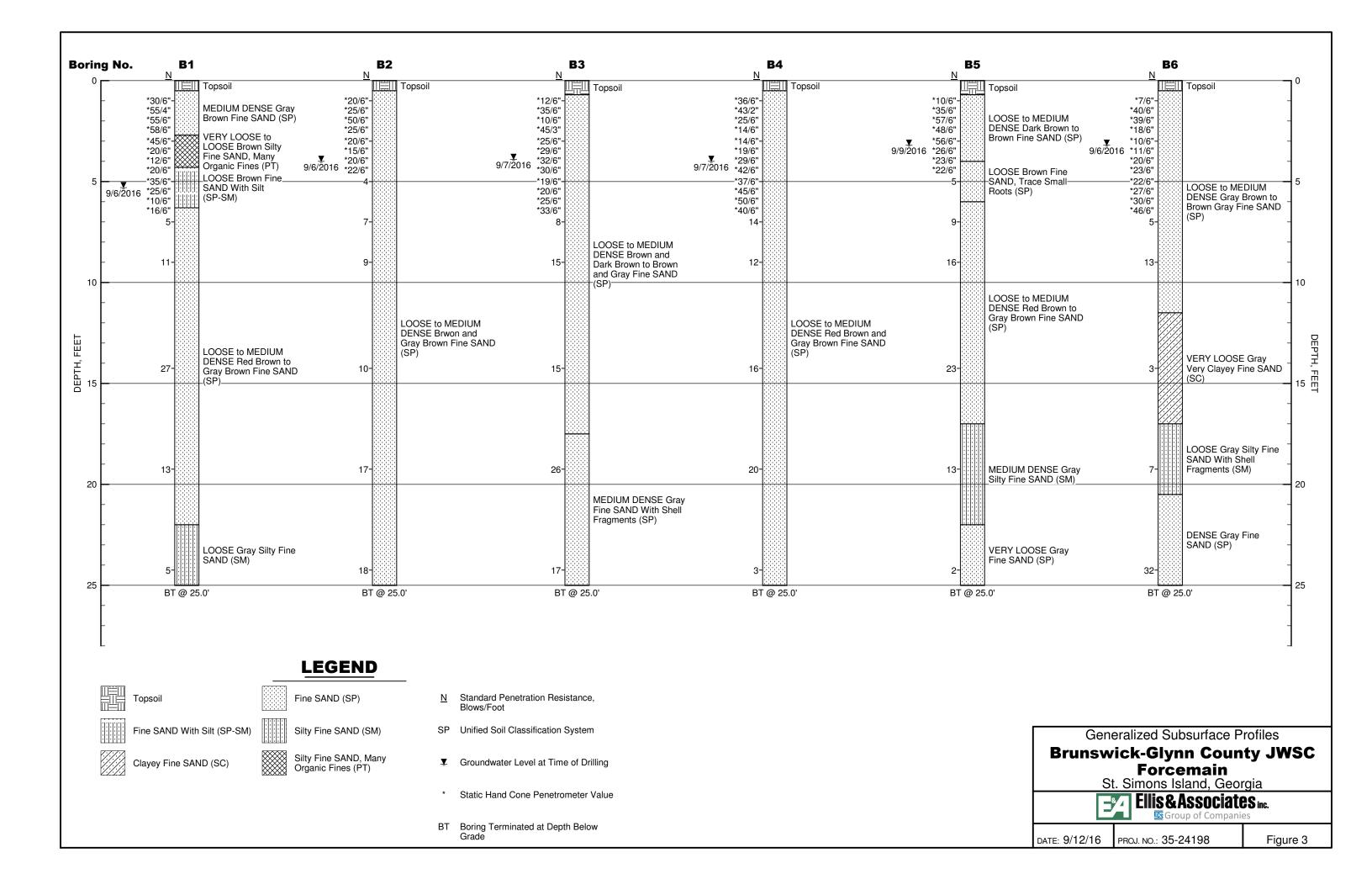




LEGEND

 Approximate Location of Standard Penetration Test (SPT) Boring

Ellis & Associates Inc. Group of Companies Geotechnical - Construction Materials - Environmental - Facilities 7064 Davis Creek Road, Jacksonville, FL 32256 p: (904) 880-0960 & (800) 273-0960 / f: (904) 880-0970	Brunswick-	Field Exploration Plan Glynn County JWSC F St. Simons Island, Georgia	orcemain	Graphical Scale
Offices: Jacksonville, FL ■ Daytona, FL ■ Brunswick, GA www.ellis@ellisassoc.com	Date: 08/06/16	Project No.: 35-24198	Figure 2	0' 800' 1,600'



APPENDIX A

SOIL BORING LOGS FIELD EXPLORATION PROCEDURES KEY TO SOIL CLASSIFICATION



 Project No.:
 35-24198

 Boring No.:
 B1

 Sheet
 1
 of
 1

Project	Project: Brunswick-Glynn County JWSC Forcemain Client: Four Waters Engineering, Inc. Drill Rig: ATV Driller: C. Morgan													
Boring	Boring Location: See Field Exploration Plan Drill Rod: AWJ Drill Mud: Super Gel-X													
		-			Cas	ing Siz	ze:			Length	of Ca	sing:		
Ground	lwater De	pth: <u>5.3 ft</u> Time: _	Drilling Date	e: <u>9/6/16</u>	Bor	ing Be	gun:	<u>9/6/1</u>	6	Boring	Comp	oleted: <u>9/6/16</u>		
SAMPLE NO.	0 DEPTH, FEET		N	<u> </u>	BLOWS PER 6 IN.	N Value	PERCENT ORGANIC MATERIAL	PERCENT PASSING NO. 200 SIEVE		+ MOISTURE + CONTENT 0 (%) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		SHEAR STRENGTH (ksf) • Pocket Penetrometer Undisturbed Sample • Pocket Penetrometer Disturbed Sample • Torvane • Unconfined Compression • Triaxial Compression • 1 • 2 • 1 • 4 • 4 • 4		
1			ay Brown Fine SAND ((SP)	*30/6" *55/4" *55/6" *58/6" *45/6"									
2		Many Organic Fines (*20/6" *12/6" *20/6" *35/6"									
3	5		AND With Silt (SP-SM	¥	*25/6" *10/6" *16/6" 2									
4		LOOSE to MEDIUM SAND (SP)	DENSE Red Brown Fir	ne	2 3 4	5								
5	10				4 5 6 7	11								
6					7 13 14	27								
COG OF BORING 35-24198.GPU ELLIS ASSOCIATES.GD1 9/2/16	20		ay Brown Fine SAND (SP)	5 5 8	13								
ORING 35-24198.GF &	25	LOOSE Gray Silty Fin			2 2 3	5								
		Boring Te atic Hand Cone Penetrom	erminated @ 25 ft. eter Value.											



Project	No.:	35-24198	3
Boring	No.:	B2	
Sheet	1	of	1

Project	t: <u>Brunsw</u>	ick-Glynn County JWSC Forcemain		Clie	nt: <u>F</u>	our W	aters	Enginee	ering, Inc.	C Mar				
Boring	Boring Location: See Field Exploration Plan Drill Rig: ATV Driller: C. Morgan Drill Rod: AWJ Drill Mud: Super Gel-X Casing Size: Length of Casing:													
			0/6/16	Cas	ing Siz	ze:	0/6/1	6						
Ground	dwater Dep	th: <u>4 ft</u> Time: <u>Drilling</u> Date	e: <u>9/6/16</u>	Bor	ing Be	egun:	9/0/1	0	_ Boring G	Î	ed: <u>9/6/16</u> HEAR STRENGTH			
SAMPLE NO.	O DEPTH, FEET		6 - 1444	BLOWS PER 6 IN.	N Value	PERCENT ORGANIC MATERIAL	PERCENT PASSING NO. 200 SIEVE	OPLASTIC LIMIT	+ MOISTURE + 000000000000000000000000000000000000		(ksf) Pocket Penetrometer Undisturbed Sample Pocket Penetrometer Disturbed Sample Torvane Unconfined Compression Triaxial Compression			
1		Topsoil LOOSE to MEDIUM DENSE Brown Fine SA (SP)	AND III	*20/6" *25/6" *50/6" *25/6"										
2		LOOSE Gray Brown Fine SAND (SP)	.	*20/6" *15/6" *20/6" *22/6"										
3	5			2 2 3 3	4									
4				3 4 5 4	7									
5	10			4 5 5	9									
		LOOSE Brown Fine SAND (SP)		2										
6	15			4 6	10									
201 BOKING 35-24198.6PJ ELLIS ASSOCIATES.6D1 9/12/16	20	MEDIUM DENSE Gray Brown Fine SAND (SP)	6 7 10	17									
KING 35-24198.GPJ ELLI				8 9 9	18									
Rema	- 25 -	Boring Terminated @ 25 ft. ic Hand Cone Penetrometer Value.		•		•		_ · · · ·						



 Project No.:
 35-24198

 Boring No.:
 B3

 Sheet
 1
 of
 1

Project: Brunswick-Glynn County JWSC Forcemain Client: Four Waters Engineering, Inc. Drill Rig: ATV Driller: C. Morgan																		
Boring	Location	: :	See Field Explorati	on Plan			Dril	l Rod:	AW	J			Dr	ill Mı	ıd:	Super G	el-X	
					Data	0/7/16	Cas	ng Siz	ze:	0/7/1	6		Le	ngth o	of Ca	sing:	0/7/16	
SAMPLE NO.	FEET	SAMPLE TYPE	: <u>3.9 ft</u> Time:	Drilling	Date:	9/7/16	BLOWS PER 6 IN.	ng Be	PERCENT ORGANIC MATERIAL	PERCENT PASSING NO. 200 SIEVE		10	+ MOISTURE + CONTENT OG			 Pocke Undist Pocke Disturi Torvar 	R STREI (ksf) t Penetron urbed Sam t Penetron bed Sampl e fined Com	neter nple neter le
1			Topsoil LOOSE to MEDIUM (SP)	DENSE Brown Fi	ne SAND		*12/6" *35/6" *10/6" *45/3" *25/6"											
2			LOOSE Dark Brown			¥	*29/6" *32/6" *30/6" *19/6"					•		•				
3	5	I (LOOSE to MEDIUM SP)	DENSE Brown Fi	ne SAND		*20/6" *25/6" *33/6" 2					•		•				
4							3 5 5 6	8				•		•				
5	10						6 9 12	15				• • • • • • • • • • • • • • • • • • • •		• • • • • • • • • • • • • • • • • • • •				
		ľ	MEDIUM DENSE GI	ay Fine SAND (SI	?)		5							- - - - - - - - - - - - - - - - - - -				
6	15						7 8	15				•		• • • • • • • • • • • • • • • • • • •				
	20	ľ I	MEDIUM DENSE Gi Fragments (SP)	ray Fine SAND Wi	th Shell		7 11 15	26				· · · · · · · · · · · · · · · · · · ·						
8	25		Boring T	erminated @ 25 ft.			5 7 10	17						<u> </u>				
Remar	ks * = Sta	atic	Hand Cone Penetrom	neter Value.														



 Project No.:
 35-24198

 Boring No.:
 B4

 Sheet
 1
 of
 1

Project: Brunswick-Glynn County JWSC Forcemain Client: Four Waters Engineering, Inc. Drill Rig: ATV Driller: C. Morgan														
Boring	Boring Location: See Field Exploration Plan Drill Rod: AWJ Drill Mud: Super Gel-X													
						Cas	ing Siz	ze:			Length	of Ca	sing:	
Ground	dwater De	pth: <u>4 ft</u> Time:	Drilling	_ Date: _	9/7/16	Bor	ing Be	gun:	<u>9/7/1</u>	6			pleted: <u>9/</u>	
SAMPLE NO.	0 DEPTH, FEET		N			BLOWS PER 6 IN.	N Value	PERCENT ORGANIC MATERIAL	PERCENT PASSING NO. 200 SIEVE		+ MOISTURE + CONTENT 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		 Pocket Pe Undisturb Pocket Pe Disturbed ▼ Torvane 	TRENGTH (sf) enetrometer ed Sample enetrometer Sample ed Compression ompression
1		Topsoil MEDIUM DENSE to SAND (SP) LOOSE to MEDIUM				*36/6" *43/2" *25/6" *14/6" *14/6" *19/6"								
2		SAND (SP) MEDIUM DENSE M	-		T	*29/6" *42/6" *37/6" *45/6"								
3		(SP)				*50/6" *40/6" 4 7								
4						7 5 5	14							
5						5 7 5	12							
6		MEDIUM DENSE G	ray Brown Fine S	AND (SP)		5 7								
12/16		MEDIUM DENSE to	VERVIOOSEI	Brown Grav		9	16							
20 01 BOKING 33-24198.65U ELLIS ASSOCIALES.60U 9/12/16	20	Fine SAND (SP)	VERT LOUSE I	nown Ordy		6 9 11	20							
0KING 35-24198.GPJ EI &	25					2 1 2	3							
		Boring T atic Hand Cone Penetron	erminated @ 25 f neter Value.	ì.										



Project	No.:	35-	2419	8	
Boring	No.:	B5			
Sheet	1		of	1	

Project: Brunswick-Glynn County JWSC Forcemain Client: Four Waters Engineering, Inc. Drill Rig: ATV Driller: C. Morgan																	
Boring	Location	n:	See Field Exploration	n Plan			Dril	1 Rod:	AW	/J		Dr	ill Mu	ıd: <u>s</u>	Super G	el-X	
Ground	lwater D	entl	h: _3.2 ft_ Time:	Drilling	Date:	9/9/16	Cas Bor	ing Siz	ze: _	9/9/1	6	Le Bo	ngth c ring (of Ca	sing: leted:	9/9/16	
SAMPLE NO.	DEPTH, FEET	SAMPLE TYPE	DESCRIPTION			575/10	BLOWS PER 6 IN.	N Value	PERCENT ORGANIC MATERIAL		OPLASTIC LIMIT				SHEAR Pocket Pocket Disturb Torvan Uncont	R STRENGT (ksf) Penetrometer urbed Sample Penetrometer bed Sample	r ssion
1		∕⊢	Topsoil LOOSE to MEDIUM D SAND (SP)	ENSE Dark Br	own Fine		*10/6" *35/6" *57/6" *48/6" *56/6"										
2			MEDIUM DENSE to L (SP) LOOSE Brown Fine SA			Y	*26/6" *23/6" *22/6"										
3	5		LOOSE Brown Fine SA			1	1 1 4 3 2	5					•				
4			SAND (SP)	ENSE Reu dio	wit rine		2 2 7 8 5	9									
5	10						5 7 9 11	16									
			MEDIUM DENSE Gray	7 Brown Fine S	AND (SP)		5						• • • • • • • • • • • • • • • • • • •				
6 	15						11 12	23				· · · · · · · · · · · · · · · · · · ·					
Ales.GDI 9/12/1			MEDIUM DENSE Gray	V Silty Fine SA	ND (SM)		5										
LOG OF BORING 35-24198.GPU ELLIS ASSOCIATES.GDI 9/2/16 8 8 8 8 8	20		VERY LOOSE Gray Fi	ne SAND (SP)			6	13									
2KING 35-24198.0 &	25						1 1 1	2									
ที่ Bemar		tatic	Boring Terr Hand Cone Penetromet	ninated @ 25 f er Value.	t.												



 Project No.:
 35-24198

 Boring No.:
 B6

 Sheet
 1
 of
 1

Project:	Project: Brunswick-Glynn County JWSC Forcemain Client: Four Waters Engineering, Inc. Drill Rig: ATV Driller: C. Morgan															
Boring	Boring Location: See Field Exploration Plan Drill Rod: AWJ Drill Mud: Super Gel-X															
			-				Casi	ing Siz	ze:				Length	of Ca	asing:	
Ground	water D	eptł	n: <u>3.2 ft</u> Time: _	Drilling	_ Date: _	9/6/16	Bor	ng Be	gun:	<u>9/6/1</u>	6		Boring	Com	pleted: <u>9/</u>	
SAMPLE NO.	DEPTH, FEET	SAMPLE TYPE	DESCRIPTIO	N			BLOWS PER 6 IN.	N Value	PERCENT ORGANIC MATERIAL	PERCENT PASSING NO. 200 SIEVE	- OPLASTIC LIMIT		+ CONTENT + (%) 20 30	-8 ¢Liquid Limit	(k ⊙ Pocket Pe Undisturb Pocket Pe Disturbed ▼ Torvane	TRENGTH ssf) enetrometer ed Sample enetrometer sample d Compression 1 2
1			Topsoil MEDIUM DENSE to I SAND (SP)				*7/6" *40/6" *39/6" *18/6"									
2			LOOSE to MEDIUM SAND (SP)	DENSE Brown	Gray Fine	¥	*10/6" *11/6" *20/6" *23/6" *22/6"									
3	5						*22/6" *27/6" *30/6" *46/6" 2									
4							2 3 4 4	5								
5	10						7 6 7	13								
			VERY LOOSE Gray V	/ery Clayey Fin	e SAND (So	C) ///	WOH/6	,								
6 2	15						2 1	3				• • • • • • • • • • • • • • • • • • •				
			LOOSE Gray Silty Fin Fragments (SM)	e SAND With S	Shell		3 3 4	7								
	20		DENSE Gray Fine SA	ND (SP)			4	1								
5-06 DNIV	25						8 15 17	32								
Remar		tatic	Boring Te Hand Cone Penetrom	rminated @ 25 eter Value.	ft.											

FIELD EXPLORATION PROCEDURES

Standard Penetration Test (SPT) Borings

The Standard Penetration Test (SPT) borings were made in general accordance with the latest revision of ASTM D 1586, "Penetration Test and Split-Barrel Sampling of Soils". The borings were advanced by rotary (or "wash-n-chop") drilling techniques. At 2 ½ to 5 foot intervals, a split-barrel sampler inserted to the borehole bottom and driven 18 inches into the soil using a 140 pound hammer falling on the average 30 inches per hammer blow. The number of hammer blows for the final 12 inches of penetration is termed the "penetration resistance, blow count, or N-value". This value is an index to several in-place geotechnical properties of the material tested, such as relative density and Young's Modulus.

After driving the sampler 18 inches (or less if in hard rock-like material), the sampler was retrieved from the borehole and representative samples of the material within the split-barrel were containerized and sealed. After completing the drilling operations, the samples for each boring were transported to our laboratory where they were examined by our engineer in order to verify the driller's field classification. The retrieved samples will be kept in our facility for a period of six (6) months unless directed otherwise.



KEY TO SOIL CLASSIFICATION

Description of Compactness or Consistency in Relation <u>To Standard Penetration Resistance</u>

Granular Materials					
Relative Density	Safety Hammer SPT N-Value (Blow/Foot)	Automatic Hammer SPT N-Value (Blow/Foot)			
Very Loose	Less than 4	Less than 3			
Loose	4 - 10	3 – 8			
Medium Dense	10 – 30	8 – 24			
Dense	30 – 50	24 – 40			
Very Dense	Greater than 50	Greater than 40			

Silts and Clays					
Consistency	Automatic Hammer SPT N- Value (Blow/Foot)				
Very Soft	Less than 2	Less than 1			
Soft	2 – 4	1 – 3			
Firm	4 – 8	3 – 6			
Stiff	8 – 15	6 – 12			
Very Stiff	15 – 30	12 – 24			
Hard	Greater than 30	Greater than 24			

DESCRIPTION OF SOIL COMPOSITION**

(Onlined Soli Classification System)							
- MAJC	MAJOR DIVISION		LABORATOR FINER THAN 200 SIEVE %	Y CLASSIFICATION CRITERIA SUPPLEMENTARY REQUIREMENTS	SOIL DESCRIPTION		
	Gravelly soils	GW	<5*	D_{60}/D_{10} greater than 4 $_{\rm l}$ $D_{30}{}^2$ / (D_{60} x D_{10}) between 1 & 3	Well graded gravels, sandy gravels		
-	(over half of coarse fraction larger than	GP	<5*	Not meeting above gradation for GW	Gap graded or uniform gravels, sandy gravels		
Coarse grained	No. 4)	GM	>12*	PI less than 4 or below A-line	Silty gravels, silty sandy gravels		
(over 50% by weight		GC	>12*	PI over 7 above A-line	Clayey gravels, clayey sandy gravels		
coarser than No.		SW	<5*	D_{60}/D_{10} greater than 6, D_{30}^2 / ($D_{60} \times D_{10}$) between 1 & 3	Well graded sands, gravelly sands		
200 sieve)	Sandy soils (over half of coarse fraction finer than No. 4)	SP	<5*	Not meeting above gradation requirements	Gap graded or uniform sands, gravelly sands		
		SM	>12*	PI less than 4 or below A-line	Silty sands, silty gravelly sands		
_		SC	>12*	PI over 7 and above A-line	Clayey sands, clayey gravelly sands		
	Low compressibility	ML	Plasticity chart		Silts, very fine sands, silty or clayey fine sands, micaceous silts		
Fine grained	(liquid limit less	CL	Plasticity chart		Low plasticity clays, sandy or silty clays		
(over 50%) • by weight	than 50)	OL	Plasticity chart, organic odor or color		Organic silts and clays of low plasticity		
finer than No. 200	High compressibility	МН	Plasticity chart		Micaceous silts, diatomaceous silts, volcanic ash		
sieve)	(liquid limit more	СН	Plasticity chart		Highly plastic clays and sandy clays		
	than 50)	ОН	Plasticity chart,	organic odor or color	Organic silts and clays of high plasticity		
Soils with fib	rous organic matter	PT	Fibrous organic	matter; will char, burn or glow	Peat, sandy peats, and clayey peat		
			•				

(Unified Soil Classification System)

* For soils having 5 to 12 percent passing the No. 200 sieve, use a dual symbol such as SP-SM. ** Standard Classification of Soils for Engineering Purposes (ASTM D 2487)

SAND/GRAVEL DESCRIPTION MODIFIERS					
Modifier	Sand/Gravel Content				
Trace	<15%				
With	15% to 29%				
Sandy/Gravelly	>29%				

ORGANIC MATERIAL MODIFIERS				
Modifier	Organic Content			
Trace	1% to 2%			
Few	2% to 4%			
Some	4% to 8%			
Many	>8%			

SILT/CLAY DESCRIPTION MODIFIERS					
Modifier	Silt/Clay Content				
Trace	<5%				
With	5% to12%				
Silty/Clayey	13% to 35%				
Very	>35%				

PUMP STATION 2032 REGIONAL FORCEMAIN IMPROVEMENTS ST. SIMONS ISLAND, GEORGIA

ATTACHMENT

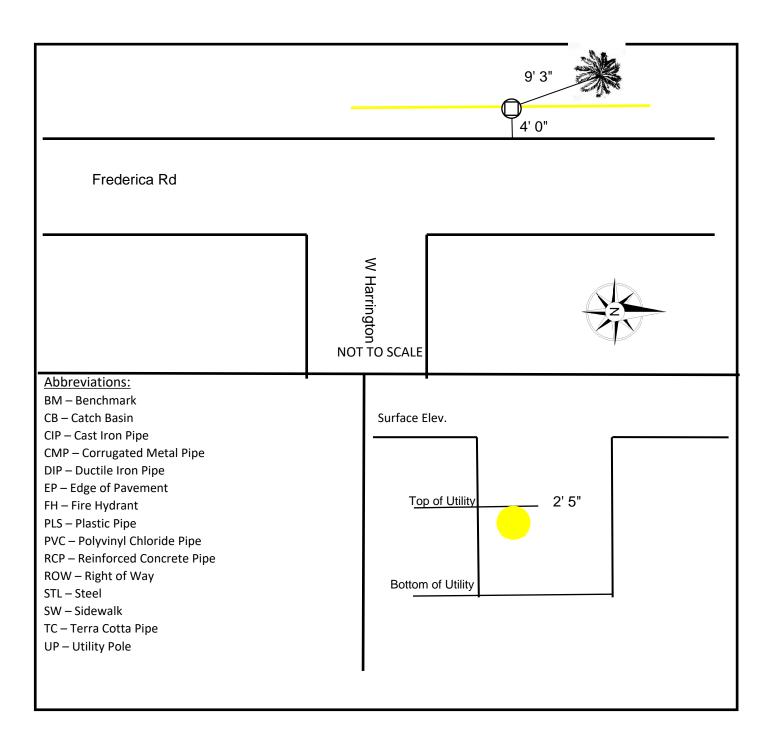
Test Hole Data: JWSC LS2032 Regional Forcemain Improvements Project Test Holes 1 – 49

RHD Services9/10/2016JWSC LS2032 Regional Forcemain Improvements Project - Test Hole Worksheet

TH # (as	Utility	Pavement/Soil	Top of Utility	Utility Size	Material	Notes
marked	•		to Ground			
in field)			Depth			
1	Gas	Soil	2' 5"	2"	Plastic	
2	Telecom	Soil	3' 6"	1.5"	Plastic	
3-A	Water	Soil	4' 0"	12"	PVC	
3-B	Telecom	Soil	6' 2"	1.5"	Plastic	
4	Water	Soil	3' 10"	12'	A/C	A/C: Asbestos Pipe
5	Telecom	Soil	10' 0"			Used probing rod to verify utility, utility below water table
6	Sanitary FM	Soil	8' 0"		PVC	High water table
7	Water	Soil	2' 8"	12"	PVC	
8	Water	Soil	1' 9"	12"	PVC	
9	Sanitary FM	Soil	2' 9"	10"	PVC	
10	Telecom	Soil	10' 9"	1.5"		FO
11	Water	Soil	2' 9"	8"	PVC	
12	Gas	Soil	3' 0"	4"	Steel	
13	Water	Soil	3' 0"	12"	A/C	
14	Telecom	Soil	3' 4"	various 1.5"	Plastic	
15	Sanitary FM	Soil	5' 5"	10"	Plastic	
16	Gas	Soil	2' 7"	2"	Plastic	
16	Telecom	Soil	aprox 8'			High water table
17	Sanitary FM	Soil	4' 0"	8"	Plastic	
18	Sanitary FM	Soil	2' 6"	8"	Plastic	
19	Water	Soil	4' 5"	8"	A/C	
20	Sanitary FM	Soil	4' 5"	8"	Plastic	
21	Telecom	Soil	4' 5"	2"	Cable	
22	Power	Soil	8' 4"		Conduit	High water table
23	Sanitary FM	Soil	2' 8"	8"	PVC	
24	Telecom	Soil	2' 4"	FO cable	FO	
25	Sanitary FM	Soil	3' 3'	8"	PVC	
26	Telecom	Soil	3' 0"	Various 1.5"		
27	Sanitary FM	Soil	3' 2"	8"	PVC	
28						Unused Number
29	Sanitary FM	Soil	2' 10"	8"	PVC	
30	Gas	Soil	3' 2"	4"	Steel	
31	Water	Pavement	3' 3"		Steel	In pavement. Depth by GPR
32	Sanitary FM	Soil	2' 4"	8"	PVC	
33	Telecom	Soil	3'0"	FO cable 4"	FO	
34 35	Gas	Soil	2' 10" 2' 7"	4" 1.5"	Steel	
35	Telecom Sanitary FM	Soil Soil	2 7	1.5 8"	Cable PVC	In payament - Depth by CDP
36	Gas	Soil	2'9	8 4"	Plastic	In pavement. Depth by GPR
37	Power	Soil	6' 11"	4 Aprox. 3"	Cable	
39	Water	Pavement	3' 5"	Αμιύλ. 5	Plastic	In pavement. Depth by GPR
40	Water	Soil	3 5	6"	A/C	
40	Telecom	Soil	2'9"	Aprox. 2"	FO	
41 42	Gas	Soil	3' 0"	2"	Steel	
42	Water	Soil	3'4"	12"	A/C	
43	Gas	Pavement	2'0"		A/C	In pavement. Depth by GPR
44	Water	Pavement	2'9"			In pavement. Depth by GPR
45	Water	Soil	3' 3"	2"	A/C	in pavement, beptir by or it
40	Gas	Pavement	2' 5"		Plastic	In pavement. Depth by GPR
47	Telecom	Soil	4' 3"	Aprox. 1"	FO	in parement, beptil by on the
48	Telecom	Soil	2' 4"	Aprox. 1 Aprox. 1"	FO	
						In pavement, Depth by GPR
49	Gas	Pavement	4' 0"		Plastic	In pavement. Depth by GPR



Client:	Four Wate	ers Engi	neering	Project:	Sea Island Road			Test Hole#	1
General	Location:	Fredric	ca Rd & W	. Harrington			Date:	8-2-16	
City: S	t. Simons	Island	County:_	Glen County		_State:	GA	_Crew Chief:_	Baron
Notes: _									
Utility:	Gas	Size:	2" [Material: PLS	Condition:Go	od	Ribbon	Color:	
Maker S	et:		Asphalt 1	Thickness: N/A	Soil Type: Gray	/ Sand	Field Co	ondition: Go	od

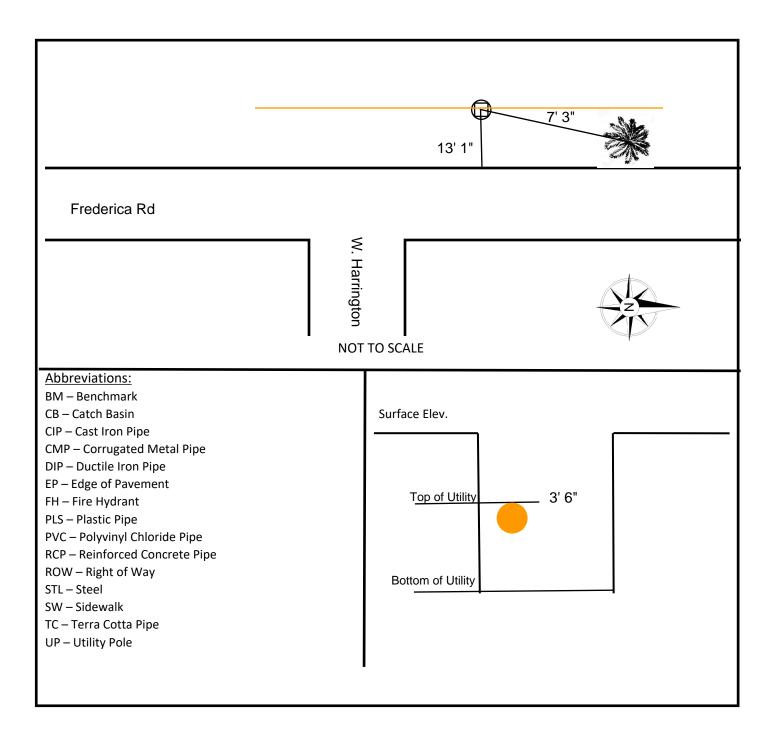


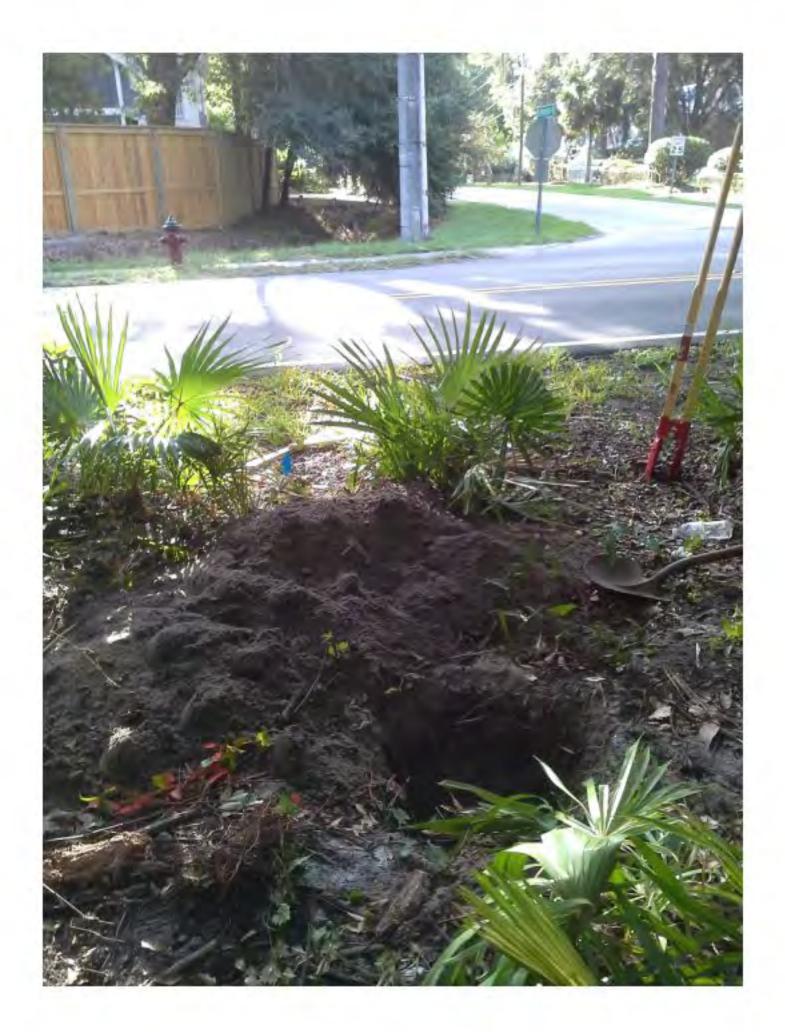






		, Inc. Project:	Sea Island Road			Test Hole# 2	
General Location:	Fredrica Rd. &	W. Harrington			_ Date:	8-2-16	
City: St. Simons	sland Count	ty: Glen County		State:	GA	_ Crew Chief: Barc	n
Notes:							
Utility: Telecom	Size:1.5"		Condition:	Good	_ Ribboı	n Color:	
Maker Set:	Aspha	alt Thickness:	Soil Type:_G	ray Sand	_ Field C	Condition: Good	

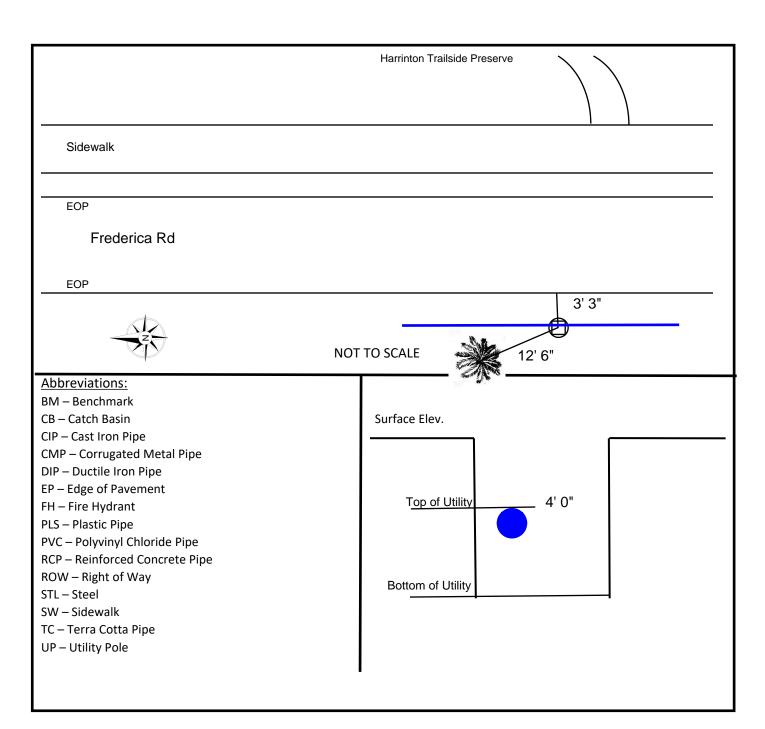


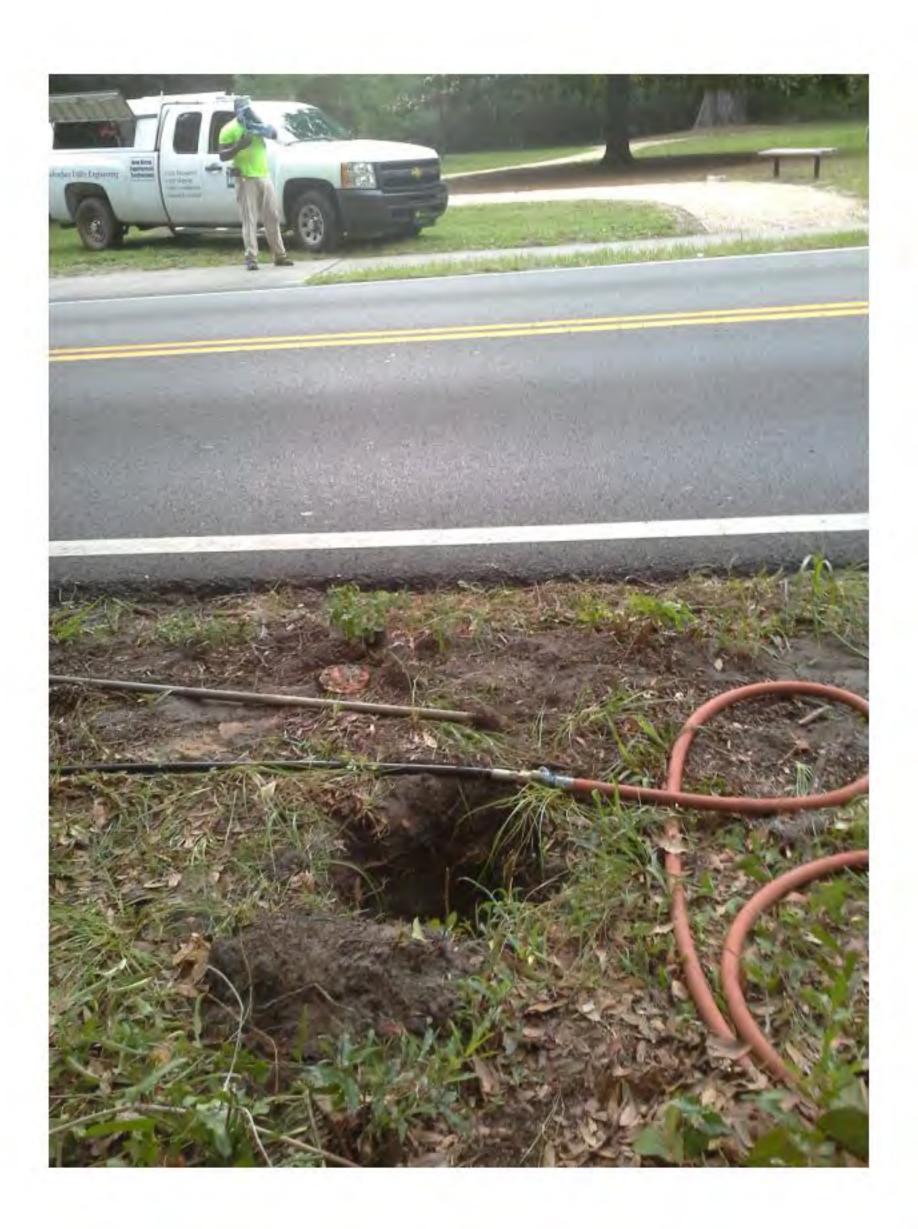






Client: Four	Vaters Engi	neering, In	C. Project:	Sea Island Road		Test Hole#3-A
General Locat	on: Fredric	a Rd			Date:	8-2-16
City: St. Sime	ns Island	County:	Glen County	State	: GA	Crew Chief: Baron
Notes:						
Utility: Water	Size:	12"N	Material: PVC	Condition: Good	Ribbo	n Color:
Maker Set:		Asphalt 1	Thickness:	Soil Type: Gray San	dField	Condition: Good

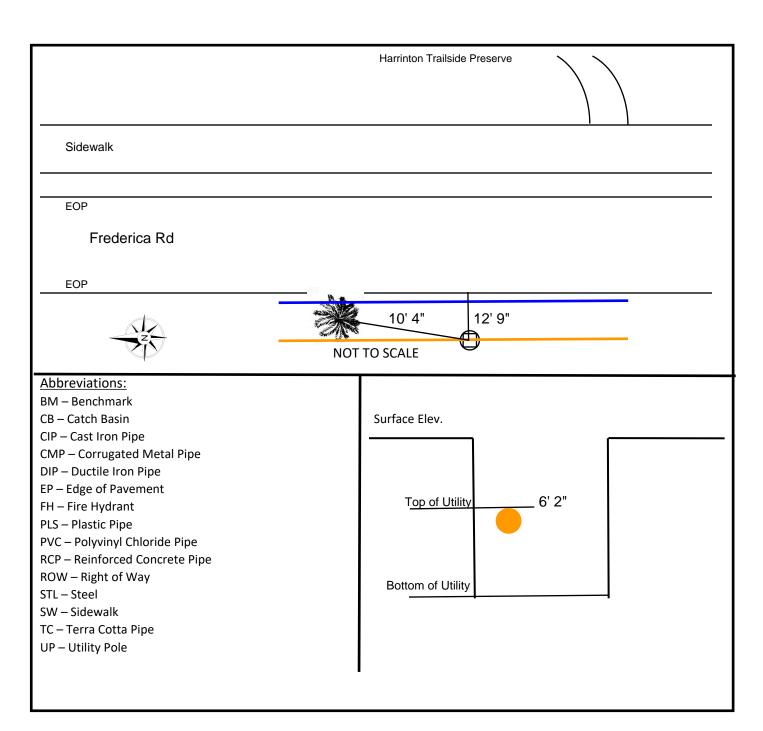








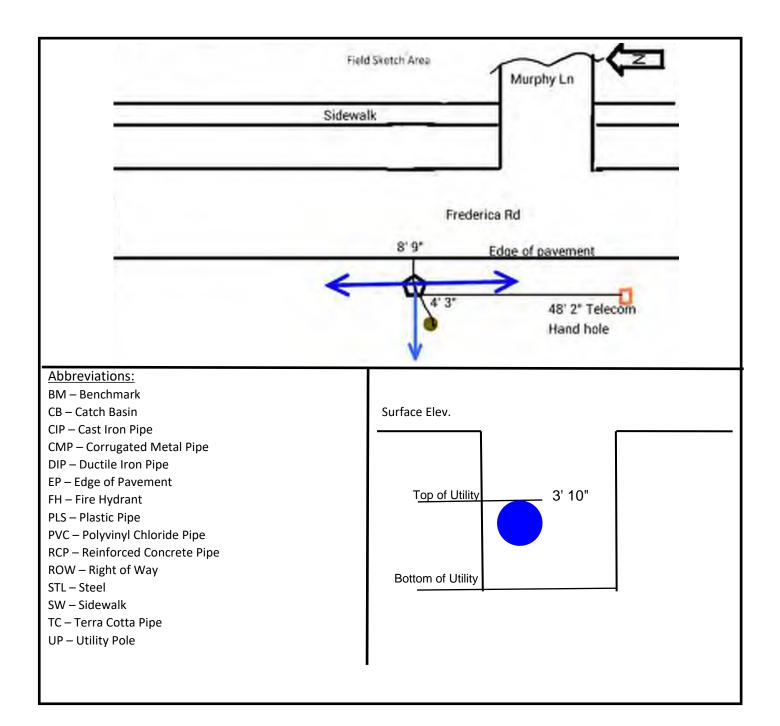
Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#_3-B
General Location: Fredrica Rd	Date: 8-2-16
	GA Crew Chief: Baron
Notes:	
Utility: Telecom Size: 1.5" Material: PVC Condition: Good	_ Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type: Gray Sand	_ Field Condition: Good







Client: Four Wate				Sea Island Road			_ Test Hole#_ 4	
General Location:	Fredr	ica Rd & I	Murphy Lane			_ Date:_	8-2-16	
City: St. Simons I	Island	County:	Glen County		State:	GA	_ Crew Chief:_E	Baron
Notes:								
Utility: Water	Size:	12"	Material: A/C	Condition:G	iood	_ Ribbor	n Color:	
Maker Set:		Asphalt	Thickness:	Soil Type: Gra	ay Sand	_ Field C	Condition: Goo	d

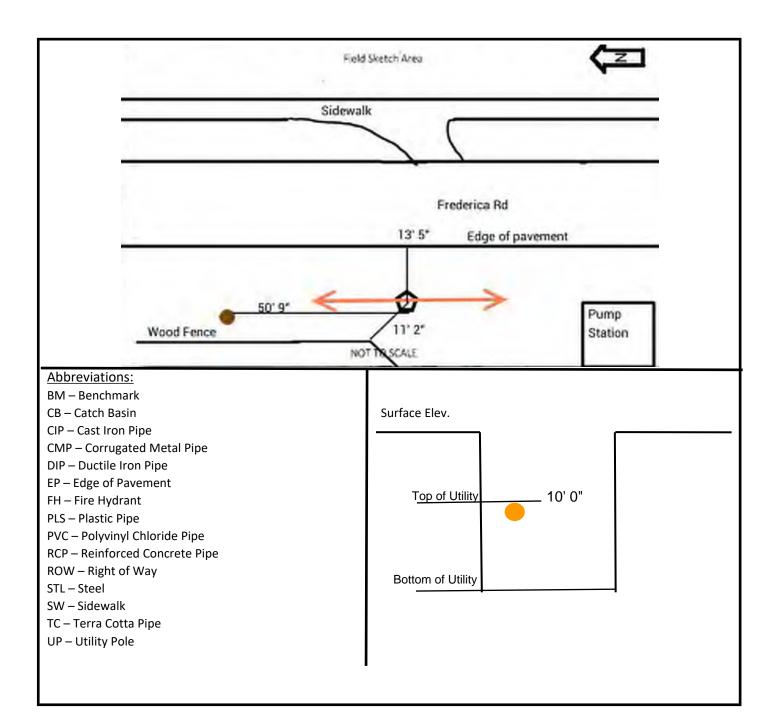


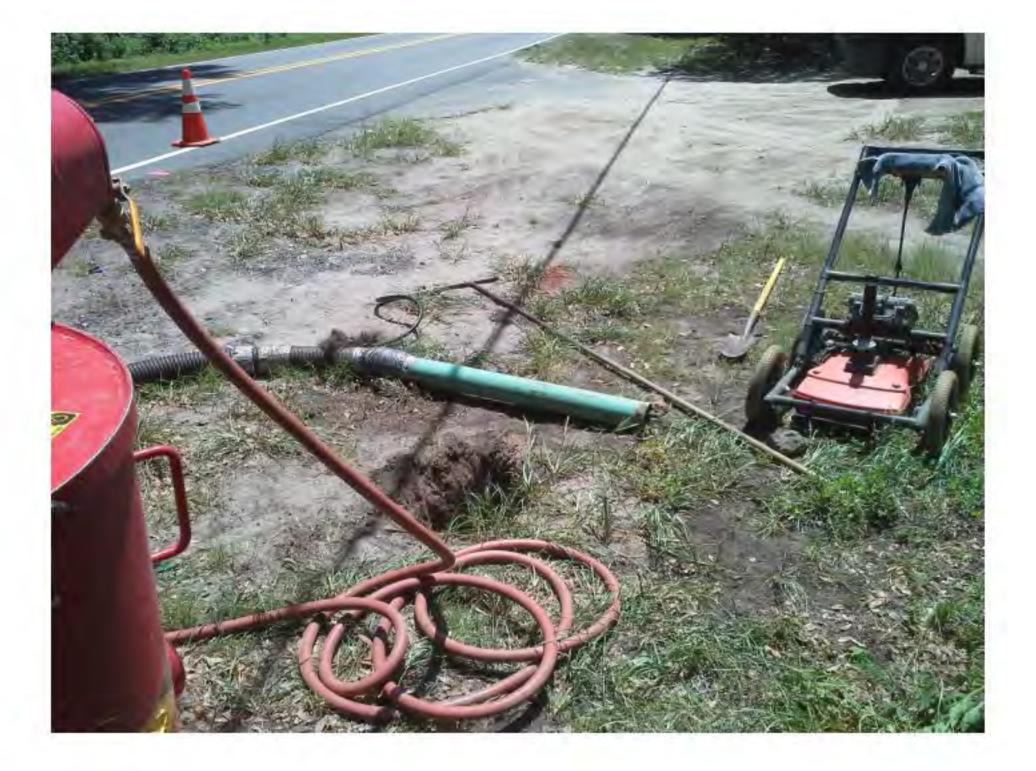






Client: Four Waters Engin	neering, Inc. Project: Sea	a Island Road	Test Hole#_5
General Location: Fredr	ica Rd		Date: 8-17-16
City: St. Simons Island	_ County: Glen County	State:	GA Crew Chief: Baron
Notes:			
Utility: Telecom Size:	Material: Cable	Condition: Good	Ribbon Color:
Maker Set:	Asphalt Thickness:	Soil Type: Gray Sand	Field Condition: Good

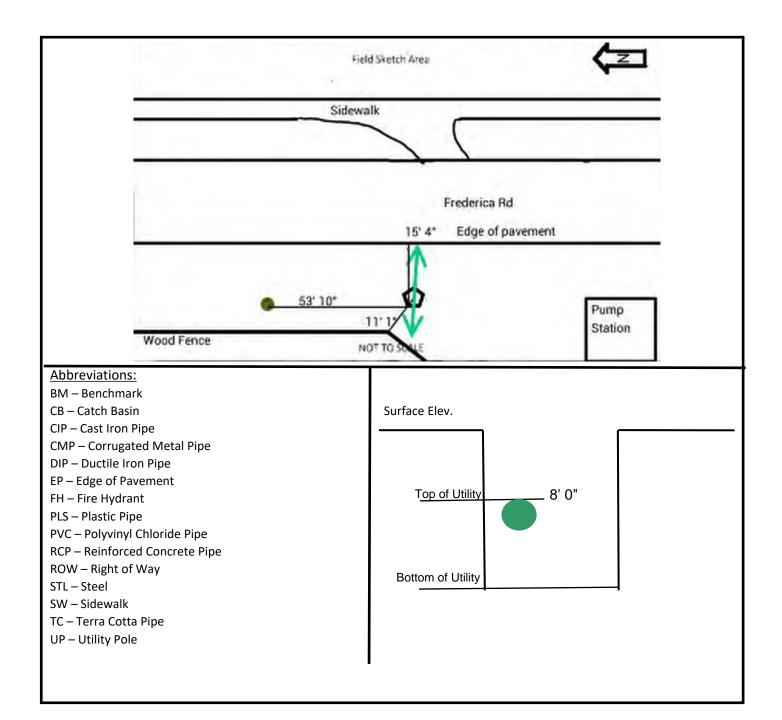


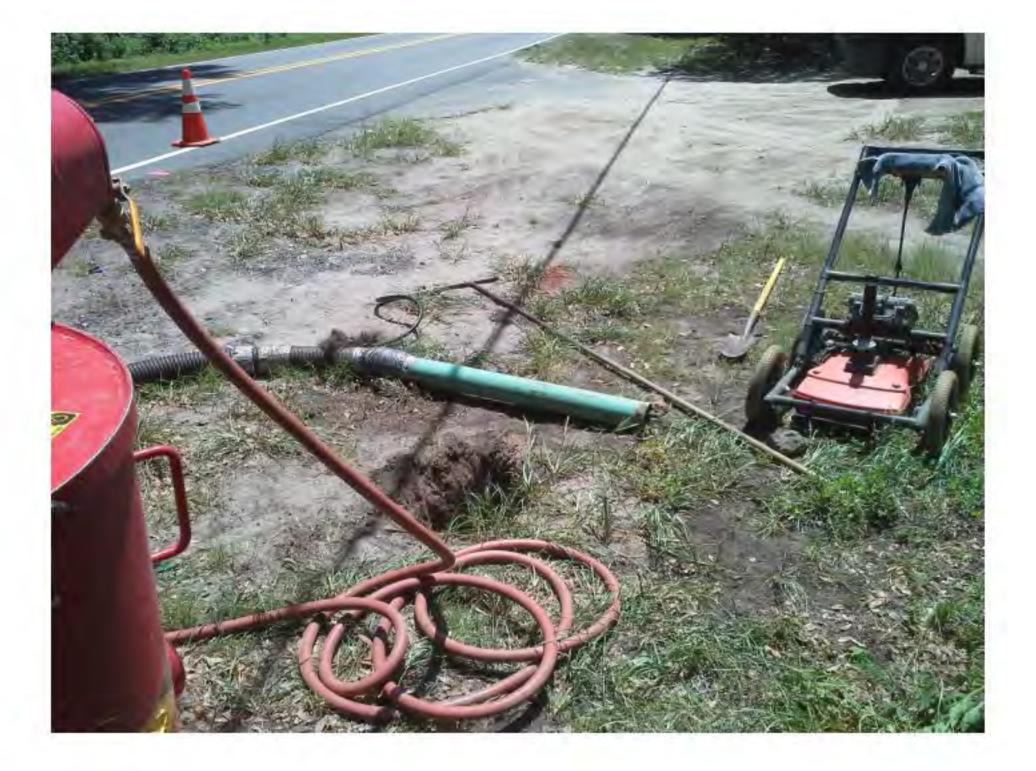






Client: Four Waters Engineering, Inc. Project: Sea Isla	nd Road Test Hole#_6
General Location: Fredrica Rd	Date:_ 8-17-16
City: St. Simons Island County: Glen County	State: GA Crew Chief: Baron
Notes: High water table, unable to get a good view of the p	ipe
Utility: Sewer Size: Unknown Material: PVC Co	ndition:GOOdRibbon Color:
Maker Set: Asphalt Thickness: So	I Type: Gray Sand Field Condition: Good



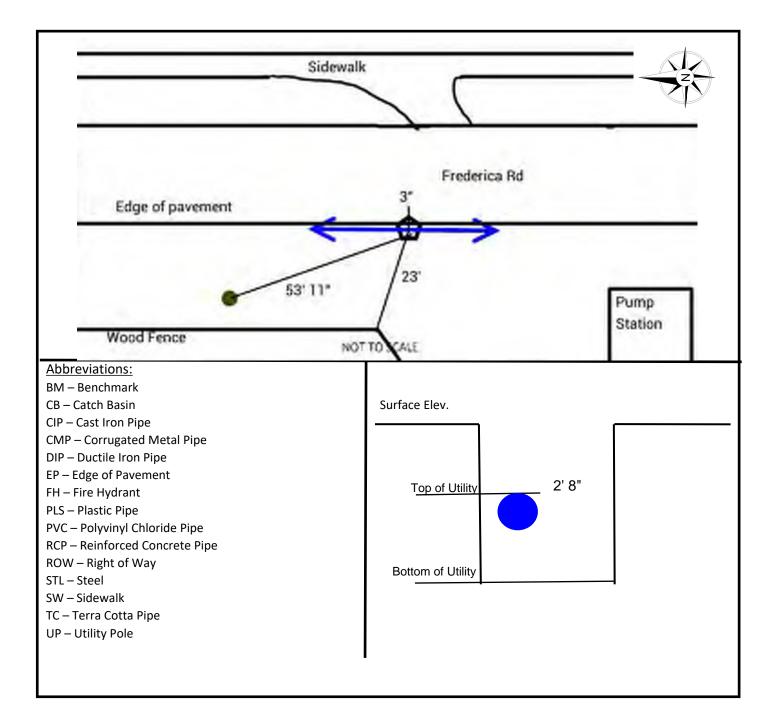






Client:	Four Wate	ers Engi	neering,	Inc.	Project:_	Sea Island Road				7
General	Location:_	Fredr	ica Rd					_ Date:_	8-18-16	
City: St.	. Simons I	sland	County	:_Glen	County		State:	GA	_ Crew Chief:_	Baron
Notes: _										
Utility:	Water	Size:	12"	_ Materia	al: PVC	Condition:	Good	_ Ribbor	n Color:	
Maker S	Set:		Aspha	lt Thickne	ss:	Soil Type: G	Fray Sand	_ Field C	Condition: Goo	bd

Field Sketch Area

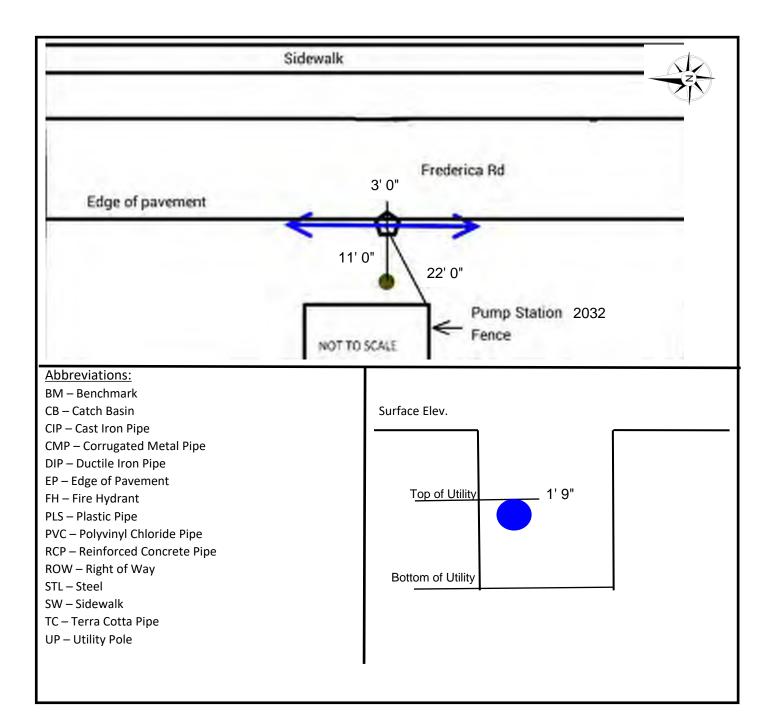








Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#8
General Location: Fredrica Rd	Date:_8-18-16
City: St. Simons Island County: Glen County State	e: GA Crew Chief: Baron
Notes:	
Utility: Water Size: 12" Material: PVC Condition: Good	Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type: Gray San	d Field Condition: Good

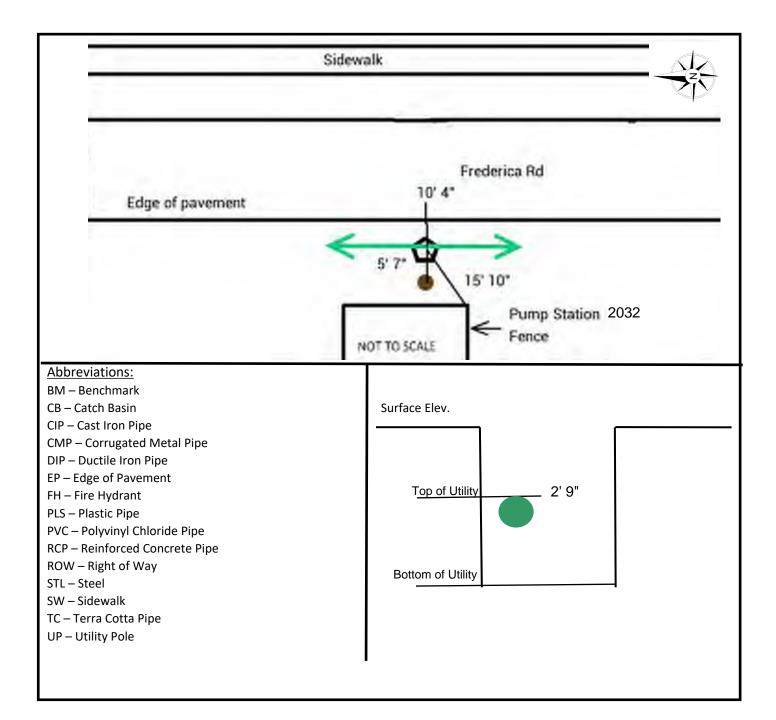








Client: Four Waters Engineerin	g, Inc. Project:_	Sea Island Road		_Test Hole#9
General Location: Fredrica Ro	bad		_ Date:_	
City: St. Simons Island Cou	nty: Glen County	State:	GA	Crew Chief: Baron
Notes:				
Utility: Sewer Size: 10"	Material:	Condition: Good	_ Ribbon	Color:
Maker Set: Asp	halt Thickness:	Soil Type: Gray Sand	_ Field C	ondition: Good

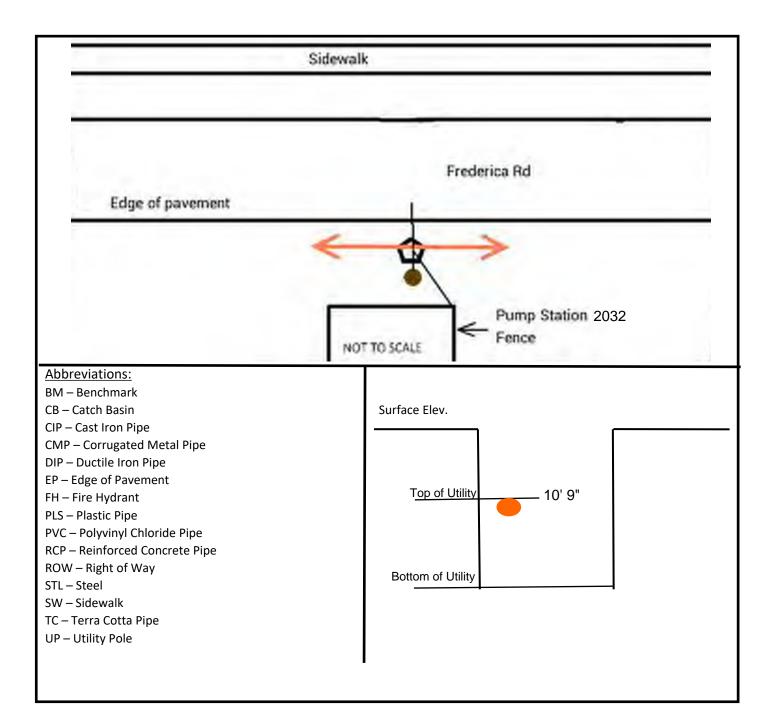








Client: Four Waters Engineeri	ng, Inc. Project: Sea	Island Road		_Test Hole#_10
General Location: Fredrica F	Road		_ Date:_	8-18-16
City: St. Simons Island Co	unty: Glen County	State:	GA	Crew Chief: Baron
Notes: High water table. unal	ble to get a good photo.			
Utility: Telecom Size: 1.5"	Material: Fiber	Condition: Good	Ribbon	Color:
Maker Set:As	phalt Thickness:	_Soil Type: Gray Sand	_ Field Co	ondition: Good

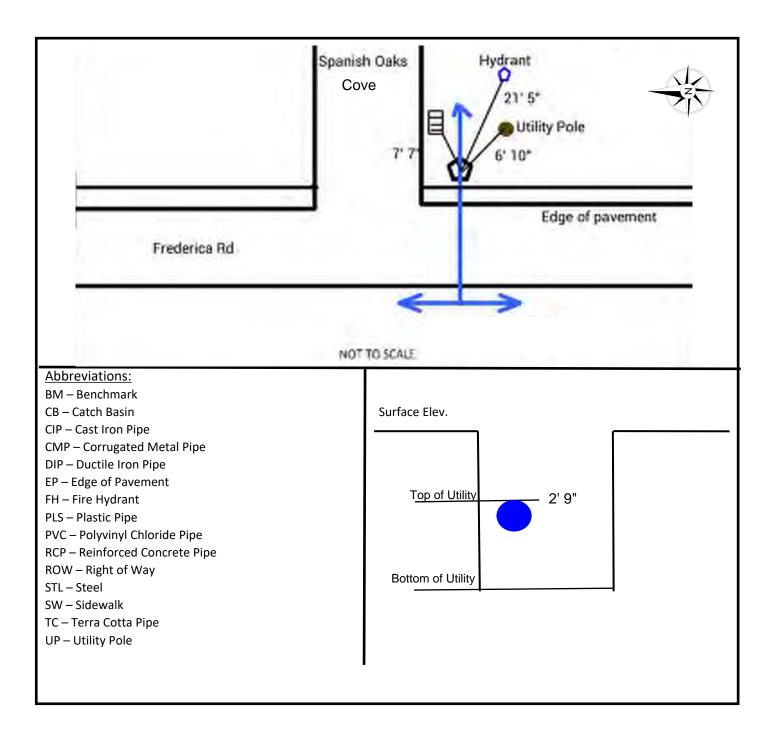


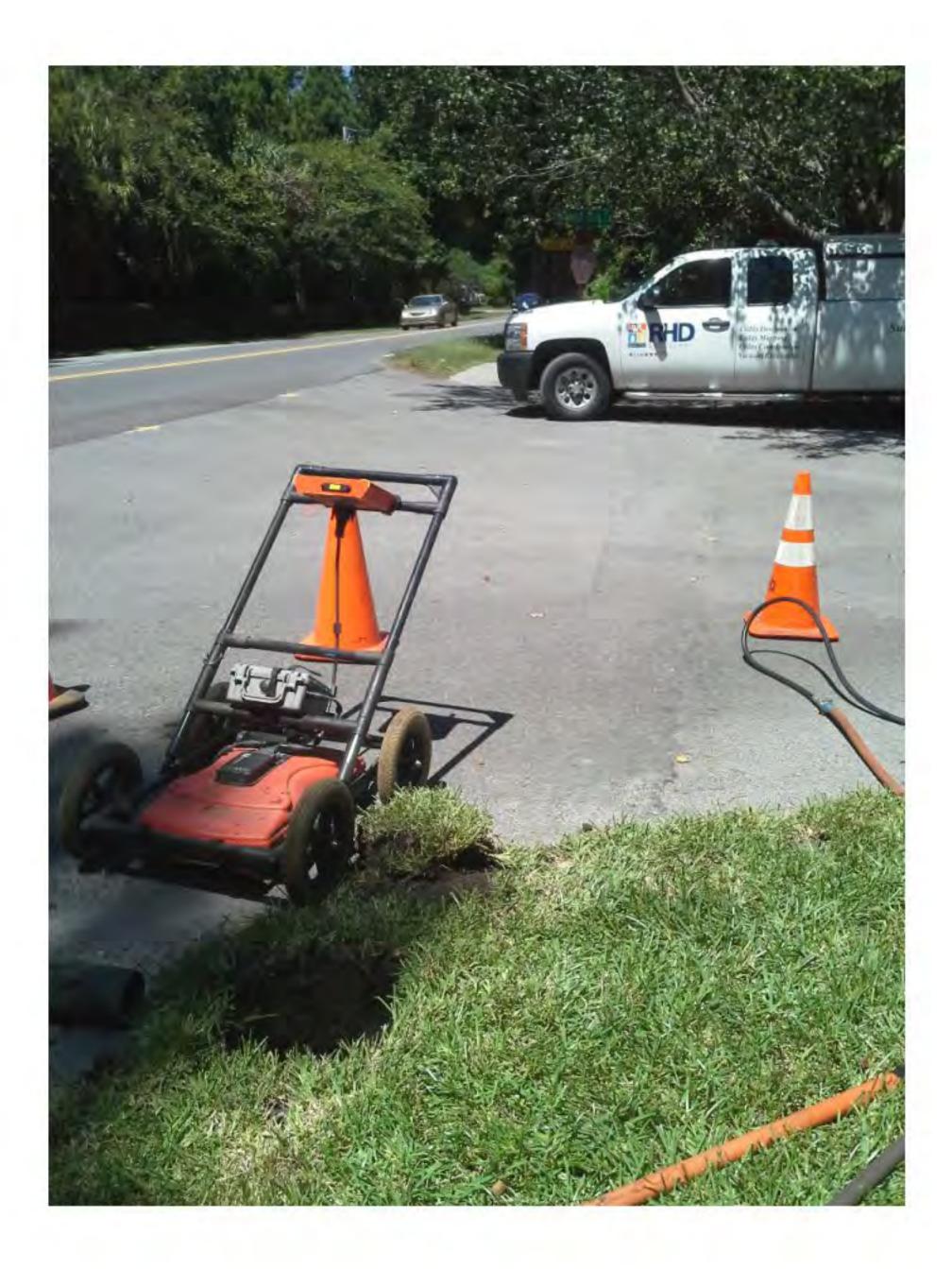


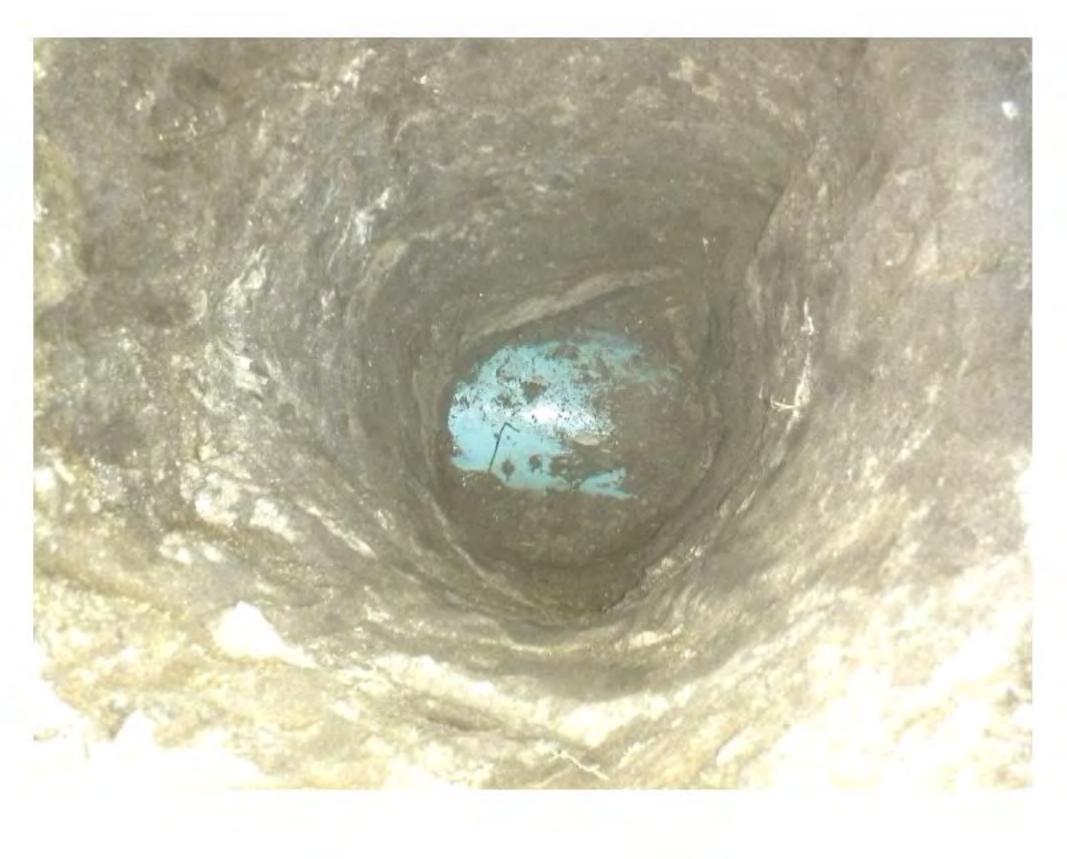




Client: Four Water	s Engin	eering, li	nc. Project:	Sea Island Road		Test Hole#11
General Location:	Fredri	ca Road	& Spanish Oaks	s Cove	Date:_	8-18-16
City: St. Simons Isl	and	_ County:	Glen County	State:	GA	_ Crew Chief: Baron
Notes:						
Utility: Water	_Size:	8"	Material: PVC	Condition: Good	_ Ribbo	n Color:
Maker Set:		_Asphalt	Thickness:	Soil Type: Gray Sand	_ Field C	Condition: Good

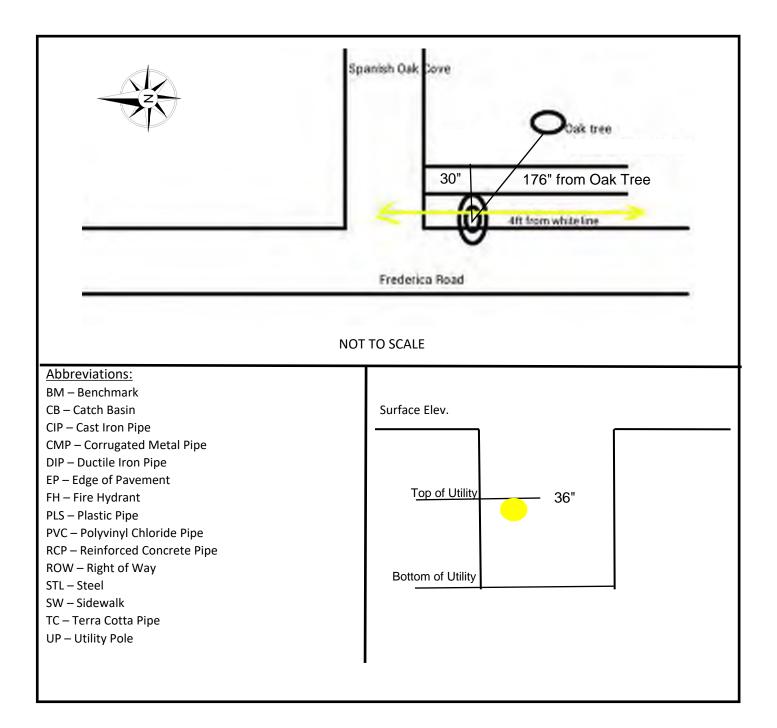








Client: Four Waters Engi				_Test Hole#_12
General Location: Fredr	ica Road & Spanish Oak cove		_ Date:_	8-22-16
City: St. Simons Island	_ County: Glen County	State:	GA	Crew Chief: Ira
Notes:				
Utility: Gas Size:	4" Material: Steel	Condition: Good	_ Ribbor	n Color:
Maker Set:	Asphalt Thickness:	Soil Type: Gray Sand	_ Field C	ondition: Good

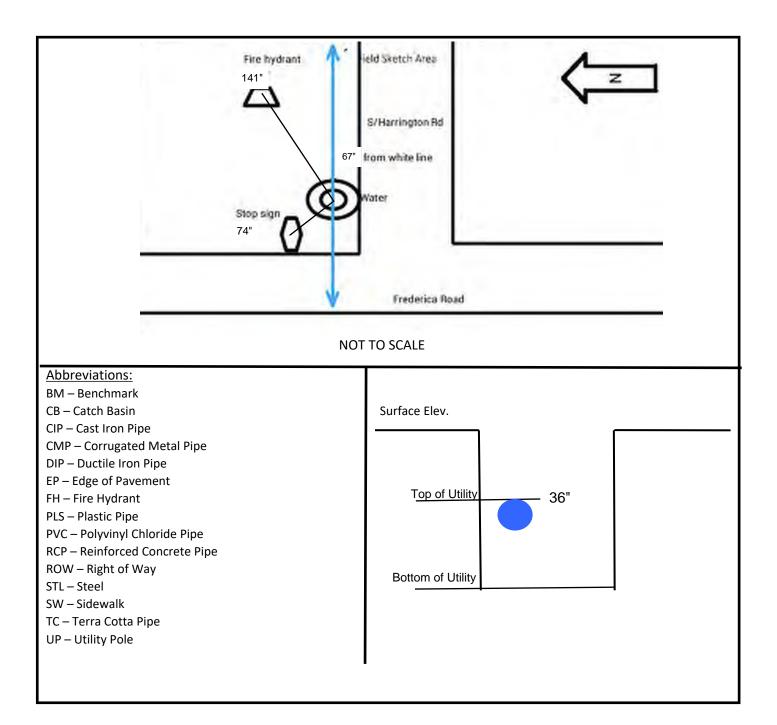








Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#13
General Location: Fredrica Road & S Harrington Road	Date: 8-22-16
City: St. Simons Island County: Glen County Sta	te: GA Crew Chief: Ira
Notes:	
Utility: Water Size: 12" Material: A/C Condition: Good	Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type: Gray Sa	nd Field Condition: Good

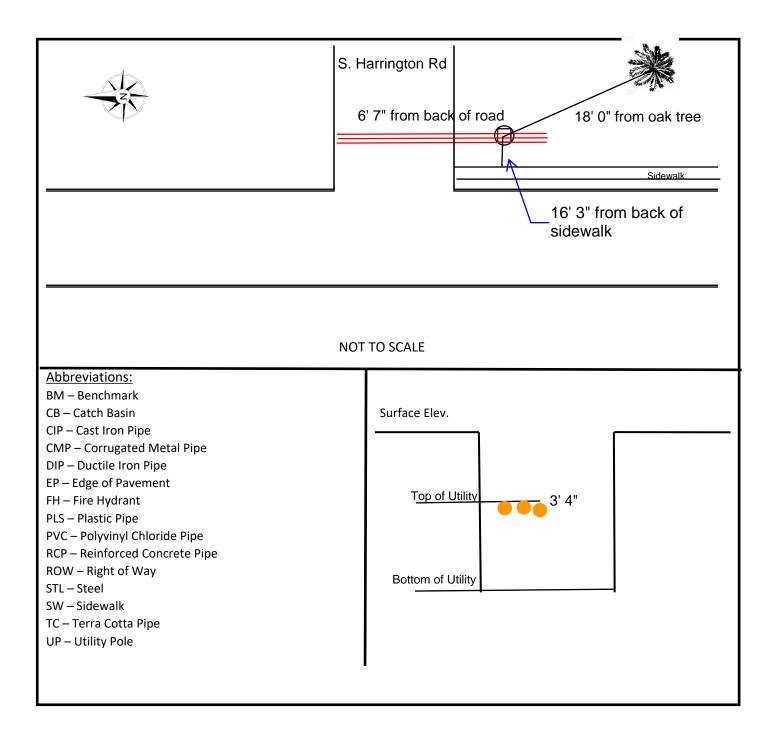








Client: Four Waters Engineering, Inc. Project: Sea		Test Hole#14
General Location: Fredrica Road & S. Harrinton Road	Dat	e: 8-22-16
City: St. Simons Island County: Glen County	State: GA	Crew Chief: Ira
Notes:		
Utility: Telecom Size: Conduits Material: PVC	_ Condition: <u>Good</u> Ribb	on Color:
Maker Set: Asphalt Thickness:	_Soil Type: Sand Field	Condition: Good





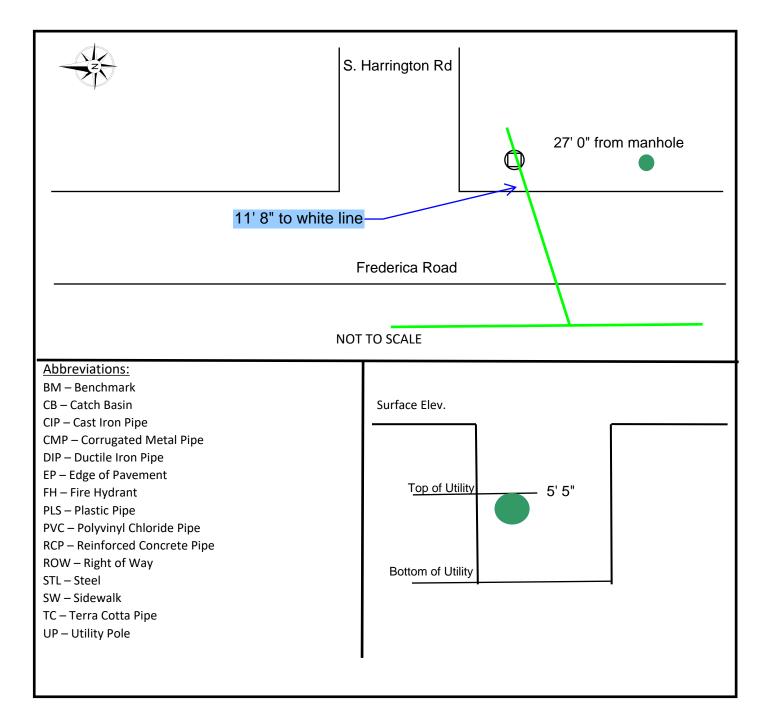








Client: Four Waters Engineering, Inc. Project:	Sea Island Road Test Hole# 15
General Location: Fredrica Road and S. Harrington	on Road Date: 8-22-16
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: Sanitary Size: 10" Material: A/C	Condition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:	Soil Type: <u>Sand</u> Field Condition: <u>Good</u>



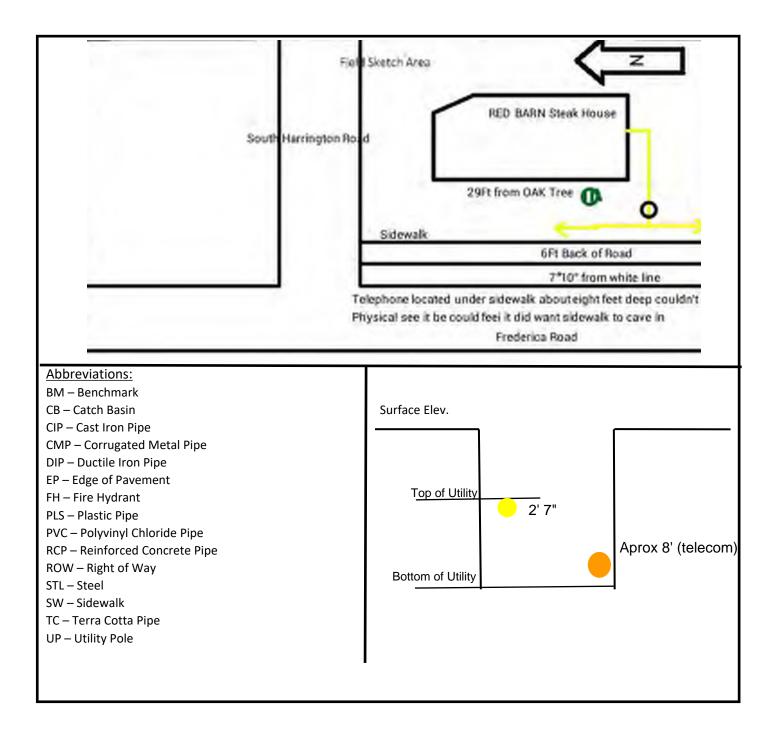








Client: Four Waters Engir	neering, Inc. Project: Sea	Island Road		_ Test Hole#16
General Location: Fredr	ica Road and S. Harrington Ro	bad	_ Date:_	8-22-16
City: St. Simons Island	_ County: Glen County	State:	GA	_Crew Chief: Ira
Notes:				
Utility: Gas/Telecomsize:	2" Material: PVC	_ Condition: <u>Good</u>	_ Ribbor	n Color:
Maker Set:	_Asphalt Thickness:	_Soil Type: Sand	_ Field C	Condition: Good



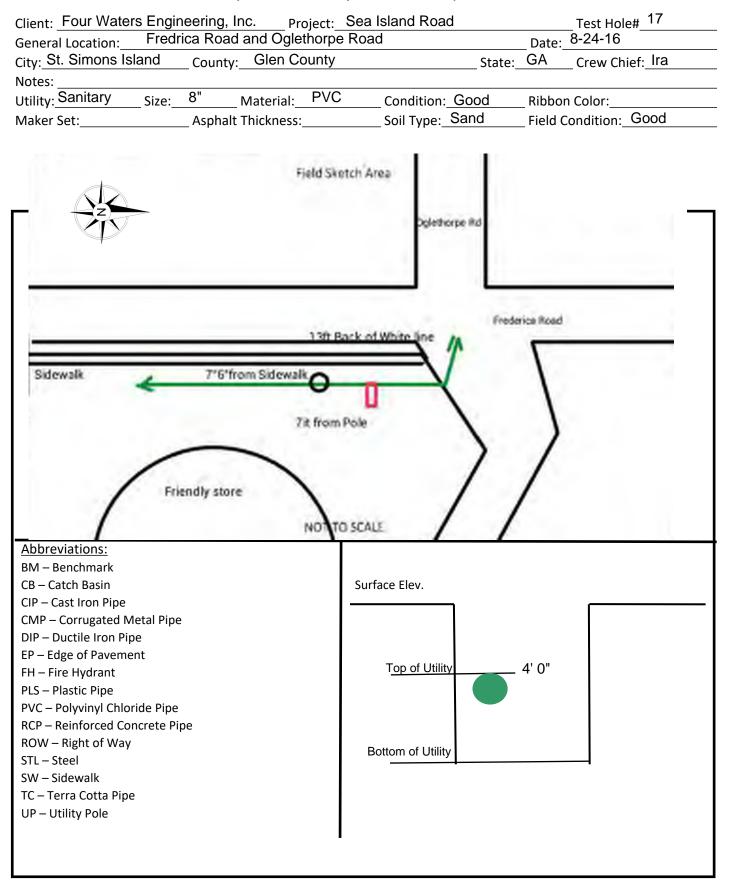






630 10th St. NW Atlanta, GA 30318 404-874-8585 RHDServices.com

Quality Level-A Utility Test Hole Report



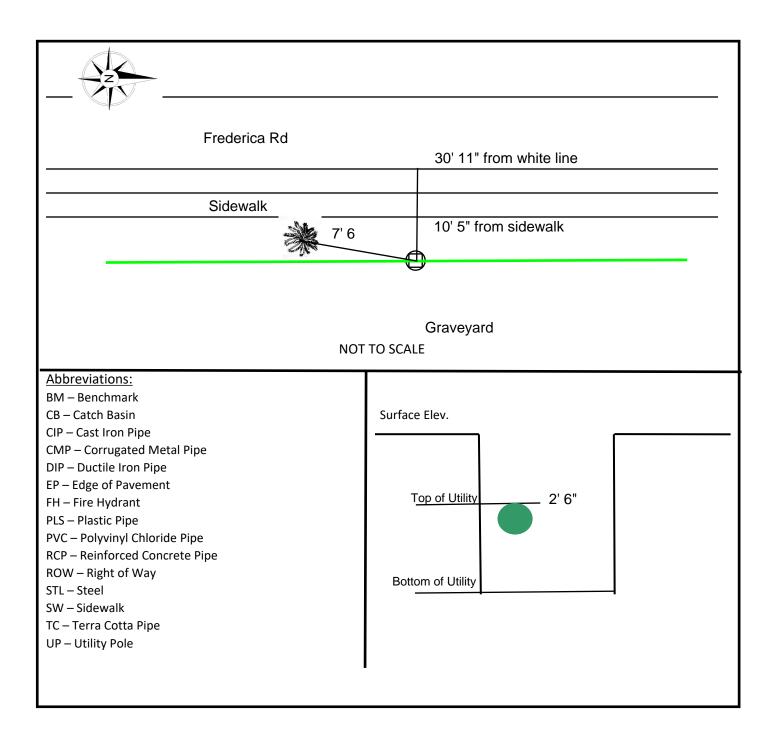








Client: Four Wate	rs Engir	neering, In	C. Pr	oject:	Sea Island Road			_ Test Hole#	8
General Location:	Fred	rica Road i	n front of	the ce	metery		_ Date:_	8-24-16	
City: St. Simons Is	sland	County:	Glen C	ounty		_State:_	GA	_ Crew Chief:_	ra
Notes:									
Utility: Sanitary	Size:	<u>8"</u>	Material:_	PVC	Condition: <u>Go</u>	od	_ Ribbor	n Color:	
Maker Set:		Asphalt 1	Thickness:		Soil Type: Sar	nd	_ Field C	condition: Goo	d



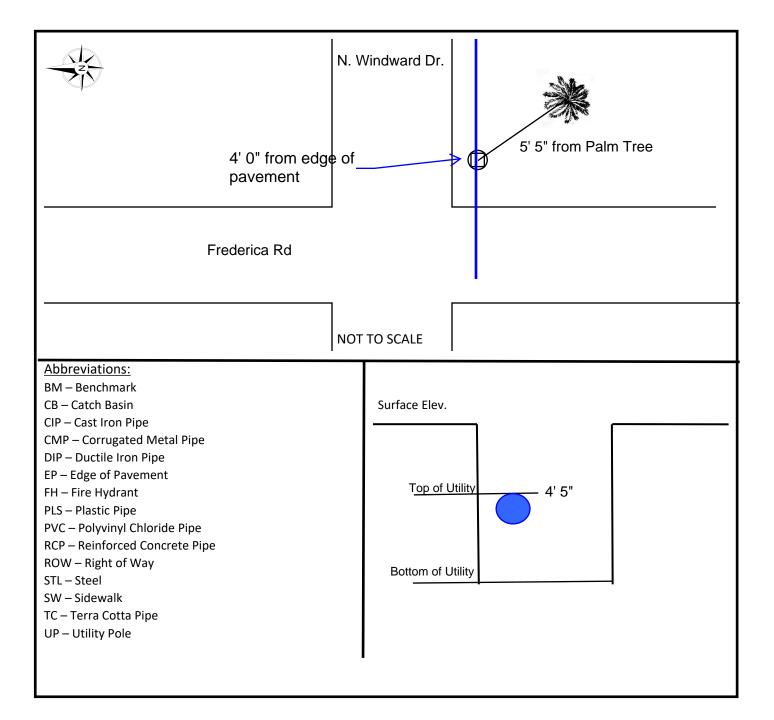








Client: Four Waters Engineer	ring, Inc. Project: Sea I	sland Road		_Test Hole#19
General Location: Fredrica	Road and N. Windward Dr		_ Date:_	8-24-16
City: St. Simons Island Co	ounty: Glen County	State:	GA	Crew Chief: Ira
Notes:				
Utility: Water Size: 8"	Material: A/C	Condition: Good	_ Ribbon	Color:
Maker Set:As	sphalt Thickness:	Soil Type: Sand	_ Field C	ondition: Good

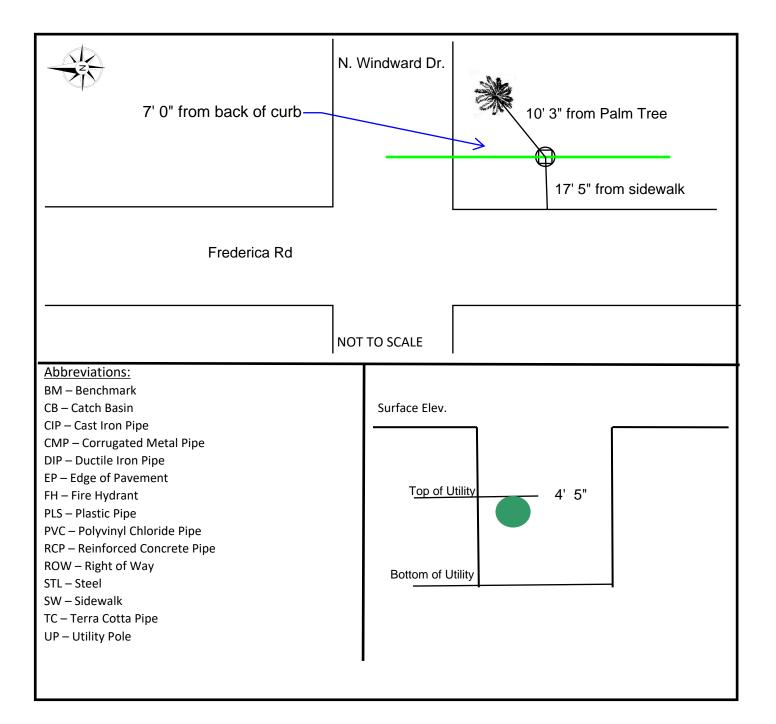


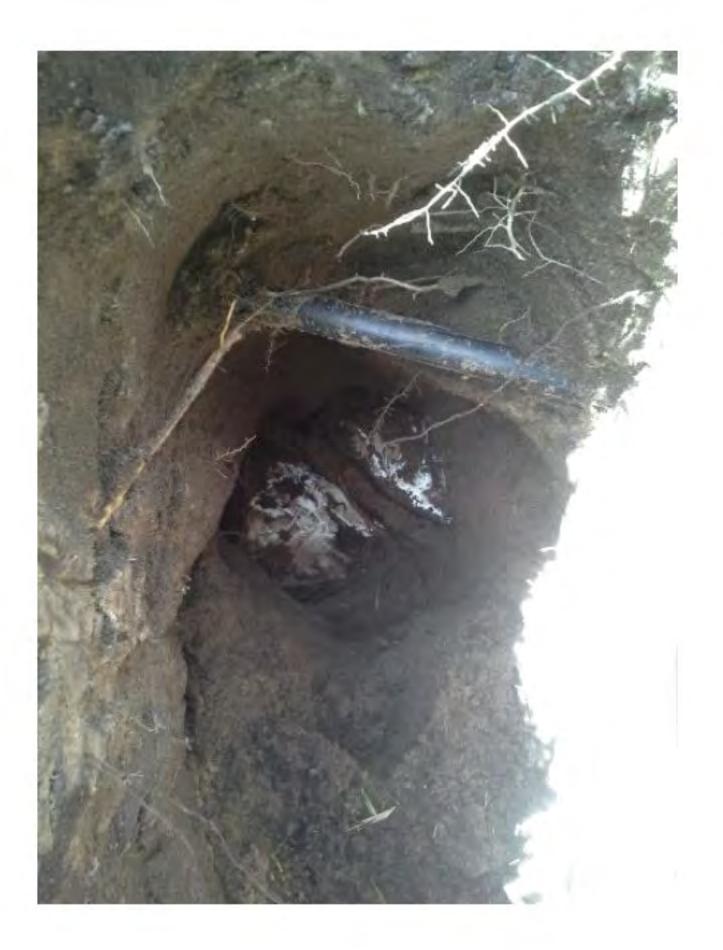


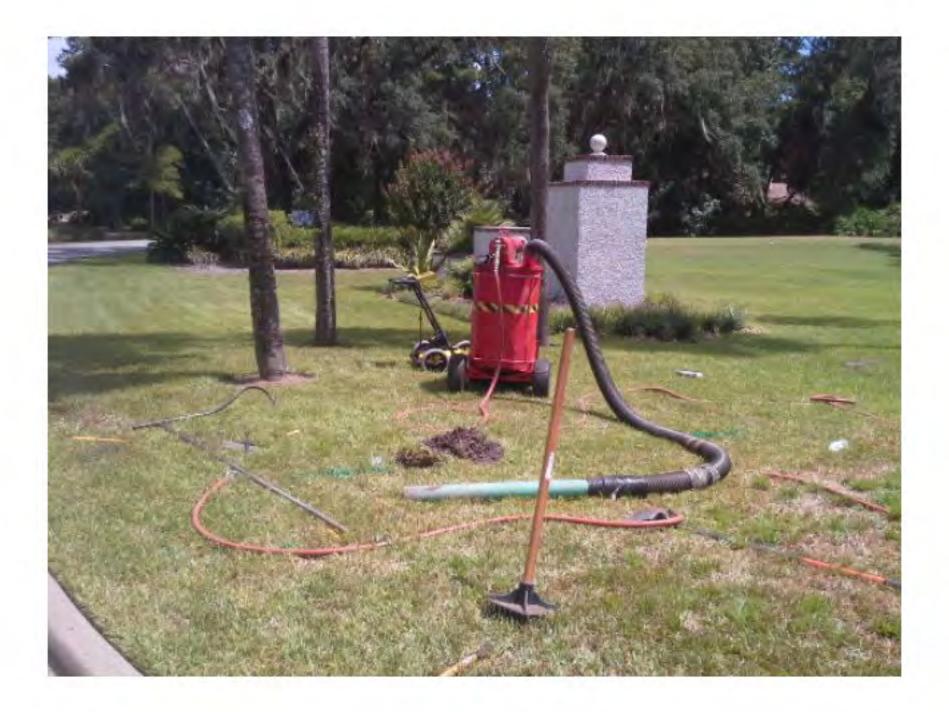




Client: Four Waters Engin			Test Hole# 20
General Location: Fredr	ica Road and N. Windward Dr.		_ Date: 8-24-16
City: St. Simons Island	_County: Glen County	State:	GA Crew Chief: Ira
Notes:			
Utility: Sanitary Size:	8" Material: Plastic	_ Condition: <u>Good</u>	_ Ribbon Color:
Maker Set:	_Asphalt Thickness:	_Soil Type: Sand	_Field Condition:_Good

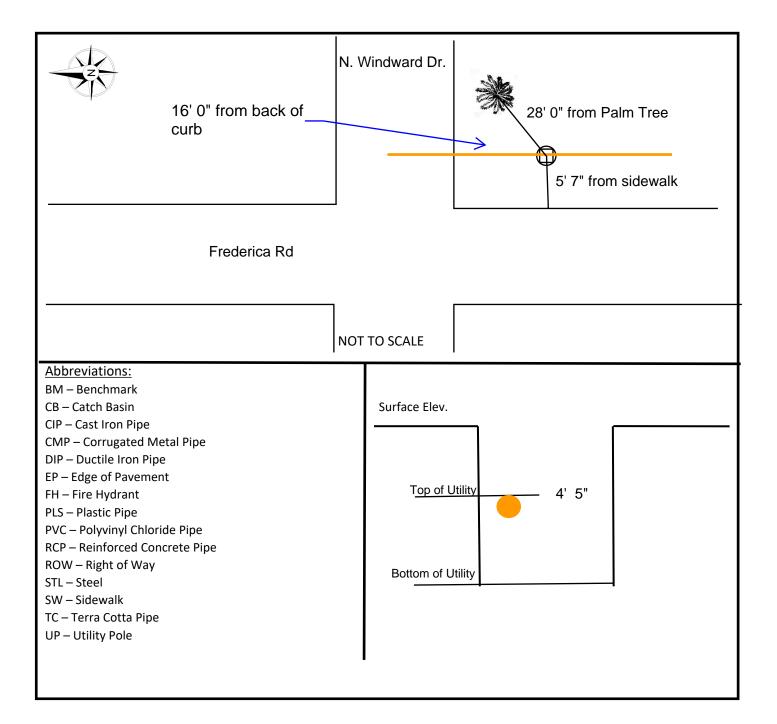








Client: _	Four Water	rs Engin	eering, Inc.	Project:_	Sea I	sland Roa	d			21
General	Location:	Fredric	ca Road and	N. Windwa	rd Dr.			Date:	8-24-16	
City: St	. Simons Is	land	County: G	en County			State:	GA	Crew Chief:	Ira
Notes: _										
Utility:	Telecom	Size:	2" cable_Mate	erial:		Condition:	Good	Ribbo	n Color:	
Maker S	Set:		_Asphalt Thicl	kness:		Soil Type:_	Sand	_ Field (Condition: Go	od

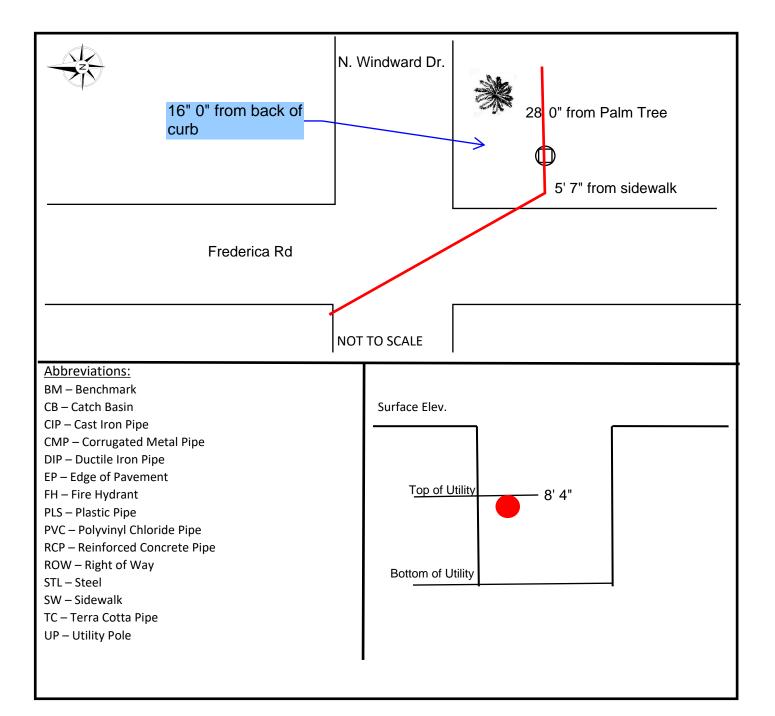




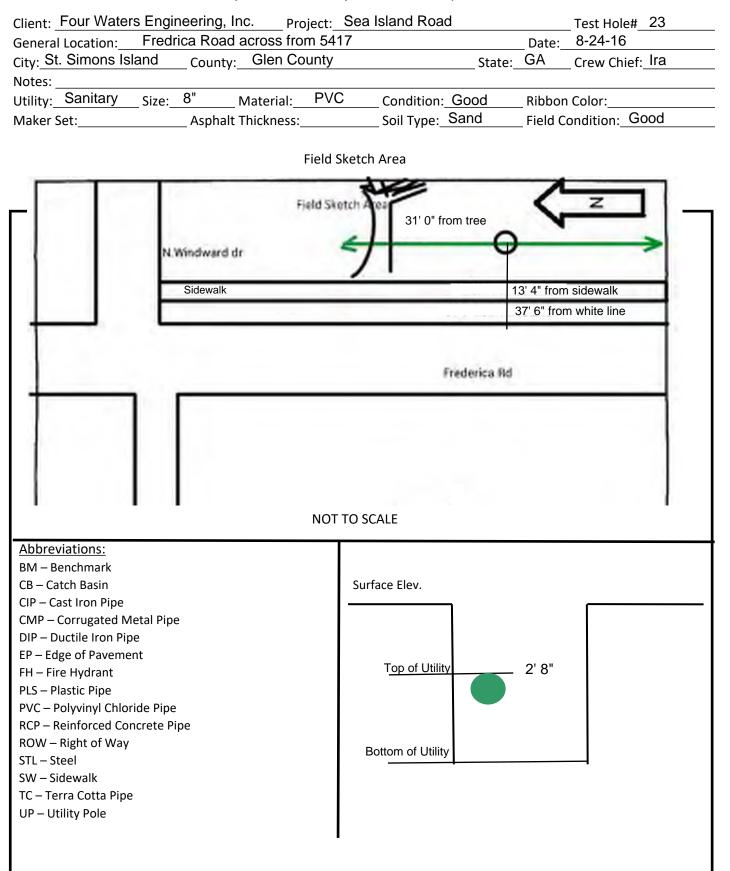




Client: Four Waters	Engineering, Inc. Project: Sea	Island Road Test Hole#_22
General Location:	Fredrica Road and N. Windward Dr.	Date: <u>8-24-16</u>
City: St. Simons Isla	and County: Glen County	State: GA Crew Chief: Ira
Notes: Unable to s	ee the power line due to water table.	We were able to probe down to the conduit.
Utility: Power	Size: See notes Material:	_ Condition: <u>Good</u> Ribbon Color:
Maker Set:	Asphalt Thickness:	_Soil Type: Sand Field Condition: Good







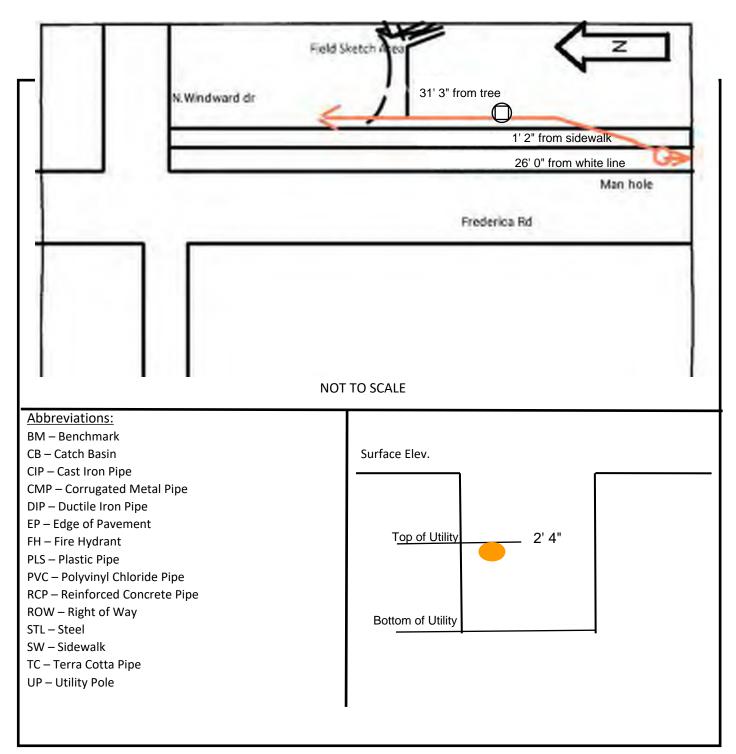




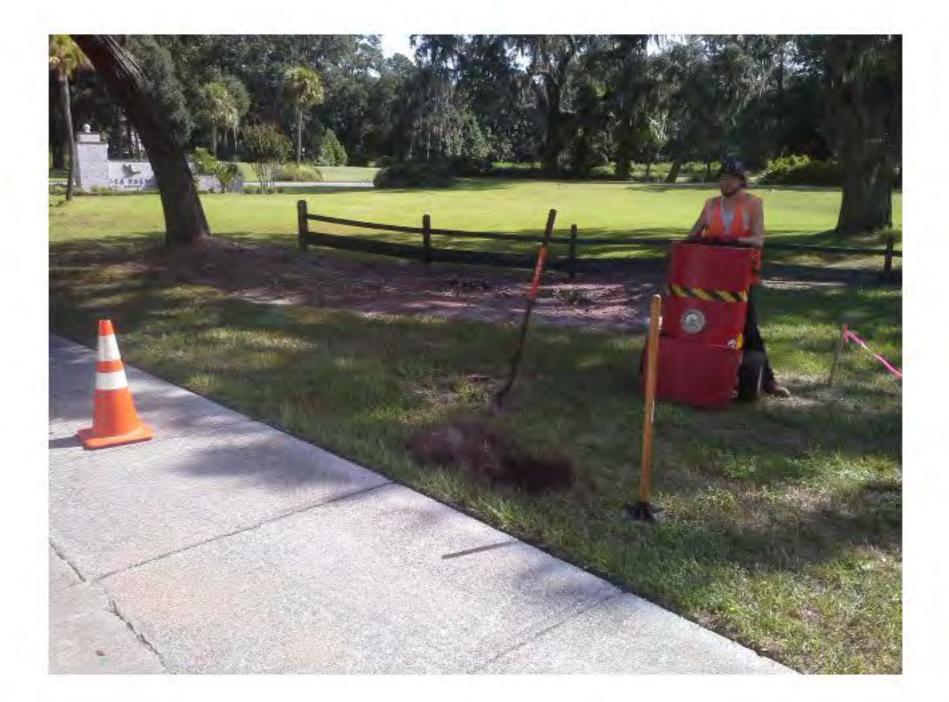




Client: Four Waters	Engineering, Inc. Project: Sea Isl	and Road	Test Hole#_24
General Location:	Fredrica Road across from 5417	Date:	8-24-16
City: St. Simons Isla	and County: Glen County	State: GA	Crew Chief: Ira
Notes:			
Utility: Telecom	Size: Material: C	ondition: <u>Good</u> Ribbon	Color:
Maker Set:	Asphalt Thickness: S	oil Type: Sand Field Co	ondition: Good



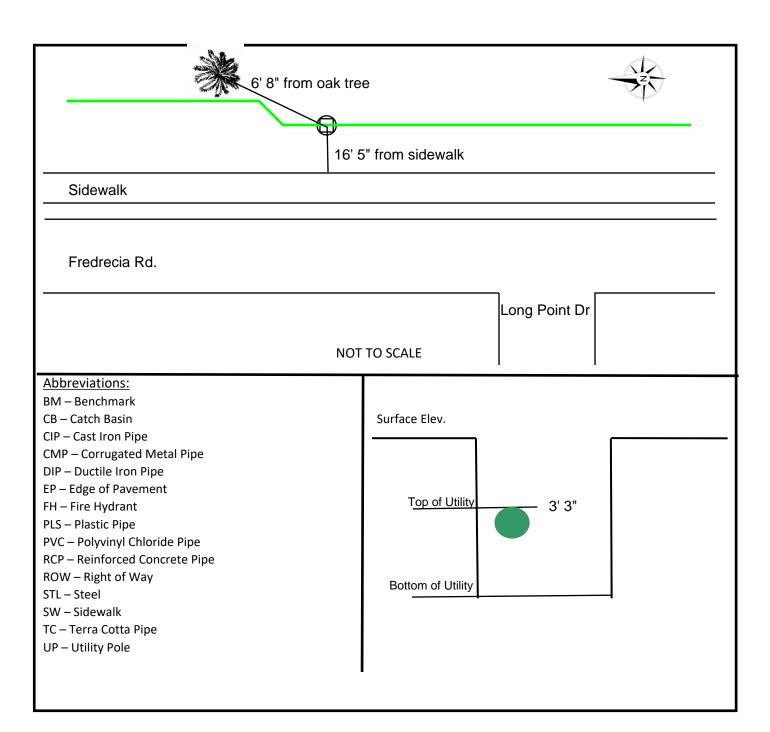








Client: Four Waters Engin	neering, Inc. Project: Sea	Island Road	Test Hole#_25
General Location: Fredr	ica Road and Long Point Drive	<u> </u>	oate: 8-22-16
City: St. Simons Island	_ County: Glen County	State: G	A Crew Chief: Ira
Notes:			
Utility: Sanitary Size:	8" Material: PVC	_ Condition: <u>Good</u> Ri	ibbon Color:
Maker Set:	Asphalt Thickness:	_Soil Type: Sand Fi	ield Condition: Good

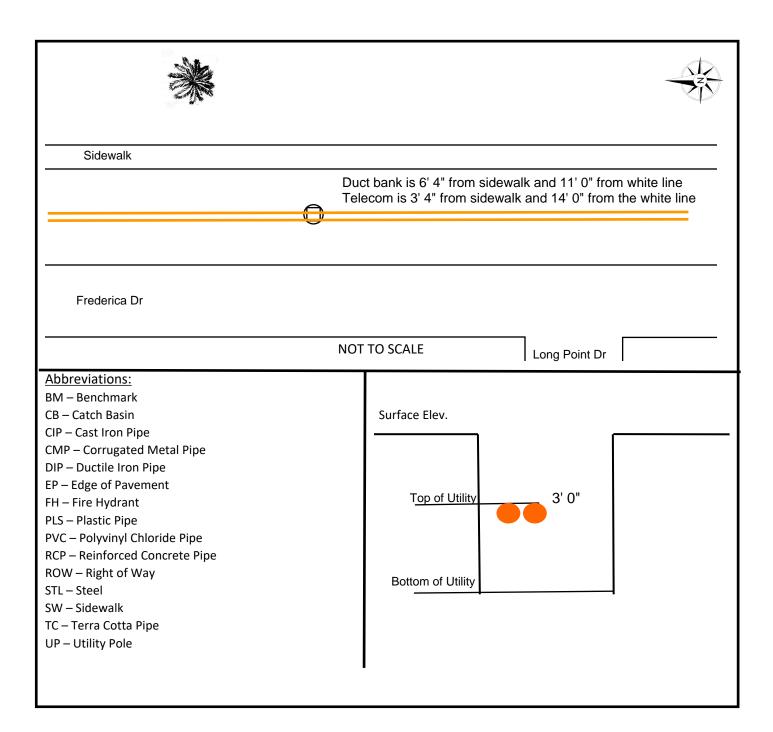


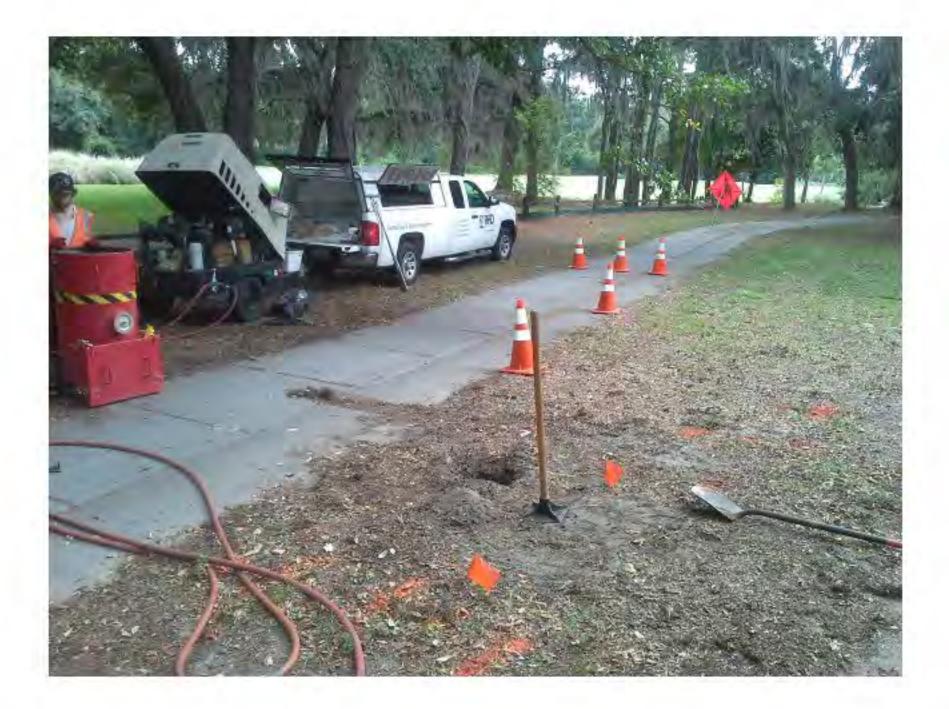




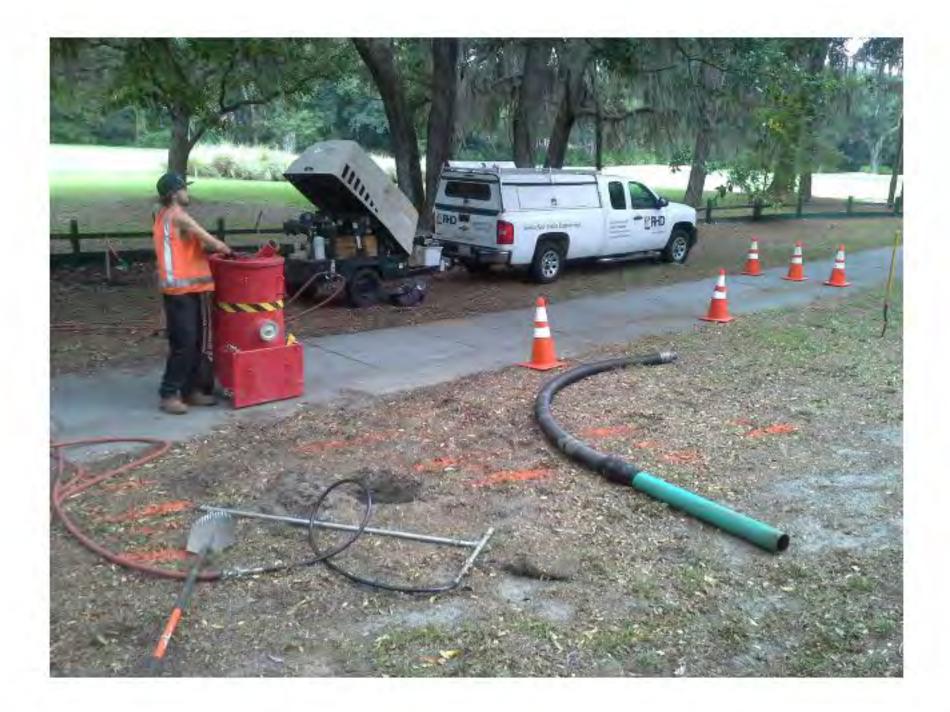


Client: Four Waters Engi	neering, Inc. Project: Se	ea Island Road	Test Hole#_26
General Location: Fredr	ica Road and Long Point Dr	ive	Date:(8-22-16)-(8-27-16)
City: St. Simons Island	_ County: Glen County	State:	GA Crew Chief: Ira
Notes:			
Utility: Fiber duct banke:	Material:	Condition: Good	_ Ribbon Color:
Maker Set:	Asphalt Thickness:	Soil Type: Sand	Field Condition: Good







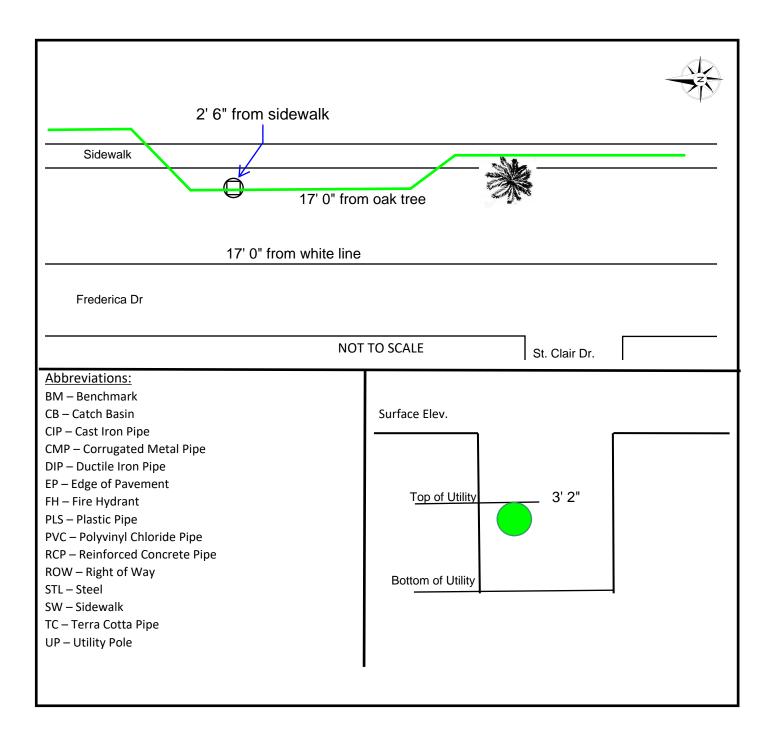








Client: Four Waters Engineering, Inc. Project: Sea	Island Road Test Hole#_27
General Location: Fredrica Road and Saint Clair Drive	Date:(8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: Sanitary Size: 8" Material: PVC	_ Condition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:	_Soil Type: Sand Field Condition: Good









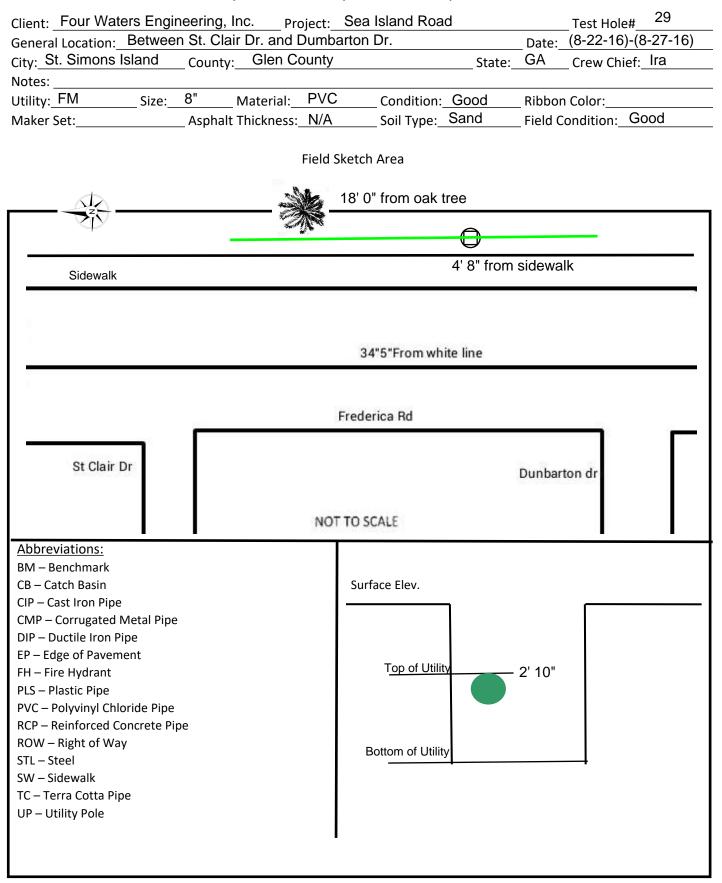
630 10th St. NW Atlanta, GA 30318 404-874-8585 RHDServices.com

Quality Level-A Utility Test Hole Report

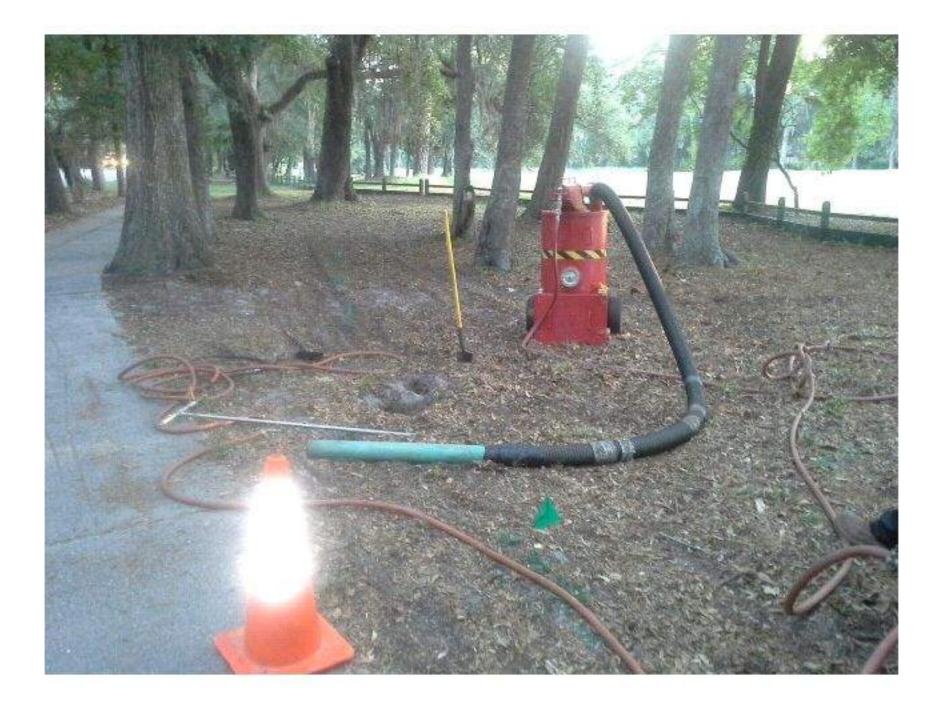
Client: Four Water	s Engin	eering, Inc. Project: S	ea Island Road	Test Hole#_28
General Location:	Fredrie	ca Road and St. Clair Drive	е	Date: (8-22-16)-(8-27-16)
City: St. Simons Is	land	County: Glen County		State: GA Crew Chief: Ira
Notes:				
Utility:	_Size:	Material:	Condition:	Ribbon Color:
Maker Set:		_Asphalt Thickness:	Soil Type:	Field Condition:

#28 is not used. No #28 Test Hole	
 NOT	TO SCALE
Abbreviations: BM – Benchmark CB – Catch Basin CIP – Cast Iron Pipe CMP – Corrugated Metal Pipe DIP – Ductile Iron Pipe EP – Edge of Pavement FH – Fire Hydrant PLS – Plastic Pipe PVC – Polyvinyl Chloride Pipe RCP – Reinforced Concrete Pipe ROW – Right of Way STL – Steel SW – Sidewalk TC – Terra Cotta Pipe UP – Utility Pole	Surface Elev.





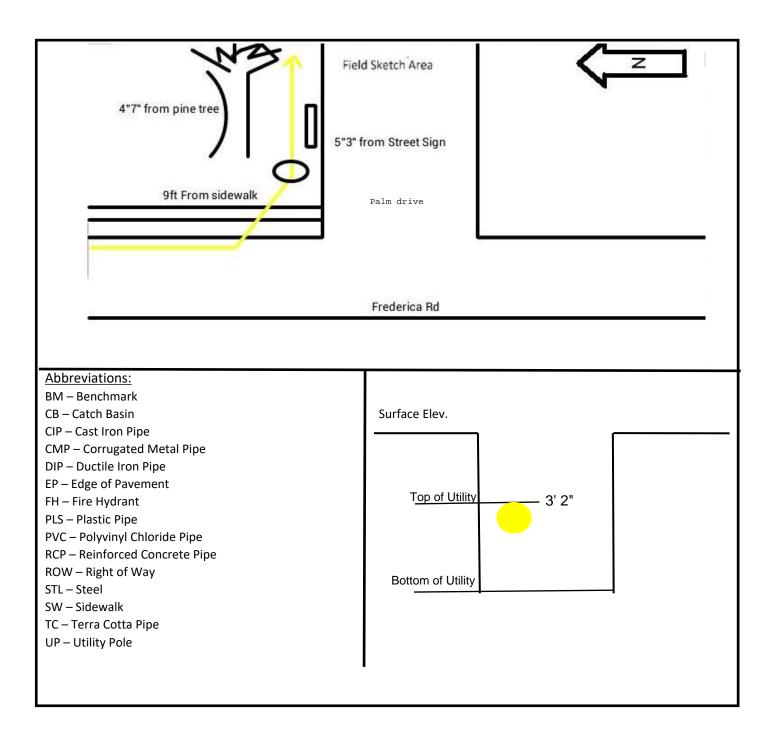








Client: Four Waters Engineering, Inc. Project: Sea	Island Road Test Hole#_30	
General Location: Fredrica Road and Palm Drive	Date:_(8-22-16)-(8-27-16)	
City: St. Simons Island County: Glen County	State:GA Crew Chief:_Ira	
Notes:		
Utility: Gas Size: Material: Steel	_ Condition:_ Good Ribbon Color:	
Maker Set: Asphalt Thickness:	_Soil Type:_Sand Field Condition:_Good	



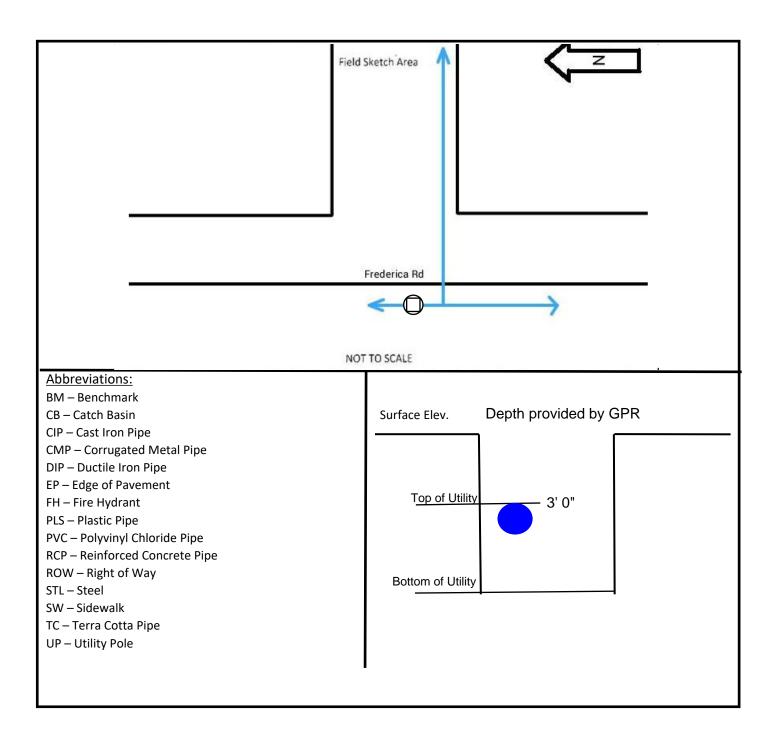






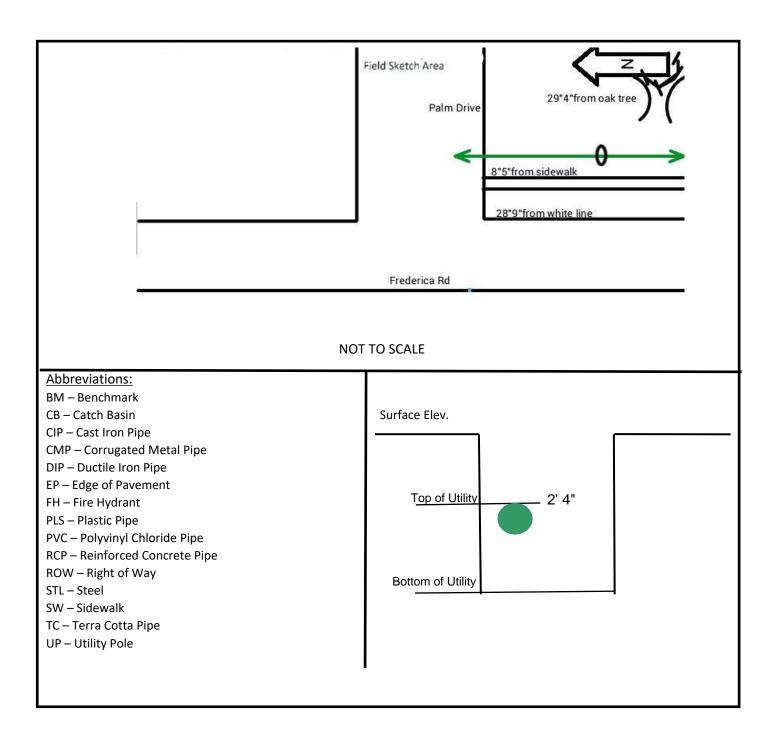


Client: Four Waters Engineering, Inc.	Project: Sea Island Road	Test Hole#31
General Location: Fredrica Road and	Palm Drive	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Gle	en County State:	GA Crew Chief: Ira
Notes: Located under pavement. Dept	th provided by GPR	
Utility:Size:Mate	erial: Condition:_Good	_ Ribbon Color:
Maker Set: Asphalt Thick	kness:Soil Type:_Sand	_ Field Condition: Good





Client: Four Waters Engineering, Inc. Project: Sea	Island Road Test Hole#_32
General Location: Fredrica Road	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State:GACrew Chief:_Ira
Notes:	
Utility: FM Size: 8" Material: PVC	_ Condition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:	_Soil Type:_Sand Field Condition:_Good



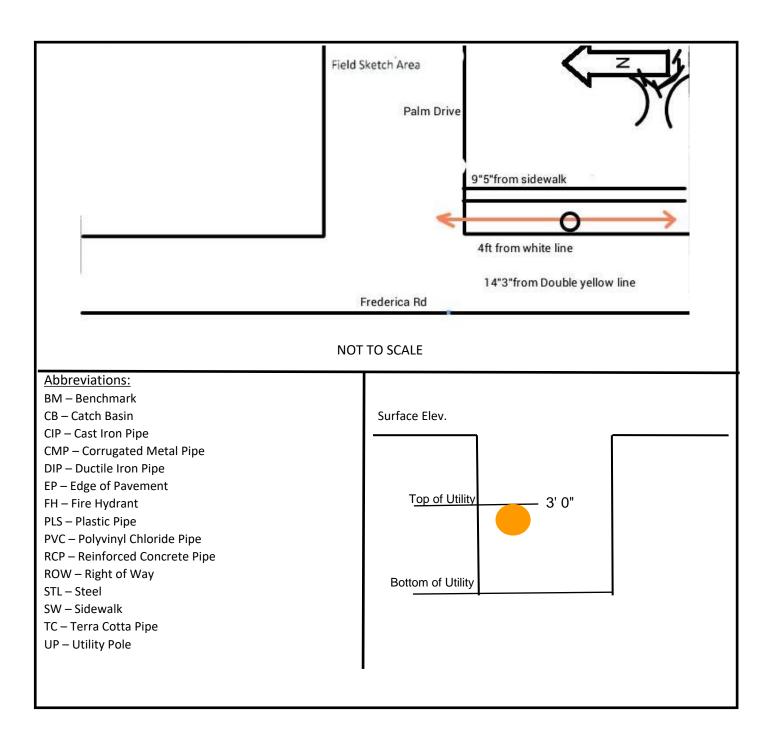








Client: Four Waters	s Engineering, Inc Project:Sea	Island Road	Test Hole# <u>33</u>
General Location:	Fredrica Road and Palm Drive		_ Date: <u>(8-22-16)-(8-27-16)</u>
City: St. Simons Isla	and County: Glen County	State:	GA Crew Chief: Ira
Notes:			
Utility: Telecom	_Size: Aprox 2" Material: Cable	_ Condition: Good	_ Ribbon Color:
Maker Set:	Asphalt Thickness:	_Soil Type: Sand	_ Field Condition:_ Good

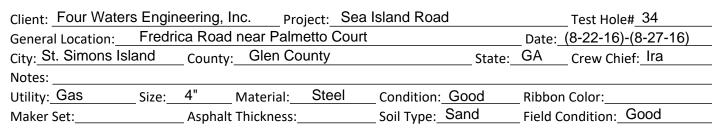


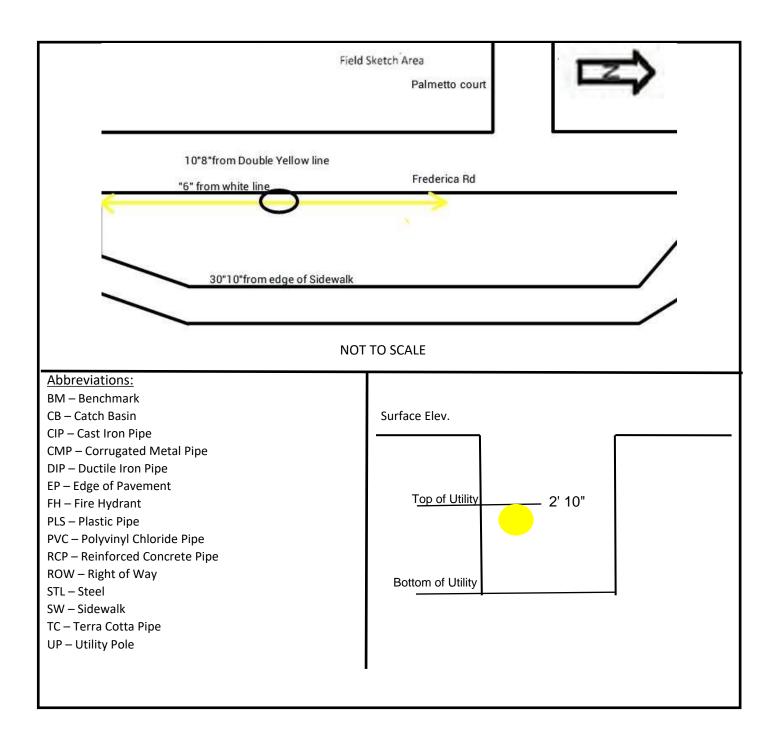












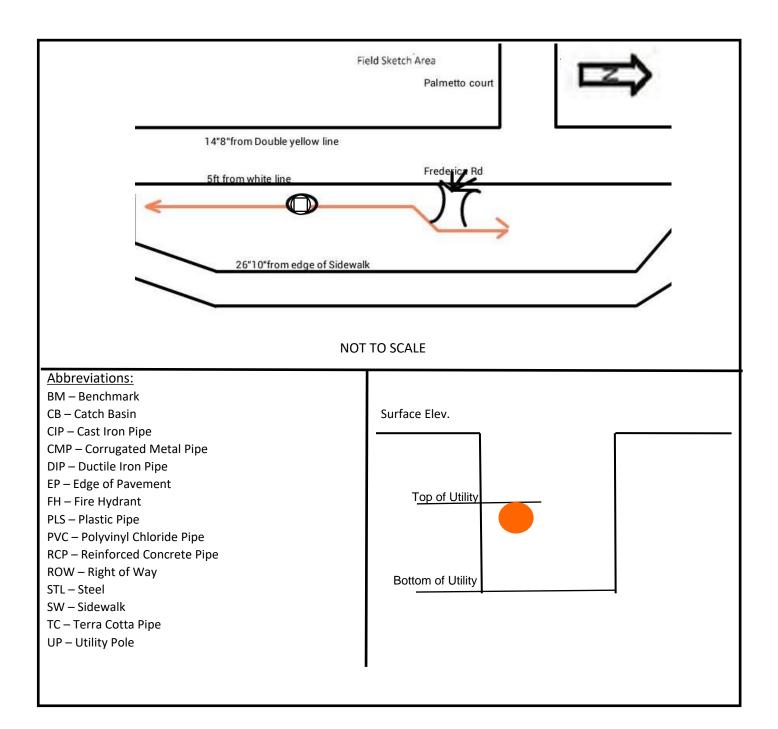








Client: Four Waters Engineering, Inc. Project: Sea Is	sland Road Test Hole#_35
General Location: Fredrica Road near Palmetto Court	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State:GACrew Chief:_Ira
Notes:	
Utility: Telecom Size: 1.5" Material: Cable	Condition: Good Ribbon Color:
Maker Set: Asphalt Thickness:N/A	Soil Type: Sand Field Condition: Good



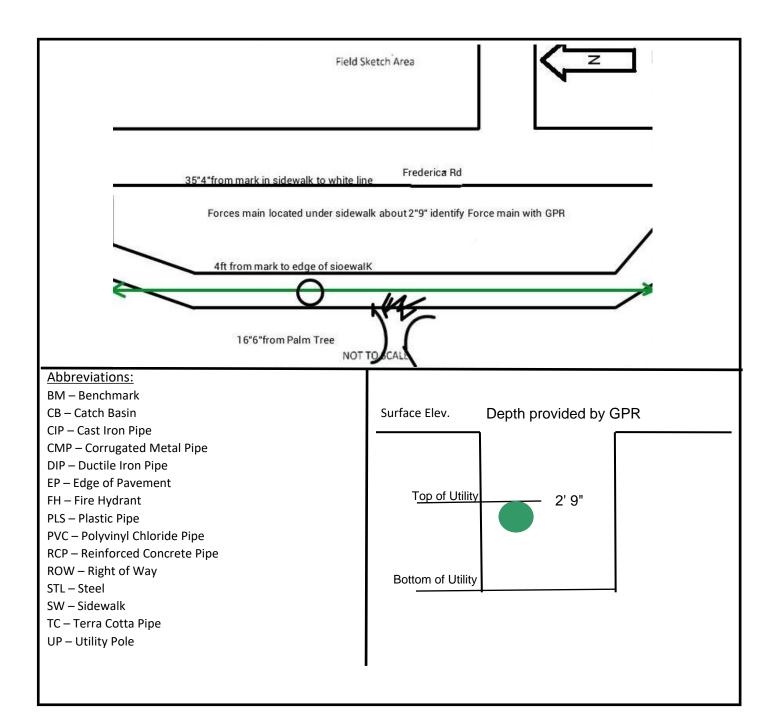








Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole# <u>_36</u>
General Location: Fredrica Road	_ Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County State:	GA Crew Chief: Ira
Notes: This was under the sidewalk. Depth provided by GPR	
Utility: FM Size: 8" Material: PVC Condition: Good	_ Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type:_ Sand	Field Condition: Good

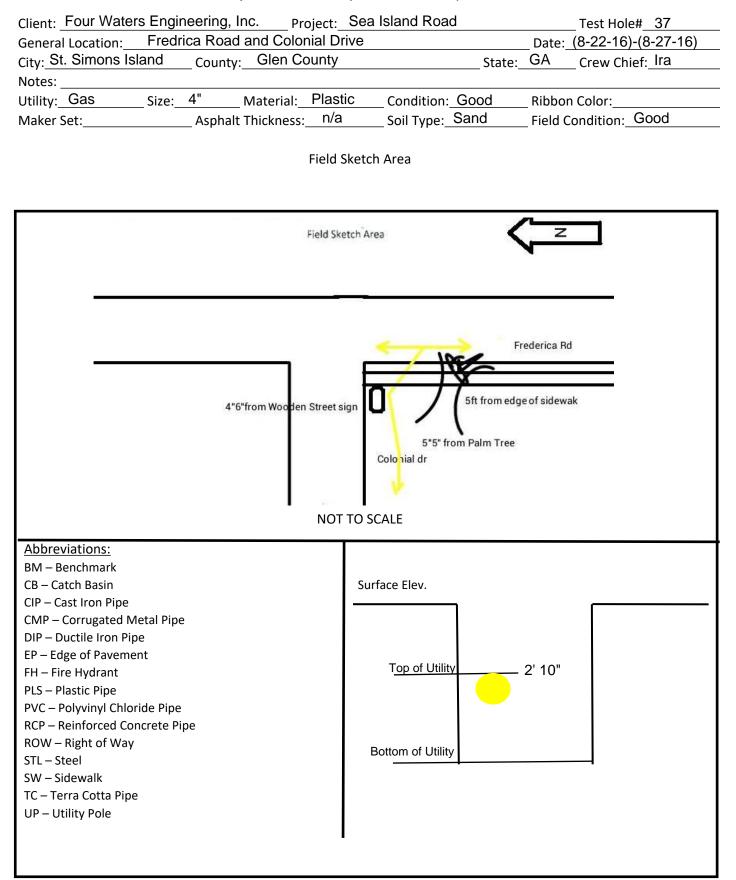












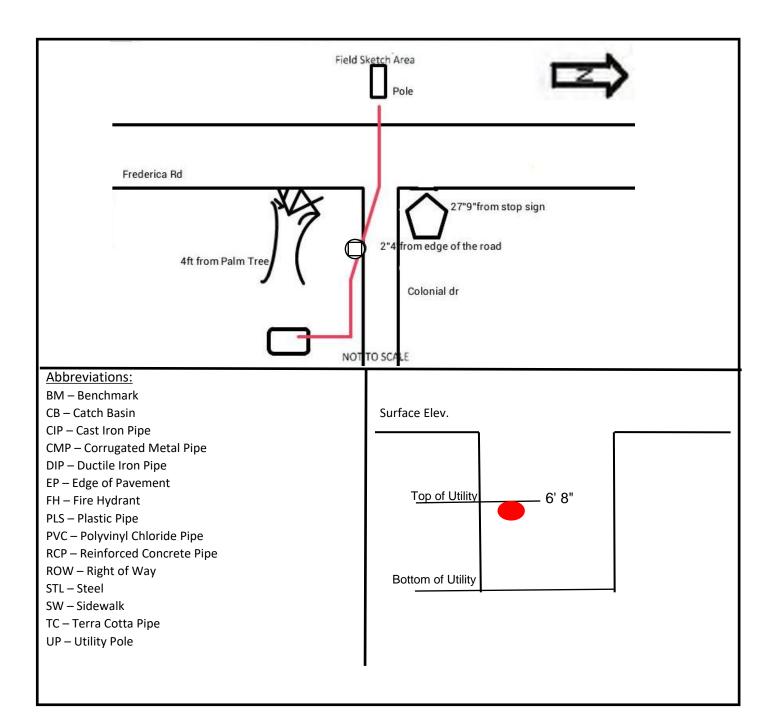








	t: Sea Island Road Test Hole# 38	
General Location: Fredrica Road and Colonial	Drive Date: (8-22-16)-(8-27-16)	-16)
City: St. Simons Island County: Glen Count	tyState:GACrew Chief:_Ira	
Notes:		
Utility: Power Size: Aprox 3" Material: Cat	bles Condition: <u>Good</u> Ribbon Color:	
Maker Set: Asphalt Thickness:	Soil Type: Sand Field Condition: Good	



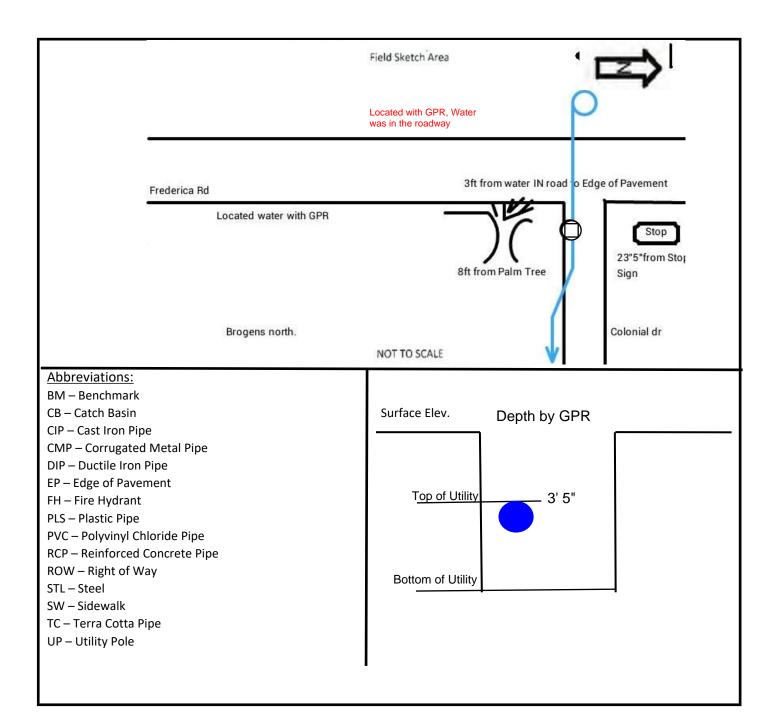








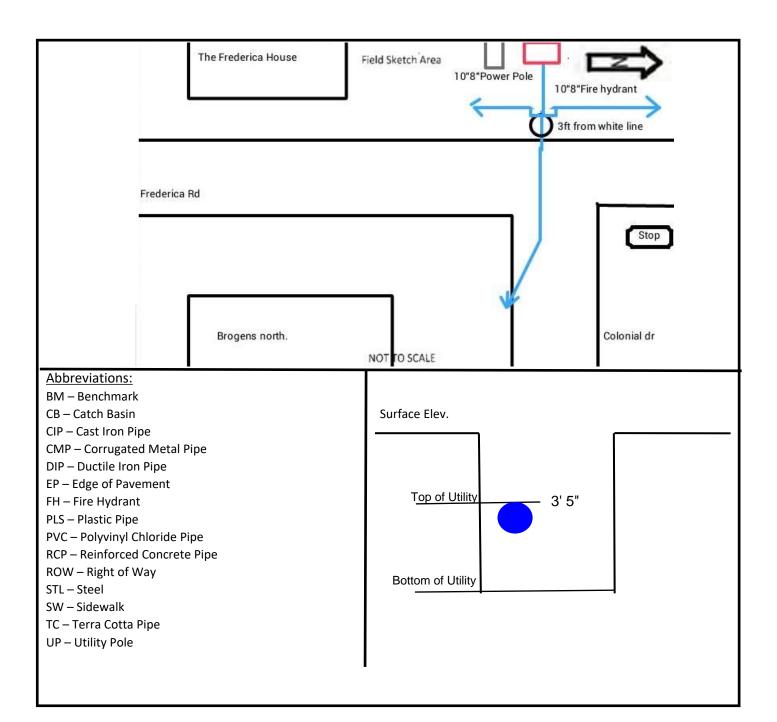
Client: Four Waters Engineering, Inc. Project: Sea	a Island Road Test Hole#_ 39
General Location: Fredrica Road	Date: <u>(8-22-16)-(8-27-16)</u>
City: St. Simons Island County: Glen County	State:GA Crew Chief: Ira
Notes: This was under the pavement. Depth by GPR	
Utility: <u>Water</u> Size:Material: <u>A/C</u>	Condition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:	Soil Type:_SandField Condition:_Good







Client:	Four Wate	ers Engir	neering, In	C. Project:	Sea Island Road			_ Test Hole#_40
Genera	Location:	Fredri	ica Road				Date:_	(8-22-16)-(8-27-16)
City: St	. Simons Is	sland	_ County:_	Glen County			GA	_ Crew Chief: Ira
Notes:								
Utility:_	Water	Size:	<u>6"</u> N	Material: A/C	Condition: Go	bod	Ribbor	n Color:
Maker S	Set:		_ Asphalt T	hickness: N/A	Soil Type: Sa	nd	Field C	ondition: Good

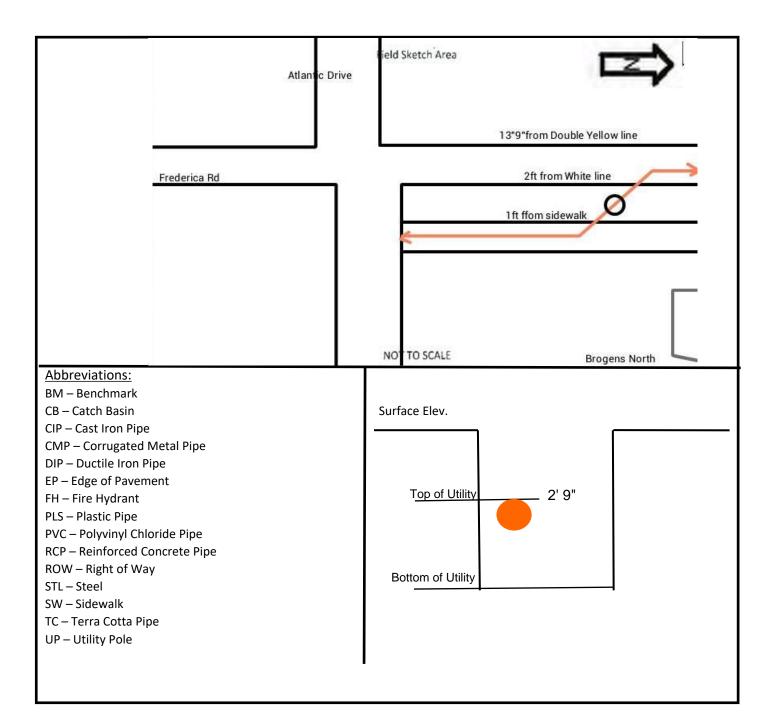


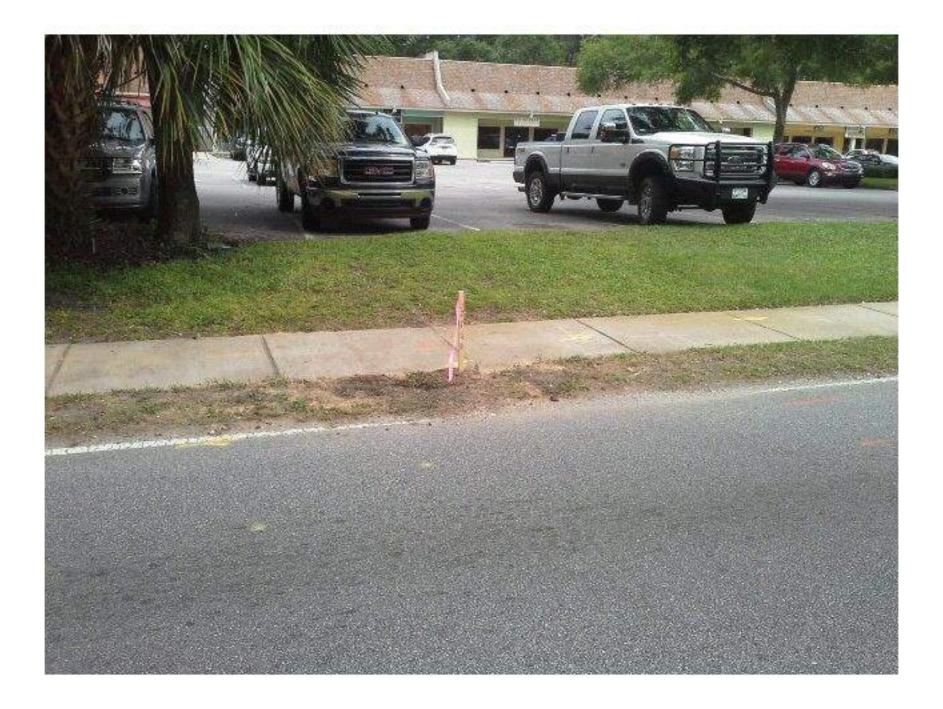






Client: Four Water	s Engi	neering, In	C. Project:	Sea Island Ro	ad		Test Hole#41
General Location:	Fred	rica Road				_ Date:	(8-22-16)-(8-27-16)
City: St. Simons Isl	land	County:_	Glen County		State:	GA	_ Crew Chief: Ira
Notes:							
Utility: Telecom	_Size:_	2" N	Material: Cab	le Condition	n: Good	_ Ribbo	n Color:
Maker Set:		Asphalt 1	hickness:	Soil Type	: Sand	_ Field (Condition: Good



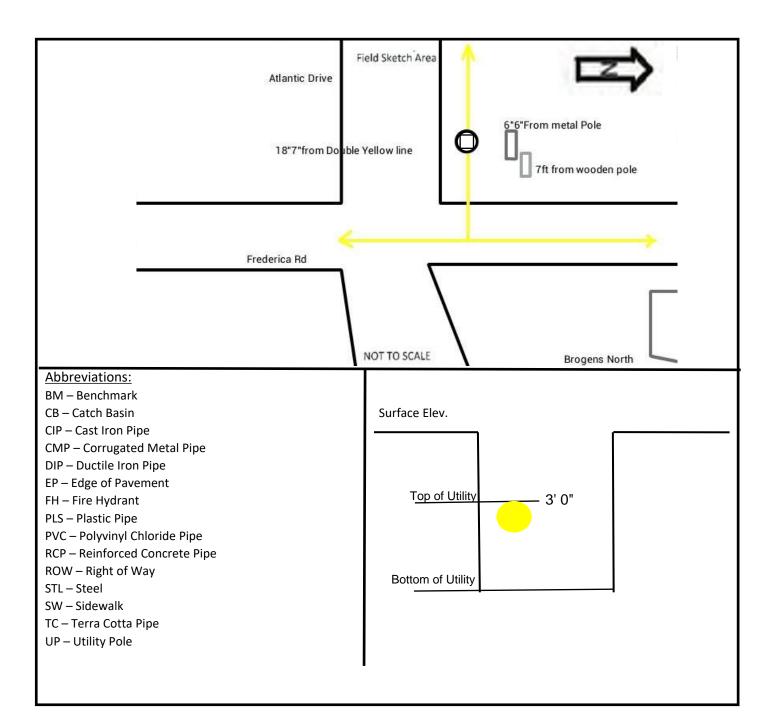


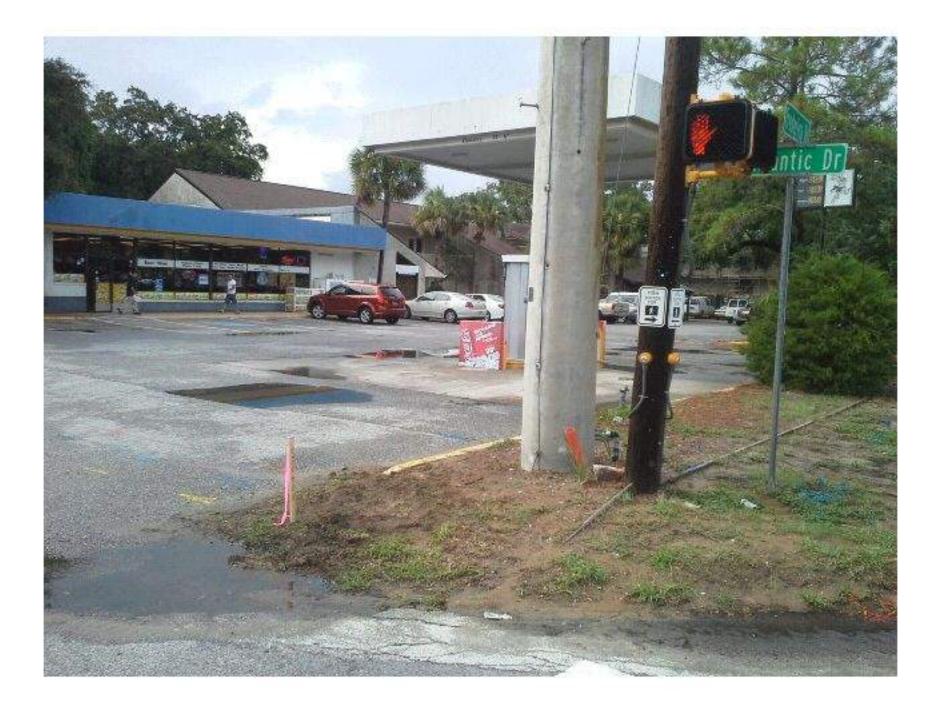






Client: Four Waters Engineering, Inc. Project: Sea Isla	and Road Test Hole#_42
General Location: Fredrica Road and Atlantic Drive	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: Gas Size: 2" Material: Steel Co	ondition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:n/a Sc	oil Type:_SandField Condition:_Good



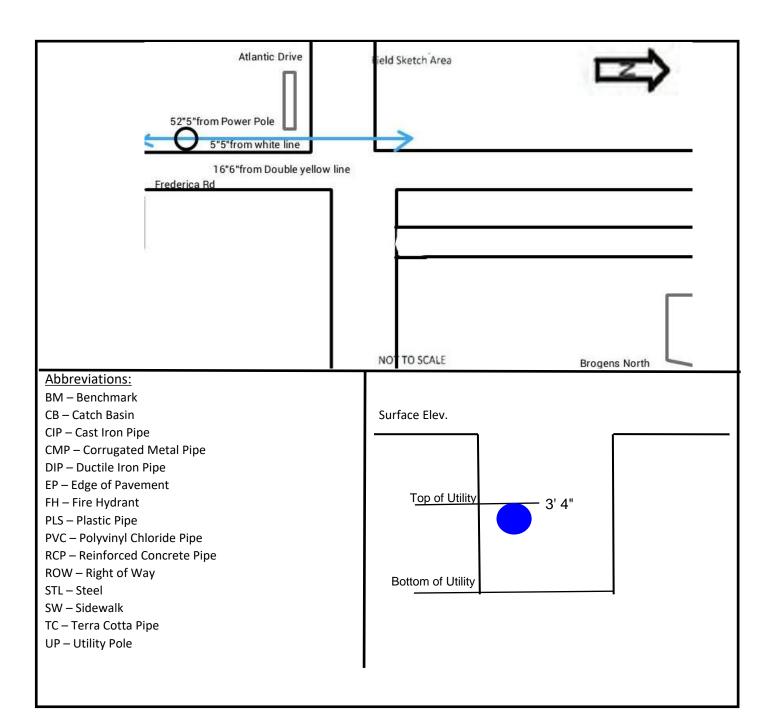








Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#_ 43
General Location: Fredrica Road	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: <u>Water</u> Size: <u>12</u> " Material: <u>A/C</u> Condition: <u>C</u>	Good Ribbon Color:
Maker Set: Asphalt Thickness:n/a Soil Type:_S	and Field Condition: Good



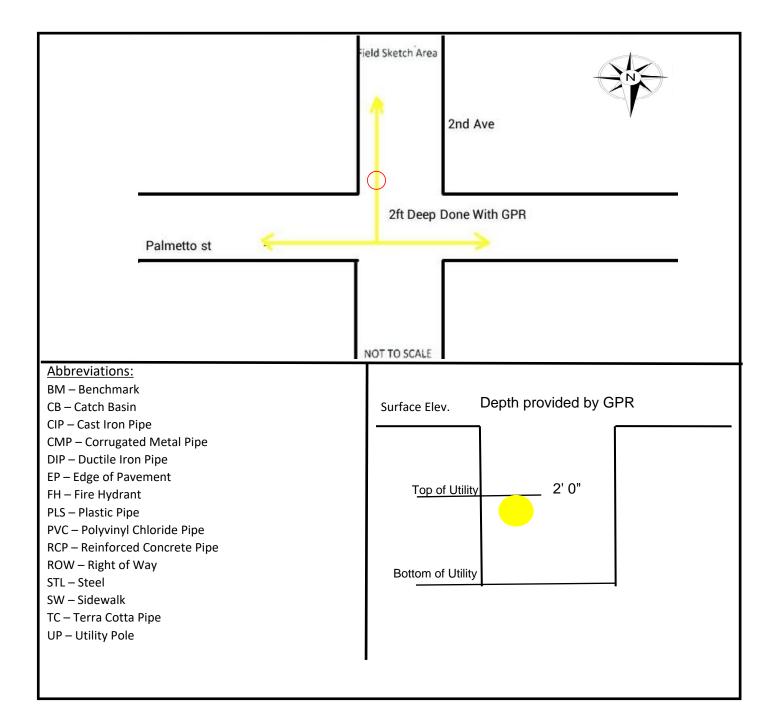








Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#_44
General Location: JPalmetto Street & 2nd Ave.	Date: <u>(8-22-16)-(8-27-16)</u>
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes: Located under pavement. Depth provided by GPR	
Utility: Gas Size: 2" Material: Plastic Condition: Go	od Ribbon Color:
Maker Set: Asphalt Thickness:n/a Soil Type:_Sar	field Condition: Good

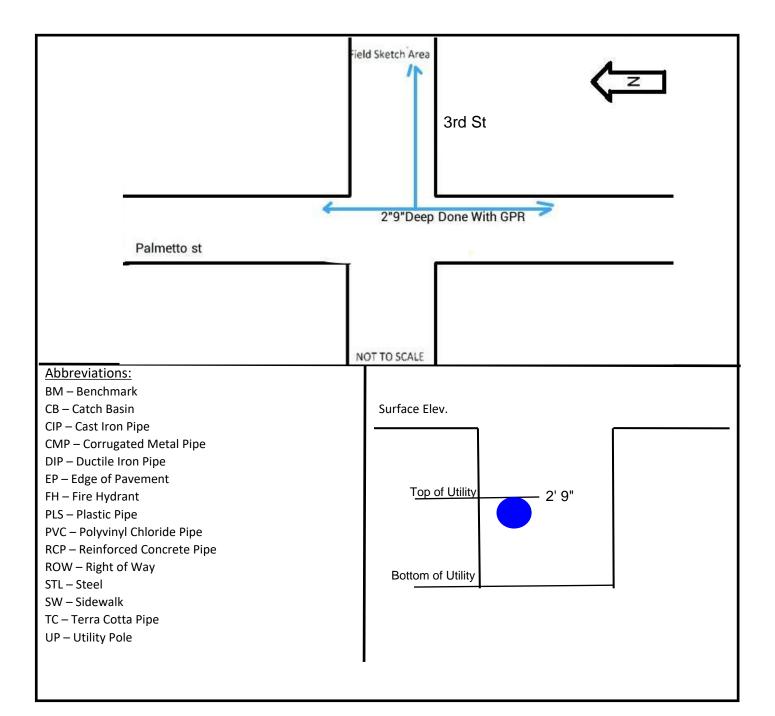




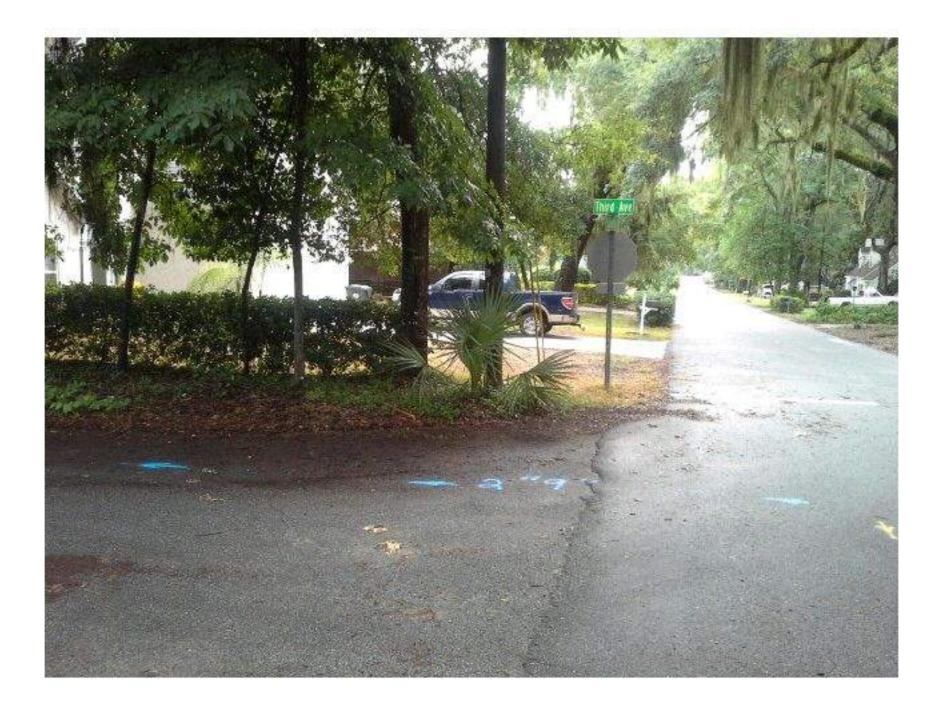




Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#45`
General Location: Fredrica Road and 3rd Street	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County Sta	ate: GA Crew Chief: Ira
Notes: Located under pavement. Depth provided by GPR	
Utility: Water Size: Material: Plastic Condition: Good	Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type:_ Sand	Field Condition: Good



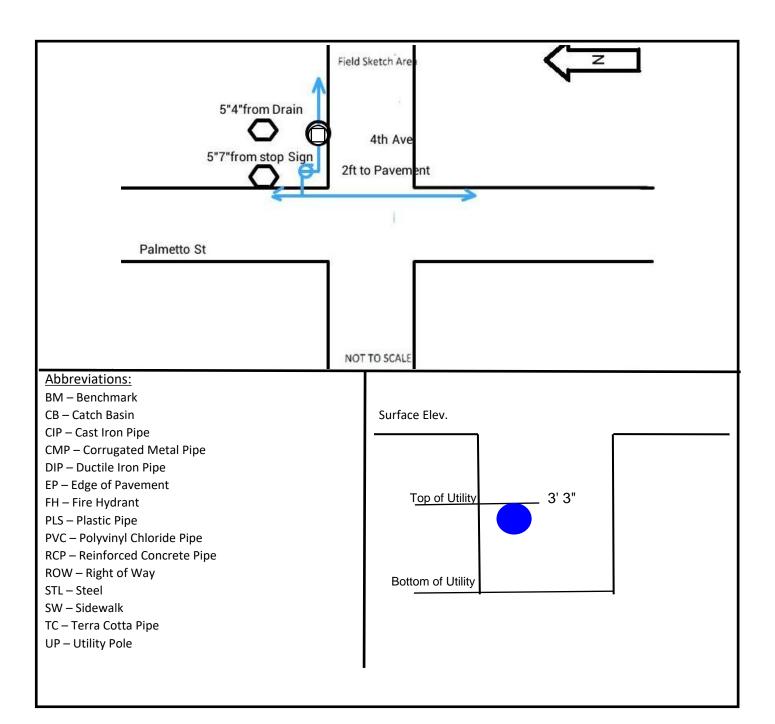






Client: Four Waters Engineering, Inc. Project: Sea I	sland Road Test Hole#_46
General Location: Fredrica Road	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: Water Size: 2" Material: PVC	Condition: Good Ribbon Color:
Maker Set: Asphalt Thickness:n/a	Soil Type: Sand Field Condition: Good









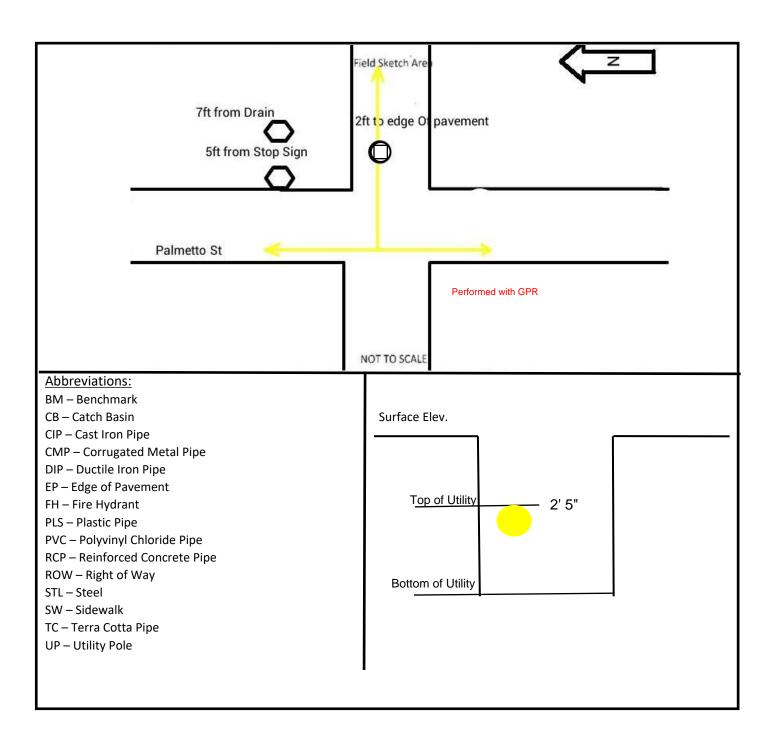


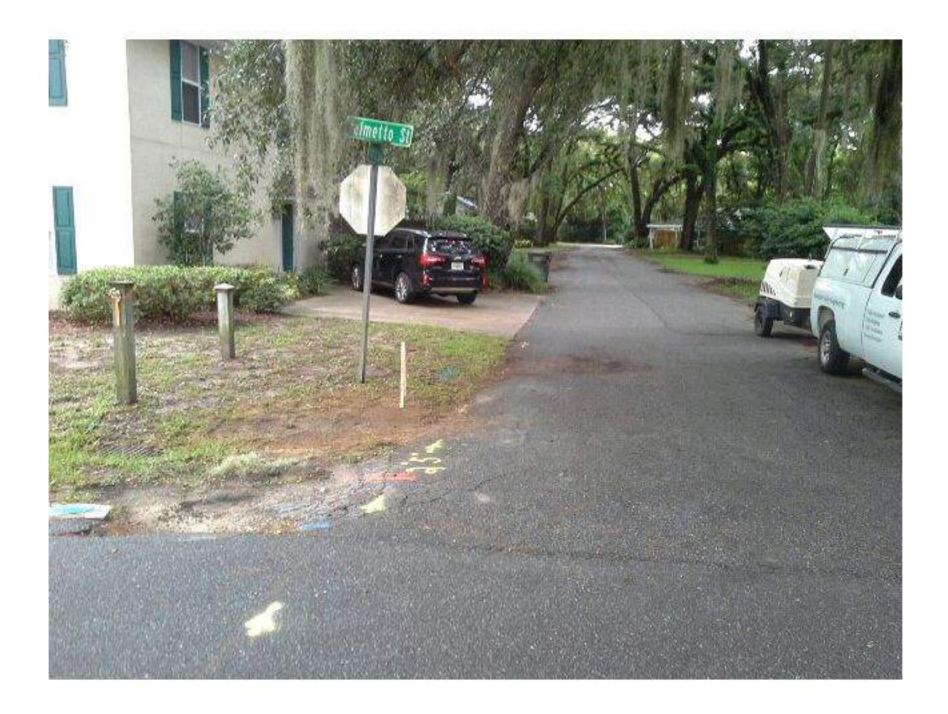


Quality Level-A Utility Test Hole Report

Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#_ 47
General Location: Fredrica Road and Fourth Ave	Date: <u>(8-22-16)-(8-27-16)</u>
City: St. Simons Island County: Glen County St	ate: GA Crew Chief: Ira
Notes: Located under pavement. Depth provided by GPR	
Utility: Gas Size: Material: Plastic Condition: Good	Ribbon Color:
Maker Set: Asphalt Thickness: Soil Type: Sand	Field Condition: Good

Field Sketch Area



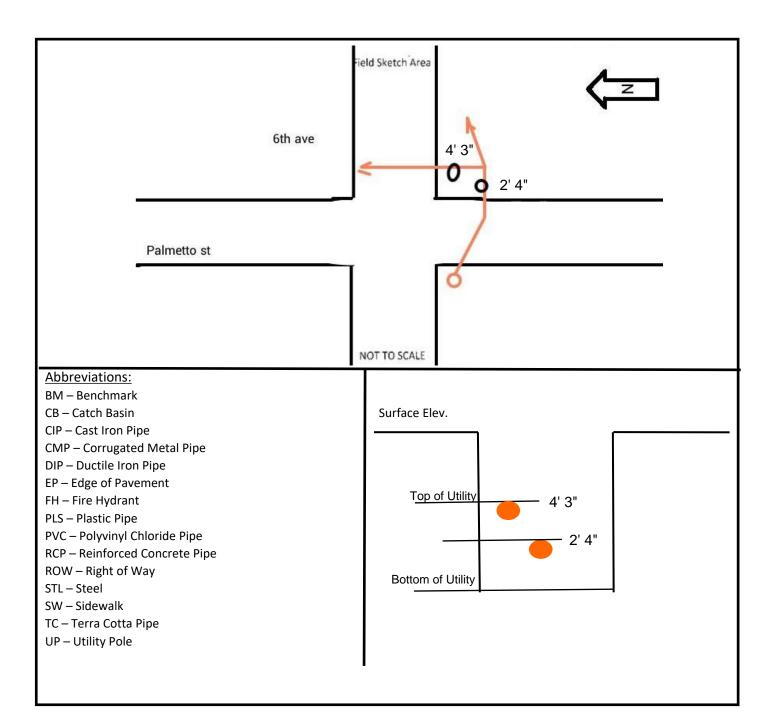




Quality Level-A Utility Test Hole Report

Client: Four Waters Engineering, Inc. Project: Sea	Island Road Test Hole#_48
General Location: Fredrica Road and 6th Ave	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County	State: GA Crew Chief: Ira
Notes:	
Utility: Telecom Size: Aprox 2" Material: Conduit	_ Condition: <u>Good</u> Ribbon Color:
Maker Set: Asphalt Thickness:	_Soil Type: Sand Field Condition: Good

Field Sketch Area









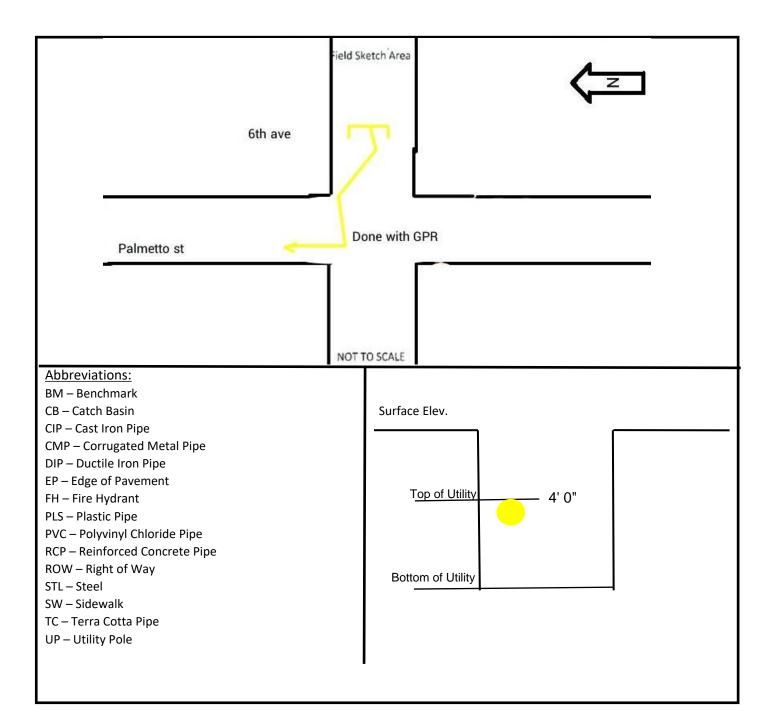




Quality Level-A Utility Test Hole Report

Client: Four Waters Engineering, Inc. Project: Sea Island Road	Test Hole#_49
General Location: Fredrica Road and 6th Ave.	Date: (8-22-16)-(8-27-16)
City: St. Simons Island County: Glen County State:	GA Crew Chief: Ira
Notes: Located under pavement. Depth provided by GPR	
Utility: Gas Size: Material: Plastic Condition: Good	_ Ribbon Color:
Maker Set: Asphalt Thickness: n/a Soil Type: Sand	_ Field Condition: Good

Field Sketch Area





PUMP STATION 2032 REGIONAL FORCEMAIN IMPROVEMENTS ST. SIMONS ISLAND, GEORGIA

ATTACHMENT

Wetland Evaluation Letter: Field Inspections for Wetlands on Proposed JWSC LS2032 Forcemain Project

Southeastern Environmental Associates, LLC

Sam N. Latham & Son 710 Ft. McIntosh Loop Hortense, Georgia 31543 Office: 912-778-5850 Mobile: 912-223-2865 slatham7@hotmail.com

August 24, 2016

Four Waters Engineering, Inc. ATTN: Michael Klink 324 6th Avenue North Jacksonville Beach, FL 32250

RE: Field site inspections for wetlands on the proposed JWSC LS2032 Forcemain Project on St. Simons Island, Glynn County, Georgia

Dear Michael,

Southeastern Environmental Associate (SEA) performed 08/18/2016 field site inspections on the above mentioned site to determine the locations, extent, and jurisdiction of any wetlands on this site. SEA is pleased to inform you that our inspections revealed that, in our professional opinions, there are no wetlands present here that would be impacted by this project.

The drainage ditch located at LS 2032 near Station 11 + 75 that you had mentioned some concern about is not close enough to the construction area to be impacted, however, we do believe that your idea to place the silt screen around any drainage ditches that are close to the construction areas is good so there is no questions about a possible impact.

Sincerely,

Sumn tothe

Sam N. Latham

PUMP STATION 2032 REGIONAL FORCEMAIN IMPROVEMENTS ST. SIMONS ISLAND, GEORGIA

ATTACHMENT

Georgia Environmental Protection Division LS2032 Regional FM Improvements Project NPDES Permit No. GA0021521



ENVIRONMENTAL PROTECTION DIVISION

Richard E. Dunn, Director

Watershed Protection Branch 2 Martin Luther King, Jr. Drive Suite 1152, East Tower Atlanta, Georgia 30334 404-463-1511

October 5, 2016

Mr. Jimmy Junkin, Executive Director Brunswick-Glynn Joint Water & Sewer Commission 1703 Gloucester Street Brunswick, Georgia 31520

RE:

Lift Station 2032 Regional Forcemain Improvement Plans and Specifications NPDES Permit No. GA0021521 EPD # 2016-255 Glynn County

Dear Mr. Junkin:

We are in receipt of the plans and specifications for the above-referenced project. Your project has not been selected for review by the Wastewater Regulatory Program.

If you have any questions, please contact me at 404-463-4936 or at Yilin.fan@dnr.ga.gov.

Sincerely,

-an

Yilin Fan Municipal Permitting Unit Wastewater Regulatory Program

cc: Ms. Angela Bryan, P.E. – Four Waters Engineering, Inc. (abryan@4WEng.com)